ANNEX G-1. GEOLOGICAL INVESTIGATIONS NORTH OF THE REDLINE



Figure G-1-1. Boring Map of CP02-EAARS-CB-0002

10 25 11 1775,528 11 12 12 12 12 12 12 12 12 12 12 12 12	. SIZI 0. CO 1. MA 2. TO 3. TO 4. EL 5. DA 6. EL 7. TO 8. SK	Status CME CME TAL: TAL: TAL: TAL: TAL: TAL: TAL: TAL:	Plan ACTUR 5-55 SAMPL NUMBER TON G	SYSTEM DATUM HORIZONTAL IN FLE NAD83 IER'S DESIGNATION OF DRILL IN I	VERTICAL NAVD88 AUTO HAMMEI MANUAL HAM UNDISTURBED 0 COMPLETE 09-03-0	R MEI (UC
10 25 11 1775,528 11 12 12 12 12 12 12 12 12 12 12 12 12	0. CO 1. MA 2. TO 3. TO 4. EL 5. DA 6. EL 7. TO 8. SK	ORDINATE BOOK TAL	Plan ACTUR 5-55 SAMPL NUMBER TON G	SYSTEM DATUM HORIZONTAL RE, FLE NAD83 RER'S DESIGNATION OF DRILL STATED OR-13-02 OP OF BORING 12.0 FL	NAVD8	R MEI (UC
25 775,528 TOR FILE NO. 12 12 12 12 14 15 16 16 17 18 18 18 18 18 18 18 18 18 18 18 18 18	1. MA 2. TO 3. TO 4. EL 5. DA 6. EL 7. TO 8. SIG	State NUF CME TAL : TAL : EVAT TE B: EVAT	Plan ACTUR -55 SAMPL NUMBER TON G	REFLE NAD83 RER'S DESIGNATION OF DRILL SEES DISTURBED 118 ER CORE BOXES 5 ROUND WATER Not Determine STARTED 08-13-02 OP OF BORING 12.0 FL	NAVD8	R MEI (UC
775,528 TOR FILE NO. 13 13 ARING 16 16	1. MA 2. TO 3. TO 4. EL 5. DA 6. EL 7. TO 8. SIG	TAL STALLS	ACTUR -55 SAMPL NUMBER TON G	ER'S DESIGNATION OF DRILL DISTURBED 118 ER CORE BOXES 5 ROUND WATER Not Determine STARTED 08-13-02 OP OF BORING 12.0 FL	AUTO HAMME MANUAL HAM UNDISTURBED G ed COMPLETE	R MEI (UC
775,528 TOR FILE NO. 13 13 ARING 16 16	2. TO 3. TO 4. EL 5. DA 6. EL 7. TO 8. SK	TAL :	-55 SAMPL NUMBE TON G ORING	ER CORE BOXES 5 ROUND WATER Not Determine STARTED 08-13-02 OP OF BORING 12.0 FL	MANUAL HAM UNDISTURBED O ed COMPLETE	(UC
113 13 14 16 16 17 18	J. TO 4. EL 5. DA 6. EL 7. TO 8. SK	TAL EVAT	TON G	ES 118 ER CORE BOXES 5 ROUND WATER Not Determine STARTED 08-13-02 OP OF BORING 12.0 FL	0 ed COMPLETE	0
12 14 14 16 16 17 18	4. EL 5. DA 6. EL 7. TO 8. SK	TE B EVAT TAL	TON G	ROUND WATER Not Determine STARTED 08-13-02 OP OF BORING 12.0 FL	ed COMPLETE	
14 16 17 18 18 18 18 18 18 18 18 18 18 18 18 18	4. EL 5. DA 6. EL 7. TO 8. SK	TE B EVAT TAL	TON G	ROUND WATER Not Determine STARTED 08-13-02 OP OF BORING 12.0 FL	COMPLETE	7
16 16 17 18	5. DA 6. EL 7. TO 8. SK	TE B	ORING	STARTED 08-13-02 09-07-07-07-07-07-07-07-07-07-07-07-07-07-	COMPLETE	7
16 17 18	6. EL 7. TO 8. SK	TAL I	ION T	08-13-02 OP OF BORING 12.0 FL	09-03-0	2
17	7. TO	TAL				
18	8. 510	MAT	RECOV			
	-			PERY FOR BORING 80 %		_
ERIALS		H.S		NO TITLE OF INSPECTOR Civil Engineer		
ERIALS		_		- Sylva Brightess	ñ.:	-
	REG.	BOXOR	RQD OR UD	REMARKS	BLOWS.	H-VALUE
		200			60	ż
naval size				12.0		
avel size		H		2222	32	
	13	1		SPT Sampler	32	40
			ΙI	10.4	8	
					4	П
- 1	40	2	1 1	SPT Sampler	6	
		9		9.0	6	13
some silt.		4.7	1	2.0	1	F
ered.	47	3		SPT Sampler	_	
ered, id. light	4/	3		or i Sampler	_	8
7.9				7.4	4	
1	100	4		SPT Sampler	16	
		2.4		6.6	50/0.4	_
	0.1				-	
9			1			
	50	6				
1			28	HP = 100 psi		
		1	1			
11	1 1					
				2.0		
9	100	7	RQD			
	100	(5)	0			
					4	П
	80	8		SPT Sampler	4	
	1	1				33
	50	Q.	1			-
	50	Đ	100	-1.0 or i dampier	30/0,4	
		вох	RQD			
	45					
	45	T	28	HP = 100 psi		
		100 80 50	100 7 80 8	BOX 28 100 7 RQD 0 80 8	50 6 ROZ 28 DT = 80 mins HP = 100 psi 20 20 100 7 ROD 4 x 5-1/2* Diamond Impro 9 mins, 100 ps 80 8 SPT Sampler -0.6 SPT Sampler 4 x 5-1/2* Diamond Impro 9 mins, 100 psi 50 9 -1.0 SPT Sampler	28 BOX 28 HP = 100 psi 20 100 7 RQD

CEPP Final PIR and EIS

DR	ILLING	LO	G (Cont. Sheet)	10,000	ksonville	Dish	rict			OF 10 SH	EET
PROJEC			and the same of the same of	COORE	HNATE S	VSTE	MIDAT	rum j	HORIZONTAL	VERTICAL	-
_	P Evergla		gricultural Area Reservoirs	_	le Plane	_		1	NAD83	NAVD88	
06,4121	736.775	W-110-1 E	75,528	C-02-30	TION TO	9 07 0	DORIN	G			
ELEV.	DEPTH	LEGEND	CLASSIFICATION OF MAYER		REC	BOX OR	MOD OR UD		REMA	exs 65	N-VALUE
4.0	16.0		"At El. 3,0 Ft., vuggy		45		ROD 39	4.0	DT = 23	Impregnated Bit	-
		臣	Limestone, hard, unweathered, fir vuggy, trace of shell, gray	ne-grained	LNR	11	F	42	HP = 10 SPT Sa	00 psi 50/0 2' mpler	
	lacola ada				84	вф	RQD 50	4x	5-1/2" Diamond DT = 15 HP = 10		
		臣						-8.0			
					70	BOX	RQD 40	4 x	5-1/2" Diamond 8 mins, 1	Impregnated Bit 00 psi	
		王	ALEI9.0 FL. little shell							6	
10		器			33	14			SPT Sa		15
		量	At Et -10.6 Ft., trace silt		-	-	1	-10.6		8	-
		E	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		47	15			SPT Sa	mpler 9	
-120	24.0	E				1		-12.0	a. 1.04	10	15
-		-:111	SAND, poorly-graded with silt, sor fine-grained quartz, some fine-gra	me angula	1			112.0		7	
			limestone, little angular shell, trace		47	16		1	SPT Sa	mpler 10	21
			phosphale, light brown (SP-SM)		13			-13.6		- 11	21
						3					
					47	17			SPT Sa		21
-15.0	27.0	:111	SAND with mostly line around a	Lingte Iron	-			-15.0		1.1	-
			SAND, sity, mostly fine-grained quantum fine-grained shell, trace of	lay trace	21/2				007.0	8	
1			phosphate, light gray (SM)		73	18		10260	SPT Sai	mpler 9	18
-						-		-16.6		7	-
1		Ш	At El -17 0 Ft., little clay		87	19			SPT Sai		1
1	3					100		-18.0	5.1.1.530	6	12
1								10.0		8	
		Ш			87	20			SPT Sar	mpler 5	
19.6	31.5	Ш						-19.6		5	10
			SAND, poorly-graded with silt, mo- fine-grained quartz, little shell, few	a section of the bearing	11	5				8	
E	c iv		brown (SP-SM)	ami ngrit	73	21			SPT Sar	mpler 8	20
1		:4						-21 D		12	
1										10	
1	1				73	22			SPT Sar	The same of the sa	24
-22.6	34.5	:11	SAND, poorly-graded, mostly fine	to				-22.6	printer 2	13	
-			on the poorly-graded, mostly fine	(M)	75	23	4		SPT Sar	npler 11	

DR	ILLING	LO	G (Cont. Sheet)	STALLATH						SHEET 3	
PROJEC				Jackson	_	_	_	um i	IORIZONTAL	OF 10 S	HEETS
		des A	gricultural Area Reservoirs	State Pla					NAD83	NAVD88	
	ON COORD		S-1	EVATION	TOP	OF B	ORIN	G.			
X = 1	736,775	_	75,528	12:0 Ft	-		_	_		_	- 10
ELEV.	DEPTH	LEGEND	CLASSIFICATION OF MATERIALS	R	EC.	BOX OF	ROD OR UD		REMARKS	BLOWS 0.5 FT.	N-VALUE
			medium-grained quartz, some angular i medium-grained shell, few phosphate, t silt, light brown to light gray (SP)	fine to trace	75	23		15.0	SPT Sample	er 11	- 22
	-	100	At El -24.0 Ft., trace shell, trace of shell	11	-			-24.0		6	+
3	E	133	7		93	24			SPT Sample	_	-
	-			1	92	24		56.0	or i sample	8	15
	В	100		-				-25.6		3	-
1	-	100		1.	37	25			SPT Sample		1
	-	100		- 1	31	25		43.9	SF1 Sample	5	10
- 1	-	133		+	-	-		-27.0		4	+
1 13		133		- 1	37	26			SPT Sample	_	-
- 4	-	133		- 1	"	20			or r sample	7	13
- 4	E			1	-			-28.6		5	-
		133			73	27			SPT Sample		1
- 9				- 1		-		20.0	Si i Suripi	7	12
	-	133	At El. 30.0 Ft., trace fine gravel-sized s	shell	-			-30:0		5	
		32	Action to the property of the party of the p		50	28			SPT Sample		
		3.5		- 1	-			20.0	on i dumpi	7	13
		133		1	+			-31.6		7	
		123		1.	13	29	Ш	-	SPT Sample		1
		433		- 1		20		22.0	Or 1 Stamps	7	11
		200		-	7			-33.0		10	1
		1.3		1,	30	30		7-9	SPT Sample	_	1
		1.00						24.0		8	16
1			At El +34.6 Ft., trace fine gravel-sized		1	-		-34.6		10	1
		100	limestone		13	31			SPT Sample	_	1
1		333	8			-		-36.0	200200	11	19
-36.6	48.5	1			1			-50,0		9	
30.0	40.5	1	Limestone, hard, fine-grained, frace of s		3	32			SPT Sample	er 29	
1			few fine grained sand, trace of clay, gra-	y		7		-37.6		34	63
1	9			1			1	301.0		14	
1		H			3	33			SPT Sample	er 6	1
-39.0	51.0	17						39.0		9	15
40.0	-		SAND, poorly-graded, mostly fine-graine	ed	1	-		ww.s		4	
1			shell trace coarse gravel-sized phosphatrace clay, gray (SP)	ate,	10	34			SPT Sample	er 5	1
1	7	12.	400-1-00-00-00-00-00-00-00-00-00-00-00-00			di		40.6		- 6	111
1		1	From El -40.6 to -45.0 Ft., mostly media		1	38.		40.0		11	
1	1		coarse-grained shell, trace fine gravel-s. shell, trace clay, light brown		3	35			SPT Sample	_	1
1						E		-42.0	- Caro Give	26	45
1		3				13		10.0	1023	17	1
1		-		- 6	7	36			SPT Sample	20	1

DR	ILLING	LOC	G (Cont. Sheet)	100000	LATION	83			gnation CP02-E	OF 10 SE	
PROJEC	eT .			_	NATE S			UM	HORIZONTAL	VERTICAL	
_			gricultural Area Reservoirs		e Plane.				NAD83	NAVD88	
	736,775			100000000000000000000000000000000000000	ION TO	OF E	ORIN	G			
ELEV.	DEPTH	LEGEND	CLASSIFICATION OF MATERI	12.0	REC	BOX OR SAMPLE	ROD OR UD		REMARK	SE ONS	H-VALUE
3		13			1	_			-		39
					87	38		-43.6	SPT Sam		-
					87	37			SPT Sam		41
-45.0	57.0	123	SAND, poorly-graded with clay, m	nstiv	-	_		-45.0		22	-
			medium-grained sand, little clay, III	ttle	100			400		12	-
	-		angular shell, trace fine gravel-size gray (SP-SC)	ed shell	100	38		100	SPT Sam	-	29
		1	2.37		-	-		-46.6		13	-
			ALEI -47.0 Ft., few shell, trace cla	W	100			100	2800	14	
			ALC: 47 OF J. Tew Shell, trace Off	y	60	39			SPT Sam	-	- 36
		: 3				-		-48.0		19	1
	0					15	111	0		10	-
					93	40		1	SPT Sam	-	28
49.6	61.5		400					-49.6		14	-
			SAND, clayey, mostly fine to medium-grained sand, some clay,	little fine						10	1
1			gravel-sized shell, gray (SC)		87	41			SPT Sam	pler 19	- 50
								-51.0		31	
						7-3				10	
					.93	42			SPT Sam	pler 13	25
	8 1		At Et52.6 Ft., some shell, trace of of clay					-52.6		12	-20
1.0				lay, lens	ay, lens					13	
- 4	9 8		or day		73	43	M I	(1)	SPT Sam	pler 13	700
) E.			-54.0		20	- 33
								7.		10	
					100	44	1 1	M	SPT Sam	pler 11	1
-55.B	67.5							-55.6		- 11	22
	-		SAND, poorly-graded with clay, mo	ostly shell				55.0		10	
			(SP-SC)		67	45			SPT Sam	pier 12	1
					12.3			-57.0		16	28
	-						1	51.4		15	
					93	46			SPT Sam		1
1								-58.6	30.350	12	26
	3							430.0		- 11	
1	3	: 3			53	47			SPT Sam		1
1		1.8				100		- AP - A	A. 1 3011	16	28
1	- 1	3						-60.0		18	+
-	: A	: 3			69	48			SPT Sam	_	1
1	-				67	100			SF (Sam	19	- 33
-		. 3	At El61 6 Ft. mostly snell		-	-		-61.5	-		-
-	5		a seed States of the sort street		100	100			ant a	6	-
F					67	49		-	SPT Sam		27
	ORM 183	170			_	1	-	-63.0	(Continu	15	

DR	ILLING	LOC	G (Cont. Sheet)	INSTALLA			Sorin			SHEET 5	
PROJEC			(comments)	COORDINA	_	_	_	DM .	HORIZONTAL	OF 10 SH	EETS
		des A	gricultural Area Reservoirs	State P				7.00	NAD83	NAVD88	
	ON COORD			ELEVATION		OF B	ORIN	G			
ELEV.	736,775 DEPTH	LEGEND	75,528 CLASSIFICATION OF MATERIA	12,0 Ft	PEC.	BOX OR SAMPLE	ROD		BEMARK	BLOWS	N-VALUE
						1200		_		20	ż
-63.8	75.7		Limestone, fine-grained, trace of cla of phosphate, gray	ay, trace	93	50		-64.6	SPT Samp	-	36
		喜			53	51		-04.0	SPT Samp	14	33
-66.0	78.0	島	2.00					-66.0		17	-
		13.53	SAND, poorly-graded, mostly fine to medium-grained quartz, trace sand trace shell, light gray (SP)	stone.	67	52		49.0	SPT Samp	10 40 45	85
					67	53		-67.6	SPT Samp	26	81
	E .							-69.0		49	01
					100	54			SPT Samp	_	122
1					H	_		-70.6		57	
					87	55		-72.0	SPT Samp	10 14 20	34
					93	56			SPT Samp	-	57
					53	57		-73.6	SPT Same	29 14 bler 17	
					00			-75.0	or i dann	- 10	
					87	59			SPT Samp		47
					-			-76.6		29	-
					87	60		-78.0	SPT Samp	der 34 42	76
					73	61			SPT Samp	9 oler 18	38
								-79.6		20	-
-80.6	92.5	18/0	From Et79.6 to -80.6 Ft., mostly coarse-grained quartz, trace phospi trace shell, trace sandstone, light gi SAND, clayey, mostly medium to	hate. ray	93	62			SPT Samp		16
			coarse-grained sand, some clay, tra phosphate, trace shell, gray (SC)	ace	100	63		-81.0	SPT Samp	6 5 oler 7	
			At El82.0 Ft., trace sand At El82.6 Ft., little shell, little lime:	stone		64		82.6	SPT Samp	12	19
	ORM 18		At El82.6 Ft., little shell, little lime:	stone	100	64		-	SPT Samp	ilei 3	

DRI	LLING	LOG	(Cont. Sheet)	INSTALLA					ation CP02-EAA	SHEET 5				
PROJEC			New york of the second	COORDINA	_	_		UM \$	HORIZONTAL	VERTICAL.	SE 18			
			ricultural Area Reservoirs	State F	lane,	FLE			NAD83	NAVD88				
	ON COORD 736,775	MODEL STATE		12.0 F		OF B	ORIN	9						
ELEV.	рертн	LEGEND	CLASSIFICATION OF MATER		REG.	BOX OR SAMPLE	RQD OR UO		REMARKS	BLOWS 0.5 FT.	N-VALUE			
					100	64		70-	SPT Sample	9	16			
	decad		-At El -84 0 Ft., trace sand		100	65		-84.0	SPT Sample	10 B	17			
			-At EI -88.0 Ft. trace limestone		87	66		-85,6	SPT Sample	9 18 21	47			
			At El87.0 Ft., some shell		67	64	H	-87.0	DOT Cample	26 12	47			
-88.6	100.5		SAND, clayey, mostly fine to coan sand, some clay, few shell, trice p	se-grained	67	67		-88.6	SPT Sample	22 29 14	51			
-90.0	102.0		gray (SC) Sandstone, fine-grained, few shell		100	68		-90.0	SPT Sample	14	23			
			clay, trace of phosphale, gray	, dave or	67	69		-91.6	SPT Sample	5 16 20	36			
					100	70		-93.0	SPT Sample	16	33			
								87	71			SPT Sample	9 17	36
					67	72		-94.6	SPT Sample	_	49			
					73	73		-96.0	SPT Sample	30 19 19				
			ALEI98 0 F1_ few day		93	74		-97.6	SPT Sample	23 24 21	42			
								-99.0		22	43			
					93	75		-100.6	SPT Sample	19 18	43			
					93	76		-102.0	SPT Sample	15	35			
	ORM 183					77			SPT Sample	16				

DRI	LLING	LOG (Cont. Sheet)	INSTALL	onville				gnation CP02-EAA	OF 10 SE	
PROJEC				COORDIN	_		_	UM -	HORIZONTAL	VERTICAL	
			Illural Area Reservoirs		Plane.	_			NAD83	NAVD88	
	ON COORD			ELEVATE		OF B	DRIN	G			
Χ=	36,775	Y = 775,5	20	12.0 F	1	-w				э.	-
ELEV.	DEPTH	LEGEND	CLASSIFICATION OF MATE	ERIALS	REC	BOX OR SAMPLE	RQD OR UD		REMARKS	BLOWS.	H-VALUE
					87	77		-103.6	SPT Sample	17	33
	-									15	7
		4			87	78			SPT Sample	17	31
- 7								-105.0		14	9.1
					July 1					12	
					93	79		(4)	SPT Sample	10	21
								-106.6		11	-
					1	T				13	
					100	80			SPT Sample	_	33
								-108.0		16	-
					1	11				12	
		-C, T			93	81			SPT Sample	_	32
		1						-109.6		16	
		闊				S				17	
					87	82			SPT Sample	_	39
	2	翻						-111.0		17	1
					15					12	-
					93	83			SPT Sample		32
- 1					-			-112.6		16	-
1					120	27			2222-70	14	-
					73	84			SPT Sample		41
- 1	- 3				-			-114.0		19	
1										15	1
1					87	85			SPT Sample		27
-		===			-			-115.6		13	-
- 1					02	00		1	COT Camela	14	
- E					93	86		200.0	SPT Sample	13	26
- E					-	-		-117.0		13	-
1					73	87			SPT Sample	_	1
1					1.9	01		10000	Se i Sample	10	24
E					-	-	7	-118.6		9	-
E					87	88	1		SPT Sample	-	1
ł					31	50		400 =	an i dumple	15	25
E	0				-			-120.0		26	-
ŀ					80	89			SPT Sample	_	1
- E	-				30	1/0		454.5	ar I destrine	16	31
t					-	-	1	-121.6		23	
t					87	90			SPT Sample	-	1
t					3,	20		100.0	Si , Sample	18	42
ALEC	ORM 183	25 A			_		-	-123,0	(Continued		_

DR	LLING	LOC	(Cont. Sheet)	Jacksonvi		B				SHEET OF 10	SHEET
PROJEC	7			COORDINATE	-			UM	HORIZONTAL	VERTICAL	971221
			gricultural Area Reservoirs	State Plan	_				NAD83	: NAVD88	
	ON COORD			ELEVATION T	09	OF B	ORIN	G			
X	736,775	7	75,528	12.0 Ft.	7	www.					ш
ELEV.	DEPTH	LEGEND	CLASSIFICATION OF MATE	HALS RE	Ġ.	BOX OR	200		REMAR	KS OF	M-VALUE
				6	0	91			SPT Sar	npler 10	
					+			-124.6		17	-
				9	3	92		400.0	SPT Sar		43
	Trace.			8		93	1	-126.0	SPT Sat	25	
				l.		24		-127.6	3F) 5B	20	48
				7	3	94		COL	SPT Sar		3 37
					1	-		129.0	OFT C	19 12 19	
				В		95		-130.6	SPT Sar	39	55
131.0	143.0		SAND, poorly-graded, mostly fine medium-grained quartz, trace pho-	s to osphate,	0	96			SPT Sar	31 npler 37	61
			light gray (SP)		+			-132.0		17	-
				10	0	97		-133.6	SPT Sar	npler 21	38
				10	0	98			SPT Sar	npler 16	-
				+	+	-		-135.0	_	120)
				10	a	99		-136.6	SPT Sai	npler 25	- 52
				10	0	100		155.0	SPT Sar	12	1
					1		ļ,	-138.0		16	-
				9	3	101			SPT Sar		34
					1	1	H	-139.6		10)
				10	0	102		-141.0	SPT Sar	npler 16	38
				6	-	103			SPT Sar	15 npler 26 26	51
142.6	154.5		Sandstone, fine-grained, some qu	iartz sand.	1	104		-142.6	SPT San		-
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South Florida Water Management District EAA Reservoir A-1 Geotechnical Data Report

March, 2006

EVERGLADES AGRICULTURAL AREA RESERVOIR A-1 GEOTECHNICAL DATA REPORT

MARCH 17, 2006

Rindral M. Vieth



March 2006

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1.0

INTRODUCTION

South Florida Water Management District EAA Reservoir A-1 Geotechnical Data Report

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1.0 INTRODUCTION

1.1 AUTHORIZATION FOR GEOTECHNICAL DATA REPORT (GDR)

This geotechnical data report (GDR) and the geotechnical investigations it documents were authorized by the South Florida Water Management District (SFWMD) under Work Order No. 9 (CN040932-WO09) approved on May 12, 2005.

1.2 PURPOSE AND SCOPE OF GDR

The purpose of the GDR is to present the results of geotechnical field investigations and laboratory testing performed for the Everglades Agricultural Area (EAA) Reservoir A1 under Work Order No. 2, Test (Embankment) Cells, and Work Order No. 9, Supplemental Geotechnical Investigation.

The Test Cell geotechnical investigation was performed to provide information for design of the Test Cell construction and seepage monitoring program.

The supplemental geotechnical field investigation program was developed to provide a more complete characterization of the subsurface conditions for embankment design, embankment stability, settlement, seepage analyses, and to provide information for identifying potential borrow materials. The program was developed considering the results of the previous preliminary geotechnical investigations performed to evaluate the suitability of the EAA Reservoir A-1 Project site, the Test Cell embankment construction results, and requirements for on-site borrow materials.

The locations of borings previously performed were considered when locating the borings for this supplemental program. The previous geotechnical investigations were performed by Williams Earth Sciences, Inc. (separate reports dated June 11 and July 30, 2004) and by Nodarse & Associates (March-May 2004). This information is available upon request to the SFWMD.

Borings CPO5-EAARS-CB-0418 and CPO5-EAARS-CB-0419 were not accessible to the drill rigs. Blank boring logs were prepared for these borings and are included in Appendix 2.

The borings for both the Test Cell investigation and the supplemental investigation were assigned identification numbers using the numbering system developed jointly by the US Army Corps of Engineers (USACE) and the SFWMD. The boring numbers assigned were based on the block of numbers provided by Karen Pitchford of the USACE Jacksonville District office.

1.3 PROJECT DESCRIPTION

The EAA Reservoir A-1 Project (Project) is a feature of the Comprehensive Everglades Restoration Plan (CERP). The plan selected for the expedited EAA Reservoir A-1 design includes the following components:

 Approximately 190,000 acre-feet EAA Reservoir A-1 with a perimeter embankment and seepage canals.

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- Northeast pump station that pumps from North New River Canal (3,600 cfs) this
 work item is included in Work Order No. 15
- A connector canal from the North New River Canal (NNRC) to the new northeast pump station
- Gated inlet and discharge structures this work item is included in Work Order No.
 15
- Seepage pump station this work item is included in Work Order No. 15
- New four lane bridge on U.S. Highway 27 across the new connector canal this work item is included in Work Order No. 15

The Project is located in Palm Beach County, Florida.

The purpose of the Project as defined in the CERP is to capture EAA basin runoff and releases from Lake Okeechobee. The facilities will be designed to improve the timing of environmental water supply deliveries to Stormwater Treatment Area 3/4 (STA-3/4) and the Water Conservation Areas (WCA), reduce Lake Okeechobee regulatory releases to the estuaries, meet supplemental agricultural irrigation deliveries, and increase flood protection within the EAA.

1.4 ORGANIZATION OF GDR

The remainder of this report is divided into three sections: Regional Geologic Setting, Field Exploration Program Summary, and Exploration Results. Regional Geologic Setting is a summary of information available on the Project geology and geologic conditions available in literature. Field Exploration Summary describes the field investigations and procedures and the laboratory testing completed on samples obtained during the investigations. The results of the investigation are described and a summary of the laboratory testing are contained in the Exploration Results Section. Boring logs and piezometer installation logs for the Test Cells and boring logs and piezometer installation logs for the supplemental borings are included in Appendix 1 and Appendix 2, respectively. Photographs of rock core and site photographs are found in Appendix 3. The hydraulic interval test results are included in Appendix 4. The detailed laboratory testing results are contained in Appendix 5.

1.5 LIMITATIONS

The data in this report were based on site conditions existing at the time of the investigations. Unanticipated conditions may be encountered during construction because of variations which were not detected during the investigation program. The construction process may also alter ground conditions. Therefore, experienced geotechnical engineering personnel were required to observe and document the conditions encountered and determine applicability of data.

This report was prepared solely for the benefit of SFWMD by Back & Veatch Corporation (B&V) under the terms and conditions of the written agreement dated July 9, 2004 between SFWMD and B&V ("the Agreement"). Neither SFWMD nor B&V have made analysis, verified, or rendered an independent judgment of the validity of the information provided by others. WHILE IT IS BELIEVED THAT THE INFORMATION AND DATA CONTAINED HEREIN

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WILL BE RELIABLE UNDER THE CONDITIONS AND SUBJECT TO THE LIMITATIONS SET FORTH HEREIN, SFWMD AND B&V DO NOT GUARANTEE THE ACCURACY THEREOF. EXCEPT AS OTHERWISE ALLOWED BY THE AGREEMENT, THIS REPORT MAY NOT BE USED BY ANYONE WITHOUT THE EXPRESS WRITTEN AUTHORIZATION OF B&V, AND SUCH USE SHALL CONSTITUTE AGREEMENT BY THE USER THAT IT'S RIGHTS, IF ANY, ARISING FROM THIS REPORT SHALL BE SUBJECT TO THE TERMS OF THE B&V AUTHORIZATION, AND IN NO EVENT SHALL USER'S RIGHTS, IF ANY, EXCEED THOSE OF SFWMD UNDER THE AGREEMENT.

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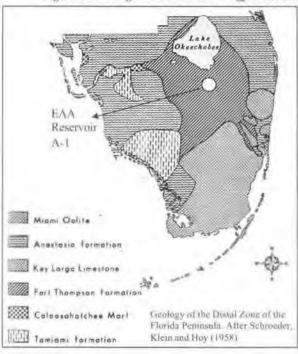
2.0 REGIONAL GEOLOGIC SETTING

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2.0 REGIONAL GEOLOGIC SETTING

Figure 2-1 Regional Surficial Geology of the Southern Florida Peninsula



The following description of the regional geologic setting was developed from a review of selective geologic literature. The EAA Reservoir A-1 Project is located south of Lake Okcechobee within Everglades physiographic subdivision of the Southern Zone (White, 1970). The Everglades is generally a flat, geologic depression between the Immokalee Rise and Big Cypress Spur physiographic subdivisions on the west, and the Atlantic Coastal Ridge physiographic subdivision on the east. The Everglades extends southward from Okeechobee to Florida Bay with elevations near sea level With the exception of the EAA, the Everglades landscape consists primarily of sawgrass marsh with hammocks of willow,

myrtle, and bay trees.

The United States Department of Agriculture, Natural Resources Conservation Service (NRCS and formerly known as the Soil Conservation Service) published a soil survey for the Palm Beach County area in the mid 1970s (McCollum et. al., 1978). Seven primary soil types were identified in the EAA region as Torry muck. Terra Ceia muck, Pahokee muck, Lauderhill muck, Dania muck, Okeelanta muck, and Okeechobee muck. The soils at EAA Reservoir A-1 include the Pahokee muck (primarily in the southern portion of the site) and Lauderhill muck (primarily in the northern portion of the site). Based on geotechnical borings performed at the EAA Reservoir A-1 Project site, the muck ranges in thickness from less than one foot to approximately five feet.

According to the NRCS, the soils located beneath the former Talisman Sugar Corporation processing facility are classified as Urban land. Urban land soils are those which have been disturbed due to development.

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The generalized regional geologic/hydrogeologic conditions for the surficial aquifer system in Palm Beach County are provided in Figure 2-1. It should be recognized that this representation is not all inclusive since the geology in southeast Florida is very complex, particularly the geology of the Plio-Peistocene to Holocene Epochs. However, the primary geologic and hydrogeologic units that are formally recognized in Palm Beach County are represented.

In general, the surface and near surface geology of the region is complex and ranges from unconsolidated, variably calcareous and fossiliferous quartz sands to well indurated, andy, fossiliferous fresh and marine limestones (Scott, 2001 and Schroeder et al., 1954). These sediments are Pleistocene to recent in age, and blanket most of Palm Beach County except for the Atlantic Coastal Ridge sediments on the east coast. The regional near surface geologic units are generally referred to, in descending order, as the Lake Flirt Marl, Fort Thompson Formation, and Caloosahatchee Formation. The total thickness of these units can range to nearly 50 feet.

The Pliocene-age Tamiami Formation underlies the Caloosahatchee Formation. The Tamiami Formation contains a wide range of mixed carbonate-siliciclastic lithologies and associated faunas (Missimer, 1992). The Tamiami Formation in the area is over 100 feet thick. The Tamiami Formation and overlying geologic units comprise the surficial aquifer system in Palm Beach County. Miller (Wesley, 1987) contoured the bottom of the surficial aquifer system (the top of the Hawthorn Group) in Palm Beach County using existing well logs. According to this work the bottom of the surficial aquifer system in the area of the Test Cell Program and the EAA Reservoir A-1 lies between about -200 to -220 feet.

Other geologic information may indicate that the Caloosahatchee Formation is thin, patchy, or not present at the EAA Reservoir A-1 Project site (Harvey et al., 2002). Also, as illustrated in Figure 2-2, recent geological work (Reese and Cunningham, 2000) has redefined the stratigraphy of the area. Presently, the Tamiami Formation has several recognized named and unnamed geologic members including the Ochopee Limestone Member and the Pinecrest Sand Member. Both Tamiami Formation members contain sandy strata, but the Pinecrest Sand Member is principally shelly, fine grained, quartz sand. The sands in the Caloosahatchee and Tamiami Formations are generally differentiated based on the fossil assemblages observed in outcrops, but key indicator fossils are typically not recovered in borings (Scott, 2005). Therefore, interpretation of the contact between the Caloosahatchee Formation and Tamiami Formation at the EAA Reservoir A-1 Project site is not possible. They will not be differentiated on the boring logs but will be designated the Caloosahatchee and Pinecrest sands.

An unnamed sand formation and the Hawthorn Group, both of Miocene-age, underlie the Tamiami Formation (Reese and Cunningham, 2002). The unnamed sand is thin in the project area, 25 to 30 feet thick and consist of very fine sand and silty sand. The Hawthorn Group consists of an interbedded sequence of widely varying lithologies and components that includes limestone, dolomite, dolosilt, shell, quartz sand, clay, phosphate grains and mixtures of these materials (Reese and Memberg, 2000). The characteristics that distinguish the Hawthorn Group from underlying units are its high and variable siliciclastic and phosphatic content; its color, which can be green, olive-gray, or light gray; and its gamma-ray log response. According to Scott (1988), the Hawthorn Group is approximately 700 feet thick in the region. The Hawthorn Group sediments retard the exchange of groundwater between the overlying surficial aquifer system and the underlying Eogene-age carbonates of the Floridan aquifer system, and are hydrogeologically referred to as the intermediate contining unit.

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Eocene-age carbonates underlying the Hawthorn Group include, in descending order, the Ocala Limestone, Avon Park Formation, and Oldsmar Formation. The overlying Oligocene-age Suwannee Limestone is thin to discontinuous in the EAA region, and likely not present in the east half of Palm Beach County (Miller, James, 1986). The cumulative thickness of the Eocene-age carbonates in the region is approximately 2,500 feet (Miller, James, 1986).

Figure 2-2 Generalized Regional Geology and Hydrogeology

(from Reese and Cunningham, 2000)

Series	Li thostrafigraphic units	Approximate thickness (feet)	Litho/ogy	Hydrogeologic unit	Approximate thickness (feet)
HOLOCENE	LAKE ILIRT MARL. UNDIFFERENTIATED SOLAND SAND	0-5	Marl, peat organic soil, quartz sand		
	PAMLICO DE SAND	0 - 50	Quartz sand	WATER TABLE	
	MIAMI LIMESTONE	0 = 30	Oplitic limestone	AQUIFER	0-120
PLEIGTOCENE	FORT OF	E-100	Marine limestone and minor gastropod-rich treshwater limestone	BISCAYNE	
	ANASTASIA P	0~140	Coquina, quartz sand and sandy limestone	AQUIFER	
	KEY LARGO LIMESTONE	0 - 20	Coralline reef rock	JIPER	
	PINECREST SAND MEMBER	0-90	Quartz sand, pelecypod-rich quartz sandstone, terrigenous mudstone	UPPER SEMICONFINING TO CONFINING UNIT	0-130
PLIOGENE	OCHOPEE LIMESTONE MEMBER	0-130	Pelecypod lime rudatore and floatatone, pelecypod-rich quartz sand, moldic quartz sandatore	GRAY UMESTONE AQUIFER	U-130
			Malaic pale bypad man guarte sand or sand storie	COWER SEMICOMPINION CHIT	0-20
Wast S	FORMATION	u - 300	Quartz sand, sandstone, and pelecypod-rich quartz sand, local abundarif phosphate grains	SAND AQUIFER(S)	0-100
MIDCENE	PEACE RIVER FORMATION	0-300	Clay-nch quartz sand, terrigenous mudstone, diatomaccous mudstone, local abundant phosphate grains	INTERMEDIATE CONFINING UNIT OR INTERMEDIATE AQUIFER SYSTEM	300±

Figure 4. Lithostratigraphic units recognized in the study area, their generalized geology, and relationship with hydrogeologic units Modified from Clason (1964), Hunter (1969), Miller (1990), Missimer (1992), and Weedman and others (1999).

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2.3

3.0 FIELD EXPLORATION PROGRAM SUMMARY

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3.0 FIELD EXPLORATION PROGRAM SUMMARY

Plate 1 shows the exploratory borings that have been completed and logged as part of this and previous phases of exploration within the perimeter of the EAA Reservoir A-1, with the exception of the Test Cell borings. Plate 2 shows borings completed and piezometers installed for the Test Cell program. Plate 3 contains the location of hydraulic interval tests. Appendix 1 and Appendix 2 contain the boring bgs of borings completed during the Test Cell Program and the Supplemental Geotechnical Investigation, respectively.

3.1 PREVIOUS EXPLORATION PROGRAMS

Soil borings from 50 to 100 feet deep were completed at the planned EAA Reservoir A-1 Project Test Cell site in December 2004 and during the test cell construction in early 2005. The boring location plan is shown in Plate 1. The boring logs for the Test Cell are included in Appendix 1.

One hundred thirty-eight geotechnical borings were completed for the SFWMD around the planned EAA Reservoir A-I in 2003 and early 2004. Four of those borings are located in the vicinity of the Test Cell site: CB-0068, CB-0069, CB-0140, and CB-0142. Boring CB-0068 is about 800 feet northwest of the Test Cell site borrow area. Boring CB-0069 is located over 1,000 feet west of Test Cell 1. Boring CB-0140 is located about 800 feet east of Test Cell 2. Boring CB-0142 is located about 200 feet east of the borrow area and 1,500 feet north of the Test Cells. The borings were completed between 50,5 and 52 feet deep with rotary wash drilling and split-barrel sampling.

3.2 EXPLORATION PROGRAM FOR DESIGN

Additional borings were completed between December 7, 2004 and September 14, 2005 for design of the temporary embankments for the Test Cell construction and monitoring program and preliminary design of the EAA Reservoir A-1. The boring locations and depths are shown in Table 3-1. Borings TW-0196 through TW-0254 were completed for piezometer installation during the Test Cell construction and monitoring only; they were not sampled or logged.

The Test Cell borings and the supplemental borings were assigned temporary identification numbers prior to drilling. These temporary boring numbers will be referred to as old numbers in this Report. After completion of the Test Cell and supplemental borings, a block of new boring numbers was received from the USACE Jacksonville District office. The borings logs and piezometer installation logs for the Test Cell borings and the supplemental borings contain the boring identification numbers that were assigned by the USACE. Table 3-1 lists the new boring number and the corresponding old boring number.

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Old Boring	New Boring	Depth			
Number	Number	(feet)	Northing	Easting	Location
		est Cell Bor			
BA-01	CP05-EAARS-CB-0168	50	776662.9	758833.1	Test Cell Borrow
BA-02	CP05-EAARS-CB-0169	50	776662.9	759333.1	Test Cell Borrow
BA-03	CP05-EAARS-CB-0170	50	776662.9	759833:1	Test Cell Borrow
BA-04	CP05-EAARS-CB-0171	50	776162.9	758833.1	Test Cell Borrow
BA-05	CP05-EAARS-CB-0172	50	776162.9	759333,1	Test Cell Borrow
BA-06	CP05-EAARS-CB-0173	50	776162.9	759833.1	Test Cell Borrow
BA-07	CP05-EAARS-CB-0174	50	775662.9	758833.1	Test Cell Borrow
BA-08	CP05-EAARS-CB-0175	50	775662.9	759333.1	Test Cell Borrow
BA-09	CP05-EAARS-CB-0176	50	775662.9	759833.1	Test Cell Borrow
BA-10	CP05-EAARS-CB-0177	50	775662.9	760333.1	Test Cell Borrow
TC-01	CP05-EAARS-CB-0178	50	774612.9	759154.5	Test Cell 1
TC-02	CP05-EAARS-CB-0179	50	774612.9	760243.1	Test Cell I
TC-03	CP05-EAARS-CB-0180	50	773531.5	759154.5	Test Cell 1
TC-04	CP05-EAARS-CB-0181	50	773531.5	760243.1	Test Cell I
TC-05	CP05-EAARS-CB-0182	50	774072.2	759698.8	Test Cell 1
TC-06	CP05-EAARS-CB-0183	50	774619.8	761239.5	Test Cell 2
TC-07	CP05-EAARS-CB-0184	50	774619,8	762328.1	Test Cell 2
TC-08	CP05-EAARS-CB-0185	50	773538.4	761239.5	Test Cell 2
TC-09	CP05-EAARS-CB-0186	50	773538.4	762328.1	Test Cell 2
TC-10	CP05-EAARS-CB-0187	50	774079.1	761783.8	Test Cell 2
TC1-E	CP05-EAARS-CB-0188	100	774072.0	760086,3	Test Cell 1
TC1-N	CP05-EAARS-CB-0189	100	774459.7	759698.8	Test Cell 1
TC1-W	CP05-EAARS-CB-0190	1.00	774072.2	7593113	Test Cell 1
TCI-S	CP05-EAARS-CB-0191	100	773684.7	759698.8	Fest Cell 1
TC2-E	CP05-EAARS-CB-0192	100	774079.1	762171,3	Test Cell 2
TC2-N	CP05-EAARS-CB-0193	100	774466.6	761783.8	Test Cell 2
TC2-W	CP05-EAARS-CB-0194	100	774079.1	761396.3	Test Cell 2
TC2-S	CP05-EAARS-CB-0195	100	773691.6	761783 8	Test Cell 2
PZIBGSA	CP05-EAARS-TW-0196	25	773021.5	759162.0	TC SW Background
PZIBGSB	CP05-EAARS-TW-0197	60	773031.5	759162.0	TC SW Background
PZIBGSC	CP05-EAARS-TW-0198	100	773041.5	759162.0	TC SW Background
PZ1/2BGSA	CP05-EAARS-TW-0199	25	774065.7	760739.5	TC Middle Backgroun
PZ1/2BGSB	CP05-EAARS-TW-0200	60	774075.7	760739.5	TC Middle Backgrou
PZ1/2BGSC	CP05-EAARS-TW-0201	100	774085.7	760739.5	TC Middle Background
PZ2BGSA	CP05-EAARS-TW-0202	25	775109.8	762335.6	TC NE Background
PZ2BGSB	CP05-EAARS-TW-0203	60	775119:8	762335.6	TC NE Background
PZ2BGSC	CP05-EAARS-TW-0204	100	775129.8	762335,6	TC NE Background
PZ2BGSE	CP05-EAARS-TW-0205	25	761239.5	773038-4	TC NE Background
PZ2BGSW	CP05-EAARS-TW-0206	29	762328.1	773038.4	TC NE Background
PZ1N2A	CP05-EAARS-TW-0207	25	774397.9	759697.0	TC1 Inner Bench
PZ1N2B	CP05-EAARS-TW-0208	60	7.74407.9	759697.0	TC1 Inner Bench
PZ1N2C	CP05-EAARS-TW-0209	100	774417.9	759697.0	TC1 Inner Bench
PZ1N3A	CP05-EAARS-TW-0210	25	774487.9	759697.0	TC1 Outer Bench
PZ1N3B	CP05-FAARS-TW-0211	60	774497,9	759697.0	TC1 Outer Bench
PZ1N3C	CP05-EAARS-TW-0212	100	774507.9	759697.0	TC1 Outer Bench
PZ1E2A	CP05-EAARS-TW-0213	25	774074.0	760028.1	TC1 Inner Bench
PZ1E2B	CP05-EAARS-TW-0214	60	774074.0	760038.L	TC1 Inner Bench

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Old Boring Number	New Boring Number	Depth (feet)	Northing	Easting	Location
PZTE2C	CP05-EAARS-TW-0215	100	774074.0	760048.1	TC Inner Bench
PZ1E3A	CP05-EAARS-TW-0216	25	774074.0	760118.1	TC1 Outer Bench
PZ1E3B	CP05-EAARS-TW-0217	60	774074.0	760128.1	TC1 Outer Bench
PZ1E3C	CP05-EAARS-TW-0218	100	774074.0	760138.1	TC1 Outer Bench
PZ1S2A	CP05-EAARS-TW-0219	25	773746.5	759700.6	TC1 Inner Bench
PZIS2B	CP05-EAARS-TW-0220	60	773736.5	759700.6	TC1 Inner Bench
PZ1S2C	CP05-EAARS-TW-0221	100	773726.5	759700.6	TC1 Inner Bench
PZ1S3A	CP05-EAARS-TW-0222	25	773656.5	759700.6	TC1 Outer Bench
PZ1S3B	CP05-EAARS-TW-0223	60	773646,5	759700.6	TC1 Outer Bench
PZ1S3C	CP05-EAARS-TW-0224	100	773636.5	759700.6	TC1 Outer Bench
PZ1W2A	CP05-EAARS-TW-0225	25	774070.4	759369.5	TC1 Inner Bench
PZ1W2B	CP05-EAARS-TW-0226	.60	774070.4	759359.5	TCI Inner Bench
PZ1W2C	CP05-EAARS-TW-0227	100	774070.4	759349.5	TC1 Inner Bench
PZ1W3A	CP05-EAARS-TW-0228	25	774070.4	759279.5	TC1 Outer Bench
PZ1W3B	CP05-EAARS-TW-0229	60	774070.4	759269,5	TC1 Outer Bench
PZ1W3C	CP05-EAARS-TW-0230	100	774070.4	759259.5	TC1 Outer Bench
PZ2N2A	CP05-EAARS-TW-0231	25	774414.8	761772.0	TC2 Inner Bench
PZ2N2B	CP05-EAARS-TW-0232	60	774414.8	761782.0	TC2 Inner Bench
PZ2N2C	CP05-EAARS-TW-0233	1.00	774414.8	761792,0	TC2 Inner Bench
PZ2N3A	CP05-EAARS-TW-0234	25	774514.8	761772.0	TC2 Outer Bench
PZ2N3B	CP05-EAARS-TW-0235	60	774514.8	761782,0	TC2 Outer Bench
PZ2N3C	CP05-EAARS-TW-0236	100	774514.8	761792.0	TC2 Outer Bench
PZ2E2A	CP05-EAARS-TW-0237	25	774070.9	762123.1	TC2 Inner Bench
PZ2E2B	CP05-EAARS-TW-0238	60	774080.9	762123.L	TC2 Inner Bench
PZ2E2C	CP05-EAARS-TW-0239	100	774090.9	762123.1	TC2 Inner Bench
PZ2E3A	CP05-EAARS-TW-0240	25	774070.9	762223.1	TC2 Outer Bench
PZ2E3B	CP05-EAARS-TW-0241	60	774080.9	762223.1	TC2 Outer Bench
PZ2E3C	CP05-EAARS-TW-0242	100	774090.9	762223.1	TC2 Outer Bench
PZ2S2A	CP05-EAARS-TW-0243	25	773743.4	761775.6	TC2 Inner Bench
PZ2S2B	CP05-EAARS-TW-0244	60	773743.4	761785.6	TC2 Inner Bench
PZ2S2C	CP05-EAARS-TW-0245	100	773743.4	7.61795.6	TC2 Inner Bench
PZ2S3A	CP05-EAARS-TW-0246	25	773643.4	761775.6	TC2 Outer Bench
PZ2S3B	CP05-EAARS-TW-0247	60	773643.4	761785.6	TC2 Outer Bench
PZ2S3C	CP05-EAARS-TW-0248	100	773643.4	761785.6	TC2 Outer Bench
PZ2W2A	CP05-EAARS-TW-0249	25	774067.3	774824.8	TC2 Inner Bench
PZ2W2B	CP05-EAARS-TW-0250	60	774077.3	774824.8	TC2 Inner Bench
PZ2W2C	CP05-EAARS-TW-0251	100	774087,3	774824.8	TC2 Inner Bench
PZ2W3A	CP05-EAARS-TW-0252	25	774067.3	774724.8	TC2 Outer Bench
PZ2W3B	CP05-EAARS-TW-0253	60	774077.3	774724.8	TC2 Outer Bench
PZ2W3C	CP05-EAARS-TW-0254	100	774087.3	774724.8	TC2 Outer Bench
	Sup	plemental E	Borings		
CB-0157	CP05-EAARS-CB-0255	100	781910.0	758371.0	A-1 Northwest Corner
CB-0158	CP05-EAARS-CB-0256	100	781984.0	761965.0	A-l North Side
CB-0159	CP05-EAARS-CB-0257	100	783716.0	767707.0	A-1 Northeast Corner
CB-0160	CP05-EAARS-CB-0258	100	780135.0	770586.0	A-l East Side
CB-0161	CP05-EAARS-CB-0259	100	776945.0	772723.0	A-1 East Side
CB-0162	CP05-EAARS-CB-0260	100	773586:0	775208,0	A-1 East Side
CB-0163	CP05-EAARS-CB-0261	100	770322.0	7,77070.0	A-1 East Side
CB-0165	CP05-EAARS-CB-0262	100	763552.0	781691.0	A-1 East Side
CB-0166	CP05-EAARS-CB-0263	300	759790.0	784011.0	A-1 East Side
CB-0167	CP05-EAARS-CB-0264	100	756500.0	786952.0	A-1 East Side

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Old Boring Number	New Boring Number	Depth (feet)	Northing	Easting	Location
CB-0168	CP05-EAARS-CB-0265	100	753214.0	789530.0	A-1 East Side
CB-0169	CP05-EAARS-CB-0266	100	750585.0	791221.0	A-I Southeast Corner
CB-0170	CP05-EAARS-CB-0267	100	750246.0	787701.0	A-1 South Side
CB-0171	CP05-EAARS-CB-0268	100	750548.0	784355.0	A-I South Side
CB-0172	CP05-EAARS-CB-0269	100	750220.0	780367.0	A-1 South Side
CB-0173	CP05-EAARS-CB-0270	100	750200.0	776528.0	A-1 South Side
CB-0175	CP05-EAARS-CB-0271	100	750130.0	769310.0	A-1 South Side
CB-0176	CP05-EAARS-CB-0272	100	750233.0	765878.0	A-1 South Side
CB-0177	CP05-EAARS-CB-0273	100	750080.0	762040.0	A-1 South Side
CB-0178	CP05-EAARS-CB-0274	100	750065.0	758699,0	A-1 Southwest Corne
CB-0179	CP05-EAARS-CB-0275	100	756315.0	758665.0	A-1 West Side
CB-0180	CP05-EAARS-CB-0276	100	764107.0	758486.0	A-1 West Side
CB-0181	CP05-EAARS-CB-0277	100	761543.0	760487.0	A-1 West Side
CB-0183	CP05-EAARS-CB-0278	100	768827.0	758019.0	A-1 West Side
CB-0184	CP05-EAARS-CB-0279	100	772221.0	757980,0	A-1 West Side
CB-0185	CP05-EAARS-CB-0280	100	776165.0	758181.0	A-I West Side
CB-0186	CP05-EAARS-CB-0281	100	779806.0	757877.0	A-I West Side
CB-0164	CP05-EAARS-RB-0282	240	766996.0	778268.0	A-1 East Side
CB-0174	CP05-EAARS-RB-0283	220	750072.0	773031.0	A-1 South Side
CB-0182	CP05-EAARS-RB-0284	240	764456.0	758050.0	A-1 West Side
CB-0190	CP05-EAARS-RB-0285	250	781923.0	766198.0	A-1 North Side
CB-0205	CP05-EAARS-RB-0286	220	764359.0	768550.0	A-1 Central
CB-0187	CP05-EAARS-CB-0287	31,5	783422:0	760195:0	A-I North Side
CB-0188	CP05-EAARS-CB-0288	30	782018.0	762205.0	A-1 North Side
CB-0189	CP05-EAARS-CB-0289	30	783462.0	764332.0	A-1 North Side
CB-0191	CP05-EAARS-CB-0290	40.5	782440.0	768965.0	A-1 East Side
CB-0192	CP05-EAARS-CB-0291	42.5	780905.0	770048.0	A-I East Side
CB-0193	CP05-EAARS-CB-0292	42.5	779231.0	771249.0	A-1 East Side
CB-0194	CP05-EAARS-CB-0293	42	777645.0	772355.0	A-1 East Side
CB-0195	CP05-EAARS-CB-0294	40	776025.0	773483.0	A-1 East Side
CB-0196	CP05-EAARS-CB-0295	30.5	774369.0	774657.0	A-I East Side
CB-0197	CP05-EAARS-CB-0296	30	773030.0	775594.0	A-1 East Side
CB-0198	CP05-EAARS-CB-0297	30.5	771865.0	776117.0	A-1 East Side
CB-0199	CP05-EAARS-CB-0298	30.5	769649.0	777979.0	A-I East Side
CB-0200	CP05-EAARS-CB-0299	30.5	768142.0	779025.0	A-1 East Side
CB-0201	CP05-EAARS-CB-0300	100	766330.0	780308.0	A-1 East Side
CB-0202	CP05-EAARS-CB-0301	30.5	764988.0	781256.0	A-1 East Side
CB-0203	CP05-EAARS-CB-0302	30.5	763912.0	781642.0	A-l East Side
CB-0204	CP05-EAARS-CB-0303	30.5	761612.0	783518.0	A-1 East Side
CB-0206	CP05-EAARS-CB-0304	34.1	758279.0	785951.0	A-1 East Side
CB-0207	CP0S-EAARS-CB-0305	10	778649.0	757994.0	A-I West Side
CB-0208	CP05-EAARS-CB-0306	30.5	755048.0	788246.0	A-1 East Side
CB-0209	CP05-EAARS-CB-0307	30.5	753802.0	789084.0	A-I East Side
CB-0209	CP05-EAARS-CB-0308	30.5	751534.0	790688.0	A-1 East Side
CB-0211	CP05-EAARS-CB-0309	30	750759.0	790521.0	A-I South Side
CB-0211	CP05-EAARS-CB-0310	35	750227.0	788770.0	A-1 South Side
CB-0213	CP05-EAARS-CB-0311	30	750227.0	786937.0	A-1 South Side
CB-0214	CP05-EAARS-CB-0312	36.5	750238.0	785150.0	A-I South Side
CB-0215	CP05-EAARS-CB-0313	30.3	750245.0	782576,0	A-1 South Side
CB-0215	CP05-EAARS-CB-0314	35	750216.0	781148.0	A-1 South Side
		35.5			
CB-0217	CP05-EAARS-CB-0315	.53.3	750528.0	779484.0	A+1 South Side

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Old Boring Number	New Boring Number	Depth (feet)	Northing	Easting	Location
CB-0218	CP05-EAARS-CB-0316	35.5	750184.0	777664.0	A-I South Side
CB-0219	CP05-EAARS-CB-0317	35.5	750159.0	775119.0	A-1 South Side
CB-0220	CP05-EAARS-CB-0318	30	750133.0	771127.0	A-1 South Side
CB-0221	CP05-EAARS-CB-0319	30.5	750096.0	767133.0	A-l South Side
CB-0222	CP05-EAARS-CB-0320	30.5	750098.0	764793.0	A-1 South Side
CB-0223	CP05-EAARS-CB-0321	30,5	750082.0	763010.0	A-1 South Side
CB-0224	CP05-EAARS-CB-0322	35.5	750063.0	761074.0	A-1 South Side
CB-0225	CP05-EAARS-CB-0323	35	749972.0	759269.0	A-1 South Side
CB-0226	CP05-EAARS-CB-0324	35	751817.0	758603.0	A-I West Side
CB-0227	CP05-EAARS-CB-0325	1.00	753491.0	758559.0	A-1 West Side
CB-0228	CP05-EAARS-CB-0326	35	755754.0	758535.0	A-1 West Side
CB-0229	CP05-EAARS-CB-0327	35	759750,0	758452.0	A-1 West Side
CB-0230	CP05-EAARS-CB-0328	35	765187.0	758085.0	A-I West Side
CB-0231	CP05-EAARS-CB-0329	35	766160.0	758074.0	A-1 West Side
CB-0232	CP05-EAARS-CB-0330	30	767982.0	758285.0	A-I West Side
CB-0233	CP05-EAARS-CB-0331	30	770143.0	758448.0	A-1 West Side
CB-0234	CP05-EAARS-CB-0332	30	775274.0	757910.0	A-1 West Side
CB-0235	CP05-EAARS-CB-0333	30	777640.0	757885.0	A-1 West Side
CB-0237	CP05-EAARS-CB-0334	30	754301.0	763206.0	A-1 Interior
CB-0238	CP05-EAARS-CB-0335	5.9	752382.0	769861.0	A-1 Interior
CB-0239	CP05-EAARS-CB-0336	30	752409.0	777773.0	A-1 Interior
CB-0240	CP05-EAARS-CB-0337	35	752266.0	784343.0	A-1 Interior
CB-0241	CP05-EAARS-CB-0338	7.5	754254.0	760704.0	A-1 Interior
CB-0242	CP05-EAARS-CB-0339	6.3	754350.0	769805.0	A-1 Interior
CB-0243	CP05-EAARS-CB-0340	30	754387.0	777734.0	A-1 Interior
CB-0244A	CP05-EAARS-CB-0341	36.5	754265.0	784323.0	A-1 Interior
CB-0244B	CP05-EAARS-CB-0342	35	754260.0	784326.0	A-1 Interior
CB-0245	CP05-EAARS-CB-0343	35	753277.0	786955.0	A-1 Interior
CB-0246	CP05-EAARS-CB-0344	6	758902.0	761980.0	A-1 Interior
CB-0247	CP05-EAARS-CB-0345	35.5	759005.0	768493.0	A-1 Interior
CB-0248	CP05-EAARS-CB-0346	10.6	758928.0	773757.0	A-1 Interior
CB-0249	CP05-EAARS-CB-0347	12.5	759074.0	780380.0	A-1 Interior
CB-0250	CP05-EAARS-CB-0348	8.5	761558.0	761980.0	A-1 Interior
CB-0251	CP05-EAARS-CB-0349	35.5	761600.0	768479.0	A-1 Interior
CB-0252	CP05-EAARS-CB-0350	12.5	761622:0	773855.0	A-1 Interior
CB-0253	CP05-EAARS-CB-0351	12	761656.0	779502.0	A-1 Interior
CB-0254	CP05-EAARS-CB-0352	34.3	759736.0	781668.0	A-1 Interior
CB-0255	CP05-EAARS-CB-0353	30	766808.0	760677.0	A-1 Interior
CB-0256	CP05-EAARS-CB-0354	g	766942.0	765980.0	A-1 Interior
CB-0257	CP05-EAARS-CB-0355	30	766738.0	771193.0	A-I Interior
CB-0258	CP05-EAARS-CB-0356	13.5	766672.0	776448.0	A-1 Interior
CB-0259	CP05-EAARS-CB-0357	- 8	769496.0	760663.0	A-1 Interior
CB-0260	CP05-EAARS-CB-0358	14.5	769587.0	765916.0	A-1 Interior
CB-0261	CP05-EAARS-CB-0359	35	769421.0	770701.0	A-l Interior
CB-0263	CP05-EAARS-CB-0360	35	774655.0	768482.0	A-I Interior
CB-0264	CP05-EAARS-CB-0361	35	778254.0	768440.0	A-1 Interior
CB-0265	CP05-EAARS-CB-0362	30	781745.0	750432.0	A+1 Interior
CB-0266	CP05-EAARS-CB-0363	13	777293.0	771082.0	A-1 Interior
CB-0267	CP05-EAARS-CB-0364	14	274291.0	771181.0	A-1 Interior
CB-0268	CP05-EAARS-CB-0365	35	776579,0	765894.0	A-1 Interior
CB-0270	CPDS-EAARS-CB-0366	14	752268:0	760704.0	A-1 Interior

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Old Boring Number	New Boring Number	Depth (feet)	Northing	Easting	Location
CB-0271	CP05-EAARS-CB-0367	30	752295.0	763219.0	A-1 Interior
CB-0272	CP05-EAARS-CB-0368	30	753762.0	767506.0	A-1 Interior
CB-0273	CP05-EAARS-CB-0369	30	753688.0	772154.0	A-1 Interior
CB-0274	CP05-EAARS-CB-0370	10.6	753744.0	775377.0	A-1 Interior
CB-0275	CP05-EAARS-CB-0371	35	753720.0	780369.0	A-1 Interior
CB-0276	CP05-EAARS-CB-0372	35.5	757929.0	776397.0	A-1 Interior
CB-0277	CP05-EAARS-CB-0373	35.5	758102.0	771124.0	A-1 Interior
CB-0278	CP05-EAARS-CB-0374	35.5	758247.0	765845.0	A-1 Interior
CB-0279	CP05-EAARS-CB-0375	35.5	762100.0	763174.0	A-1 Interior
CB-0280	CP05-EAARS-CB-0376	35.5	758795.0	760203.0	A-1 Interior
CB-0281	CP05-EAARS-CB-0377	35.5	756384.0	773911.0	A-1 Interior
CB-0282	CP05-EAARS-CB-0378	12.5	758500.0	760561.0	A-1 Interior
CB-0283	CP05-EAARS-CB-0379	13.5	769333.0	775402.0	A-1 Interior
CB-0284	CP05-EAARS-CB-0380	12	750090.0	763879.0	A-I South Side
CB-0285	CP05-EAARS-CB-0381	10	773957.0	760070.0	A-l Interior
CB-0286	CP05-EAARS-CB-0382	15	750598.0	784000.0	A-I South Side
CB-0287	CP05-EAARS-CB-0383	11,5	773646.0	757946.0	A-1 West Side
CB-0288	CP05-EAARS-CB-0384	12	757725.0	783598.0	A-1 Interior
CB-0289	CP05-EAARS-CB-0385	12	764206.0	772656.0	A-1 Interior
CB-0290	CP05-EAARS-CB-0386	6.1	764152.0	761783.0	A-1 Interior
CB-0291	CP05-EAARS-CB-0387	12	772195.0	775012.0	A-I Interior
CB-0292	CP05-EAARS-CB-0388	10	773931.0	759315.0	A-1 Interior
CB-0293	CP05-EAARS-CB-0389	13	783437.0	761044.0	A-1 Interior
CB-0294	CP05-EAARS-CB-0390	10.5	782063.0	757971.0	A-1 Interior
CB-0295	CP05-EAARS-CB-0391	12	750115.0	768244:0	A-1 Interior
CB-0296	CP05-EAARS-CB-0392	10	774441.0	759699.0	A-1 Interior
CB-0297	CP05-EAARS-CB-0393	10	773699.0	761785.0	A-1 Interior
CB-0298	CP05-EAARS-CB-0394	12	776830.0	772805.0	A-I East Side
CB-0299	CP05-EAARS-CB-0395	40	763485.0	781629.0	A-1 East Side
CB-0300	CP05-EAARS-CB-0396	35	770101.0	777065.0	A-1 East Side
CB-0301	CP05-EAARS-CB-0397	12	757683.0	774766.0	A-I Interior
CB-0302	CP05-EAARS-CB-0398	17	750753.0	791158.0	A-1 Southeast Corner
CB-0303	CP05-EAARS-CB-0399	13	758686.0	750096.0	A-1 Southwest Corner
CB-0304	CP05-EAARS-CB-0400	35.5	764135.0	758486.0	A-I West Side
CB-0305	CP05-EAARS-CB-0401	10	779802.0	757725.0	A-I West Side
CB-0306	CP05-EAARS-CB-0402	15	750729.0	791135.0	South Side
CB-0307	CP05-EAARS-CB-0403	11.5	761502.0	760599.0	A-I West Side
CB-0308	CP05-EAARS-CB-0404	11.5	776148.0	757913.0	A-1 West Side
CB-0309	CP05-EAARS-CB-0405	35	775014.0	773698,0	A-1 East Side
CB-0310	CP05-EAARS-CB-0406	35	778285.0	765848.0	A-1 Interior
CB-0311	CP05-EAARS-CB-0407	38.5	756377.0	786638.0	A-I East Side
			6.00		A-1 West Side on Main
CB-0312	CP05-EAARS-CB-0408	16	753570.0		Canal Levee
CB-0313	CP05-EAARS-CB-0409	16	757151.0		A-1 West Side On Main Canal Levee
CB-0314	CP05-EAARS-CB-0410	153	760732.0		A-1 South Side on Manu Canal Levee
CB-0315	CPDS-EAARS-CB-0411	- 12		762679.0	A-I South Side on Main Canal Loyee
CB-0316	CP05-EAARS-CB-0412	12		766635.0	A-1 South Side on Man Canal Levee

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Old Boring Number	New Boring Number	Depth (feet)	Northing	Easting	Location
CB-0317	CP05-EAARS-CB-0413	12		770593.0	A-I South Side on Mair Canal Levee
CB-0318	CP05-EAARS-CB-0414	13	750042.0	774548.0	A-I South Side on Mair Canal Levee
CB-0319	CP05-EAARS-CB-0415	14	750050.0	778522.0	A-l South Side on Mair Canal Levee
CB-0320	CP05-EAARS-CB-0416	17		782462.0	A-1 South Side on Main Canal Levee
CB-0321	CP05-EAARS-CB-0417	11.5	750108.0	786154.0	A-1 South Side on Main Canal Levee

A-1= EAA Reservoir A-1

3.2.1 Test Cell Borings

The Test Cell program involved the design, construction, installation of instrumentation, and monitoring of seepage from two Test Cells. Each Test Cell measured 500 feet square (at the embankment centerline) and consisted of an impoundment enclosed by a zoned earthen embankment surrounded by a seepage collection canal. The Test Cell site is located within the footprint of the planned EAA Reservoir A-1. Construction of the Test Cells was completed between January 10 and April 9, 2005.

Twenty geotechnical borings, CP05-EAARS-CB-0168 to CP05-EAARS-CB-0187, were completed at the Test Cell site in December of 2004, ten at the site borrow area and five at each Test Cell for design of the cells. The borings were drilled to a depth of 50 feet, primarily by rotary wash drilling using a heavy drilling mud to support the holes. The near surface limestone (caprock) was cored in each one of the holes, and a deeper, thinner limestone was cored at about 26 feet depth in two of the borings. Soils were sampled with Standard Penetration Test (SPT) methods. Drilling began on December 7, 2004 with the mobilization of two Diedrich D-50 Turbo drilling rigs to the site and was completed on December 11, 2004.

During the Test Cell program a series of eight borings CP05-EAARS-CB-0188 to CP05-EAARS-CB-0195, were drilled to a depth of 100 feet, one on each side of each Test Cell, to aid in the placement of Test Cell piezometer sensing zones. Test Cell piezometer installation logs are shown in Appendix 1. The borings were collared in the caprock in the stripped foundation of the Test Cells. The caprock was cored in three of the borings but drilled with a tricone bit in the others. The remainder of each boring was completed by rotary wash methods with soil sampling by SPT methods. The drilling was done with a Diedrich D-50 Turbo drilling rig. The two Test Cells were drilled over different time periods and the boring sequence was selected to not interfere with the Test Cell construction. The Test Cell 2 borings were completed between February 8 and 11, 2005. Test Cell 1 borings were completed between February 23 and 28, 2005.

3.2.2 Supplemental Borings

Borings CP05-EAARS-CB-0255 to CP05-EAARS-CB-0281, CP05-EAARS-CB-0282 to CP05-EAARS-CB-0286, and CP05-EAARS-CB-0287 to CP05-EAARS-CB-0417 were drilled for the supplemental geotechnical investigation. The supplemental geotechnical investigation included 100-foot deep perimeter borings, 30-foot deep perimeter borings intermediate between the 100-foot borings, and 50-foot deep interior borings. The exploration program also included 250 feet deep borings drilled to obtain continuous samples and to perform hydraulic interval testing. The

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100-foot deep borings were generally drilled between the existing borings performed in 2003 and 2004, to achieve an equidistant spacing around the perimeter of the proposed EAA Reservoir A-1. The main purpose of the 100-foot deep borings was to investigate the stratigraphy beneath the proposed embankment and to provide data for developing seepage models. The 30-foot deep perimeter borings were placed between the 100 feet deep borings. This resulted in a spacing a perimeter boring spacing of about 900 to 1,000 feet. The interior borings were placed to fill gaps between the borings completed in 2003 and 2004. The resultant spacing of interior borings is between 2,000 and 3,000 feet. The primary purpose of the interior borings was to provide information for assessing the availability of borrow materials, especially the limestone caprock.

The five 250-feet deep rotosonic drill borings were drilled to characterize the stratigraphy and perform hydraulic testing at selected intervals. One boring was located at the approximate center of the planned EAA Reservoir A-1 and one near the middle of each side of the EAA Reservoir A-1. Piezometer installations for rotosonic drill borings are shown in Appendix 2. The exploration program began with the 100-foot perimeter borings to establish the general, overall site conditions. These were followed by the 30-foot perimeter borings, and then the interior borings. The 250-foot borings were scheduled and completed between July 25 and August 14, 2005.

The planned program was modified during drilling on the basis of the subsurface conditions discovered. Some of the planned 30-foot perimeter borings were deepened to core a limestone layer often encountered at 25 to 35 feet depth. The strength and continuity of this layer was investigated because any proposed cut-off wall would be excavated through it. Many of the interior borings were shortened when it became evident that shallow material for potential borrow below the caprock was consistent and continuous. These borings were terminated below the caprock.

Two series of borings were added to the program. Twenty-five short borings were added to check the caprock thickness at locations where previous borings had indicated thin, absent, or unusually thick caprock, or produced inconsistent data on the thickness. Ten shallow (approximately 12 to 16 feet deep) borings were also added to investigate the fill placed to construct the STA-3/4 main Supply Canal levee that is adjacent to the proposed EAA Reservoir A-1. The borings performed for the Test Cells, the piezometer borings and the borings performed during the summer of 2005 are listed in Table 3-1 with their depths and location coordinates.

The majority of the drilling was completed with standard rotary wash drilling in soil and rock coring, except the five deeper holes that were completed by rotosonic drilling. Five different drill rigs were used during the course of the investigations:

- Two Diedrich D-50 Turbo rotary drill rigs mounted on all-terrain-vehicles (ATV) with large pneumatic tires
- CME-55 rotary, truck mounted drill rig.
- CME-45B rotary, tracked vehicle (Go Track) mounted drill rig
- SRO-190 truck mounted rotosonic drill rig

Drilling began the week of June 20, 2005 with one Diedrich D-50 Turbo on an ATV. The following week the second Diedrich D-50 Turbo rig was mobilized, and the week of July 11.

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2005 the CME-55 was brought to the site. One of the Diedrich D-50 Turbo rigs was replaced by the CME-45B track mounted rig on August 30, 2005 because it was better suited to reach some of the interior holes with difficult access. The track mounted rig left the site on September 13, 2005 followed by the other rotary rigs on the following day when drilling was completed. The SRO-190 truck mounted rotosonic drill rig was on site from July 26 though August 12, 2005.

3.3 BORING LAYOUT AND SURVEYS

Boring locations and elevations for the supplemental borings were determined by Weidener Surveying and Mapping. The December Test Cell program borings were located by taping from existing surveyed points established by Weidener Surveying and Mapping. During the Test Cell construction the borings were located by taping from previously installed surveyed points established by the Test Cell contractor. The supplemental borings were originally located in the field at the planned coordinates using hand-held GPS units. The finished holes were staked for later survey. During the time period between the completion of the supplemental borings and the survey, many of the stakes were destroyed by hurricane Katrina and farming activities. The locations given on the boring logs are the surveyed location when available, or the GPS location when no survey data was available.

3.4 DRILLING PROCEDURES

Except for the five rotosonic drilled holes, the borings were advanced by a combination of rotary wash boring and coring. Coring with HQ sized core barrels was used to sample the caprock and deeper limestones in some of the rotary wash borings. Double tube, swivel type, "M" design core barrels were used to recover rock cores according to the American Society for Testing and Materials (ASTM) D2113 test procedure. Core runs were restricted to a length of five feet. When coring below the caprock, 4-inch casing was advanced down to the cored interval to prevent the hole from caving onto the core barrel. The core was placed into temporary, waxed, corrugated paper boxes and core pieces of suitable length for unconfined compressive strength were wrapped in plastic film and aluminum foil to prevent dehydration. Total core recovery and Rock Quality Designation (RQD) were measured and calculated for each coring run according to ASTM D6032.

Rock bits with heavy bentonite mud flush were used to advance the borings through soil-like materials and through intervals of limestone that were not cored. The mud was recirculated through a trough that was periodically cleaned of the retained cuttings. Occasionally, caving conditions were encountered in the borehole and casing had to be advanced through the caving interval to keep the borehole open.

The soils were sampled with split-barrel samplers using the Standard Penetration Test (SPT) method in accordance with ASTM D1586 at 2.5-foot intervals or continuously above 10 feet depth and 5-foot intervals below 10 feet depth. In two of the 100-foot borings, CB-0256 and CB-0266, continuous split barrel samples were completed for the full length of the boreholes below the caprock. The soil samples were logged according to ASTM D2488 test procedures and placed in jars for transport to the testing laboratory.

Five holes, RB-0282 through RB-0286, were completed using a rotosonic drilling rig which drives a casing and core barrel into the ground by means of high frequency resonant energy. The core barrel was advanced and then overridden by the larger duameter casing that maintains an

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open hole and prevents material from collapsing into the borehole. The nominal outside diameter of the casing was six inches and the outside diameter of the core barrel was five inches. The hole was cased continuously for the full depth and a continuous sample was recovered from the core barrel and placed into thin plastic tubes for inspection and subsequent sampling. These tubes were placed in corrugated plastic boxes. Samples were taken at 5-foot intervals from the plastic boxes and placed in 1-gallon plastic bags for shipment to the laboratory. The plastic core boxes are currently stored in a container at the SFWMD G-370 Pump Station construction trailer site.

All borings that were not used for piezometer installation were backfilled with cement/bentonite grout immediately upon completion.

3.5 PIEZOMETER INSTALLATION PROCEDURES

Standpipe piezometers were installed in three of the borings performed by the rotasonic drilling method. The piezometers were installed for long term monitoring of water levels, groundwater sampling and possibly extended aquifer performance tests in the future.

The installations comprise 3-inch diameter schedule S/40 PVC well casing and slotted screen. The slotted screen is 10 feet long and set in a sand filter. The screen has four rows of 0.010-inch slots at 3/16-inch spacing. The sand pack sensing zone is isolated with bentonite seals above and below the screen.

The installation details are shown in Table 3-2.

Boring Installation RB-0283 RB-0284 RB-0286 **Detail Depths** (Feet) Ground surface to 108 Ground surface to 68 Ground surface to 148 Upper Grout And Bentonite Seal (Feet) 108 to 121,5 68 to 81 148 to 161 Sensing Zone (Feet) 81 to 240 121,5 to 220 161 to 220 Lower Bentonite Seal (Feet) Ochopee limestone of the Ochopee limestone of the Ochopee limestone of the Aquifer Monitored Tamaimi Formation Tamaimi Formation Tamaimi Formation

Table 3-2 Piezometer Installation Details

3.6 HYDRAULIC INTERVAL TESTING PROCEDURE

A program of hydraulic interval tests was performed during the investigation over the period of July to August, 2005. These tests were carried out in the rotosonic drilled borings at intervals as the borings were drilled to final depth. A 10-foot interval was drilled for each test. However, the open hole depth was measured again after the testing, and it was often less than the drilled 10 feet, indicating that the hole had partially collapsed during the testing. The depth intervals that were tested are listed in Table 3-3 along with the corresponding static water level.

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Table 3-3 Hydraulic Interval Test Locations

Intervals	RB-0282	RB-0283	RB-0284	RB-0285	RB-0286
Drilled Depth (feet)	Depth to static water level (feet), measured post- test zone	Depth to static water level (feet), measured post-fest zone	Depth to static water level (feet), measured post-test zone	Depth to static water level (feet); measured post-test zone	Depth to static water level (feet), measured post-test zone
40-50		6.39 (40-49)	7.62 (40-46)	(40-50)	8.2 (40-52)
60-70	8.2 (60-64)				
70-80		7.44 (70-80)	7.45 (70-77)	11.68 (70-80)	8,3 (70-78,5)
80-90	8.21 (80-87)				
110-120		7.58 (150-160)	7.655 (110-118.5)	(110-120)	8,45 (110-120)
120-130	8.21 (120-130)				
150-160	7.95 (150-156)	7.67 (150-160)	7,79 (150-160)	11.44 (150-160)	8.53 (150-160)

Note: Water levels are measured from the deck of the drill rig.

Wherever possible a length of open hole was formed beneath the bottom of the casing and an electric submersible pump (2-inch diameter, 15HP Grundfos Model 15 SQ/SQE 290) was lowered into the casing and water was pumped out. There was a period of development pumping lasting up to two hours to clean up the discharge (removal of suspended material) that was followed by the hydraulic interval test. Initially the pumping was carried out at increasing discharge rates; this regime was then changed to pumping at one continuous discharge rate for the duration of the pumping phase. Water levels and discharge measurements were made throughout the pumping period. On cessation of pumping, recovery water level measurements were made.

Where the borehole would not stay open below the casing, a 10-foot length of well screen (Johnson continuous slot wire wrapped stainless steel) was lowered into the zone, and the top of the screen was sealed at the bottom of the cased length by means of a pneumatic packer. The 2- inch diameter electric submersible pump assembly was then used to pump out the water.

In two boreholes (RB-0283 and RB-0284) the pH, temperature and electric conductivity of the water discharged was monitored during the pumping phase. Readings were taken early in the pumping stage, usually within the first 20 minutes of the test. The later readings were taken prior to stopping the pump.

3.7 LABORATORY TESTING PROCEDURES

Laboratory testing was assigned for selected samples of soil and rock core from the borings. Laboratory testing was performed by Nodarse & Associates, Inc. The testing procedures assigned are identified in Table 3-4 and Table 3-5.

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Table 3-4 Laboratory Rock Testing Procedures

Rock Laboratory Test	Testing Procedure
Unconfined Compressive Strength (UCS)	ASTM D2938
Resistance to Degradation by Abrasion	ASTM C535
Sulfate Soundness	ASTM C88
Specific Gravity and Absorption	ASTM D6473

Table 3-5 Laboratory Soil Testing Procedures

Soil Laboratory Test	Testing Procedure
Grain Size Analysis	ASTM D422
Carbonate Content (CO ₃)	Florida DOT
Corrosivity	Florida DOT
Moisture Content	ASTM D2216
Hydrometer Analysis	ASTM D422

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4.0

EXPLORATION RESULTS

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4.0 EXPLORATION RESULTS

4.1 GEOLOGY

The site is generally covered by approximately one half to two feet of surficial peat/muck and marl. The marl beneath the peat and muck is known by some authors as the Lake Flirt Marl (Reese and Cunningham, 2000; Harvey et. al., 2002), but is undifferentiated from the peat and muck layer for this report. The borings completed at the Test Cell site in December 2004, during the Test Cell Program and during the supplemental investigation penetrated through the surficial peat, marl. (in some locations) road fill, and caprock, then through about 15 to 40 feet of primarily carbonate sand and limestone, and then into primarily shelly quartz sand with sparse limestone for about 25 to 60 feet.

The upper carbonate sand and limestone constitutes the Fort Thompson Formation at the site. At the top of the Fort Thompson is a hard limestone layer generally about 3.5 to 6- foot thick, locally called caprock. The caprock is underlain primarily by silty carbonate sand varying from about 18 to 42 feet depth where another hard limestone layer, generally 1.5 to 3-foot thick, is often encountered. Visual inspection of the sand samples from the borings reveals that the sand consists at least partly of shell fragments, and tends to be angular and platy. Thinner, hard limestone layers are sometimes encountered in the interval.

All the limestone layers exposed in cores from the site are very fossiliferous. The silty sand of the Fort Thompson Formation is also abundantly fossiliferous with gastropods, pelecypods, corals, and echinoderms. The caprock is white, light gray, tan and yellowish brown. The sand and lower limestone layers are white to very pale brown.

Below the Fort Thompson Formation, the shelly sand with sparse limestone constitutes the Caloosahatchee Formation and the upper member (the Pinecrest Sand) of the Tamiami Formation, which are not differentiated in this report. The deeper borings penetrated into mixed carbonate and quartz sand with carbonate predominant. The mixed sand with carbonate sand predominate is the Ochopee Limestone member of the Tamiami Formation.

The deepest borings, the rotosonic borings, passed through the mixed carbonated and quartz sand and then between 140 to 177-foot depth into very fine sand and silty sand grading to clayey sand at 191 to 200-foot depth. The very fine quartz sand and silty to clayey sand belongs to the unnamed sand formation and the top of the underlying Peace River Formation.

4.2 GROUND CONDITIONS AND LABORATORY TESTING RESULTS

The identification of the stratigraphic units below the Fort Thompson Formation in the borings is based on descriptions in Reese and Cunningham (2000). The laboratory testing results are summarized in Table 4-1 through Table 4-6. Figures 4-1 and 4-2 plot soil sample percent passing the 200 sieve and carbonate content versus depth, respectively.

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Figure 4-1 Percent Finer Than the 200 Sieve Versus Sample Depth

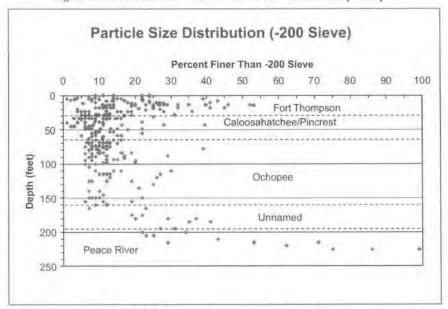
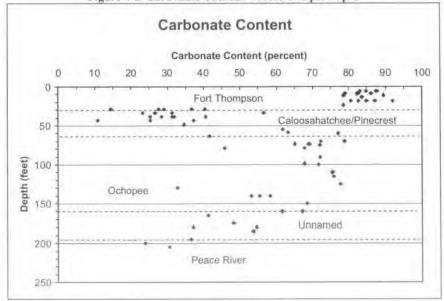


Figure 4-2 Carbonate Content Versus Sample Depth



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4.2.1 Caprock

Immediately below the site soil layer is the top of the Fort Thompson Formation, a limestone layer locally called caprock. The thickness of the caprock in the borings ranged from 0 to 9.2-foot thick, but it is most often about 3.5 to 6-foot thick. The caprock is not a uniform hard limestone. It is thinly to medium bedded with bedding thickness generally less than one foot. The beds range from dense, hard, and strong to soft and friable. The hardness, strength, and density are related to the amount of fine, carbonate cement in the limestone. The softest beds consist of poorly cemented, calcite sand grains with possible shell fragments. In other beds, the grains are cemented at the contacts, and the rock is porous but generally moderately hard and moderately strong. In the hard and strong beds the grains are completely contained in a matrix of fine grained cement.

The caprock is jointed and contains solution cavities including local areas of anastomosing channels especially near the top, and single channels up to several inches in diameter that penetrate the full thickness. The solution channels in the caprock locally contain soil including the peat and marl.

Because of the variable material quality, core recovery from the borings was generally under 50 percent. The core recovery from the caprock ranged from 0 to 100 percent with an average of 42.4 percent. The RQD ranged from 0 to 92 percent with an average of 19 percent. The combination of variable rock hardness, thin bedding, and solution cavities combined to produce the low core recovery and RQD.

Pieces of core with sufficient length were selected from the borings, wrapped to preserve them, and sent to a laboratory for unconfined compressive strength (ASTM D2938), specific gravity and absorption testing (ASTM D6473). Crushed stone produced from the caprock during the Test Cell Program was also tested for specific gravity, absorption, and abrasion resistance (ASTM C535). Larger pieces of the caprock stockpiled as riprap were sent for sulfite soundness testing (ASTM C88).

The unconfined compressive strengths ranged from 433 to 9,768 pounds per square inch (psi) with an average of 2,928 psi. The bulk specific gravity ranged from 2.62 to 1.44 with an average of 2.25. The absorption ranged from 1.5 percent to 29.5 percent with an average of 6.1 percent. The losses on abrasion testing of three samples with "A" gradation were 31.3, 31.3, and 30.6 percent. The losses on soundness testing for three samples were 0 percent.

Three larger samples of caprock excavated for riprap during the Test Cell Program were selected from a stockpile and sent to the laboratory for specific gravity and absorption testing. The bulk specific gravities determined were 2.35, 2.4, and 2.35. The corresponding absorptions were 3.02, 3.12, and 2.93 percent, respectively.

It must be stressed that boring core recoveries in the caprock averaged less than 50 percent. These test results represent the high end of the caprock quality. The softer, less dense, and weaker rock was lost during the coring process or was retrieved in pieces too small for testing. Data from the testing is presented in Table 4-1.

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Table 4-1 Caprock Laboratory Testing and Core Data

Boring Number	Run Depth (feet)	Core Recovery (percent)	RQD (percent)	Bulk Specific Gravity	Absorption (percent)	UCS (psi)
CP05-EAARS-CB-0255	1-6	22	16			
CP05-EAARS-CB-0256	1-5.5	61	35	2,41	3.3	2600
CP05-EAARS-CB-0257	4.75-9.75	76	48	2.03	7.71	1250
CP05-EAARS-CB-0257	9.75-11.75	100	92			
CP05-EAARS-CB-0258	4.5-9.5	54	22			
CP05-EAARS-CB-0259	3-8	56	24	1.44	29.5	1430
CP05-EAARS-CB-0259	8-10	55	0			
CP05-EAARS-CB-0260	7-12	48	22			9768
CP05-EAARS-CB-0261	5.5-10.5	44	32	2.32	5.39	4340
CP05-EAARS-CB-0261	5.5-10.5	44	32			9768
CP05-EAARS-CB-0262	6.5-11.5	48	30	2,43	2.39	3690
CP05-EAARS-CB-0263	8.5-13.5	20	0	34.10		
CP05-EAARS-CB-0264	6.5-11.5	62	40	2.27	4.93	1530
CP05-EAARS-CB-0265	4.5-9.6	44	0	2.0.	7.26	1
CP05-EAARS-CB-0266	1-4	17	0			
CP05-EAARS-CB-0267	6-9.5	54	53	2.32	5.35	
CP05-EAARS-CB-0268	2-7	50	24	2.00		
CP05-EAARS-CB-0269	2,5-7.5	36	28	2.52	3.1	1570
CP05-EAARS-CB-0270	3.5-8.3	26	22	2.35	3.32	1860
CP05-EAARS-CB-0271	3,5-6.5	48	45	2.62	2.1	4620
CP05-EAARS-CB-0271	6.5-10.5	25	-0.	2.02	Sec. 1	7020
CP05-EAARS-CB-0272	0.1-5.1	46	14	2.4	4.65	2650
CP05-EAARS-CB-0272	4.5-9.5	54	30	2.14	4.73	3090
CP05-EAARS-CB-0274	9.3-12	33	15	2,26	4.1	433
	18-22	63	0	2,20	46.1	+33
CP05-EAARS-CB-0274 CP05-EAARS-CB-0275	0.5-5.5	67	24	2.4	4,44	1676
	1-4.5	21	13	2,24	4.7	1070
CP05-EAARS-CB-0276	1.5-6.5	13	0	4,64	4.7	-
CP05-EAARS-CB-0277 CP05-EAARS-CB-0278	3.5-8.5	41	7			-
	4-9	35	18	2:22	6.7	1870
CP05-EAARS-CB-0279	2-7	18	18	2,66	301/	1670
CP05-EAARS-CB-0280		0	0	-		
CP05-EAARS-CB-0281	8-13	36		2	10.97	1105
CP05-EAARS-CB-0287	1-6		18		5.4	650
CP05-EAARS-CB-0288	1-6	73	52	2.33	3,4	650
CP05-EAARS-CB-0289	1-6	42	14	-	-	-
CP05-EAARS-CB-0289	5.5-10.5	68	38	_		-
CP05-EAARS-CB-0290	4-9	60	24	-		10754
CP05-EAARS-CB-0290	9-14	54	2.4	-		874
CP05-EAARS-CB-0290	6-11	60	45			-
CP05-EAARS-CB-0291	11-16	5	0	-	-	-
CP05-EAARS-CB-0292	7-12	56	19	_	-	
CP05-EAARS-CB-0293	6-8.5	80	35			-
CP05-EAARS-CB-0293	8.5-13.5	92	55		-	
CP05-EAARS-CB-0293	13.5-17.5	30	10		1	
CP05-EAARS-CB-0294	6-11	54	8			
CP05-EAARS-CB-0295	6-17	80	40			
CP05-EAARS-CB-0295	11-16	60	23			
CP05-FAARS-CB-0296	7-12	42	8			

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Table 4-1 Continued - Caprock Laboratory Testing and Core Data

Boring Number	Run Depth (feet)	Core Recovery (percent)	RQD (percent)	Bulk Specific Gravity	Absorption (percent)	UCS (psi)
CP05-EAARS-CB-0297	4.5-9.5	80	50			
CP05-EAARS-CB-0297	9.5-14.5	66	31			
CP05-EAARS-CB-0298	5.5-7.5	100	38			
CP05-EAARS-CB-0298	8-13	74	46			
CP05-EAARS-CB-0298	13-17	44	16			
CP05-EAARS-CB-0299	8-13	62	13			
CP05-EAARS-CB-0300	6-11	62	24	2,31	7.96	
CP05-EAARS-CB-0301	7-12	95	85			
CP05-EAARS-CB-0301	12-17	34	10			
CP05-EAARS-CB-0302	6-11	46	28			
CP05-EAARS-CB-0302	11-16	48	25			
CP05-EAARS-CB-0303	6-10	85	78			
CP05-EAARS-CB-0303	10-15	65	52	2,05	10.43	
CP05-EAARS-CB-0303	15-18	37	28			
CP05-EAARS-CB-0304	8-9.5	67	29			
CP05-EAARS-CB-0304	9.5-14.5	60	43			
CP05-EAARS-CB-0305	0-5	22	0			
CP05-EAARS-CB-0305	5-10	8	0			
CP05-EAARS-CB-0306	6-8.3	100	82			
CP05-EAARS-CB-0306	8.3-11	93	63	2,47	2.69	8080
CP05-EAARS-CB-0306	[1-14	60	27			
CP05-EAARS-CB-0306	14-17	42	28			
CP05-EAARS-CB-0307	5,5-10.5	90	68			
CP05-EAARS-CB-0307	10.5-15.5	78	42			
CP05-EAARS-CB-0308	5.5-10	45	30			
CP05-EAARS-CB-0308	10-15	46	8			
CP05-EAARS-CB-0309	5-10	58	32	2.46	1.83	5200
CP05-EAARS-CB-0309	11-16	22	0		1000	
CP05-EAARS-CB-0310	4.5-8.5	50	0			
CP05-EAARS-CB-0310	8.5-9.5	100	0			1
CP05-EAARS-CB-0310	10-15	20	14	1.74	8.41	271
CP05-EAARS-CB-0311	3,5-8,5	28	0	100	W.T.	
CP05-EAARS-CB-0311	8.5-13.5	14	10			
CP05-EAARS-CB-0312	4,5-8,5	38	0			
CP05-EAARS-CB-0312	8.5-13.5	8	0			
CP05-EAARS-CB-0313	1-6	26	0			
CP05-EAARS-CB-0313	6-11	24	0			
CP05-EAARS-CB-0314	4-8.5	22	0			
CP05-EAARS-CB-0314	8.8-13.5	12	0			
CP05-EAARS-CB-0316	5-10	38	30			
CP05-EAARS-CB-0317	5.5-7	83	70			
CP05-EAARS-CB-0318	5-10	50	32			
CP05-EAARS-CB-0318	3.5-8.5	45	22	1		1
CP05-EAARS-CB-0319	8.5-13.5	97	0			-
CP05-EAARS-CB-0320	5-10	45	17			
		32	21			
CP05-FAARS-CB-0320	10-14	8	- 21 - 8	-		-
CP05-EAARS-CB-0321 CP05-EAARS-CB-0322	5,5-11.5	48	27	-	_	

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Boring Number	Run Depth (feet)	Core Recovery (percent)	RQD (percent)	Bulk Specific Gravity	Absorption (percent)	UCS (psi
CP05-EAARS-CB-0323	4.9	60	38			
CP05-EAARS-CB-0323	9-14	28	20		F	
CP05-EAARS-CB-0324	4.75-9.75	70	44	2.34	4,35	
CP05-EAARS-CB-0324	9,75-14.75	1.0	0			
CPOS-EAARS-CB-0325	5.75-10.75	96	36	2,4	3.27	
CP05-EAARS-CB-0326	6.5-11.5	48	34	2,204	7.11	
CP05-EAARS-CB-0326	11,5-14	36	0		1	
CP05-EAARS-CB-0327	4-9	78	56	2.16	6.53	7
CP05-EAARS-CB-0327	9-12	26	0			
CP05-EAARS-CB-0329	2-7	46	12			186
CP05-EAARS-CB-0330	1-6	54	7			
CP05-EAARS-CB-0331	2.5-7.5	70	15	2.31	4	
CP05-EAARS-CB-0331	8-13	6	0			
CP05-EAARS-CB-0332	3-8	36-	0			
CP05-EAARS-CB-0333	3.5-8.5	20	0			
CP05-EAARS-CB-0334	2.25-7.25	20	- D			3
CP05-EAARS-CB-0335	0.9-4.9	40	0			100
CP05-EAARS-CB-0336	0.9-5.9	30	0			
CP05-EAARS-CB-0337	1.5-6.5	56	- 0			
CP05-EAARS-CB-0337	6.5-11.5	26	0		1 - 33 - 1	
CP05-EAARS-CB-0338	2.5-7.5	46	3.			
CP05-EAARS-CB-0339	1.5-6.5	18	7			
CP05-EAARS-CB-0340	1.1-6.1	24	0			
CP05-EAARS-CB-0341	1.5-6.5	46	14			
CP05-EAARS-CB-0341	6.3-115	24	18			
CP05-EAARS-CB-0342	2.7	46	22			
CP05-EAARS-CB-0343	1.9	32	- 0			
CP05-EAARS-CB-0344	0.8-5.8	34	0			
CP05-EAARS-CB-0345	4.9	20	0			
CP05-EAARS-CB-0345	9-14	10	- 0			
CP05-EAARS-CB-0346	0.1-5.1	12	- 0			
CP05-EAARS-CB-0346	5.1-10.1	40	24			
CP05-EAARS-CB-0347	2-6	85	43			
CP05-EAARS-CB-0347	6-11	42	10			
CP05-EAARS-CB-0348	1.1-6.1	40	n			
CP05-EAARS-CB-0349	4-9	40	0			
CP05-EAARS-CB-0350	3.5-8.5	34	12			
CP05-LAARS-CB-0350	7.5-12.5	8	8.			
CP05-EAARS-CB-0351	2.7	20	D.			
CP05-EAARS-CB-0351	7-12	22	8			
CP05-EAARS-CB-0352	5.5-10.5	36	15			
CP05-EAARS-CB-0353	3.7.5	56	34			
CP05-LAARS-CB-0354	2575	30	10			
CP05-EAARS-CB-0355	4585	30				
	-	94	42	_		-
CP05-EAARS-CB-0356	1-6	30				
CPOS-EAARS-CB-0356	7-12	35	15			
CP05-EAARS-CB-0357	12-6.5		- X			-
CPBS-EAARS-CB-0358	3.7 8.5-13	56	45			_

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Table 4-1 Continued - Caprock Laboratory Testing and Core Data

Boring Number	Run Depth (feet)	Core Recovery (percent)	RQD (percent)	Bulk Specific Gravity	Absorption (percent)	UCS (psi)
CP05-EAARS-CB-0359	3.5-8.5	36	10		-	
CP05-EAARS-CB-0360	3.5-8.5	3.5	- 9			
CP05-EAARS-CB-0361	1.5-6.5	50	16			
CP05-EAARS-CB-0362	3.5-7.5	12	0			
CP05-EAARS-CB-0363	3-8	68	34			
CP05-EAARS-CB-0363	8-13	14	i).			
CP05-EAARS-CB-0364	1.5-6.5	42	8			
CP05-EAARS-CB-0364	7.5-12.5	42	21			
CP05-EAARS-CB-0365	6.5-11.5	68	50			
CP05-EAARS-CH-0366	4.9	52	26			
CP05-EAARS-CB-0366	9-14	10	9			-
CP05-EAARS-CB-0367	2-7	22	7			
CP05-EAARS-CB-0367	7-12	22	0			
CP05-EAARS-CB-0368	0.25-5.25	36	10	-		
CP05-EAARS-CB-0369	2-7	28	0			
CP05-EAARS-CB-0370	0.9-5.6	7	0			
CP05-EAARS-CB-0370	5.6-10.6	24	10			
CP05-EAARS-CB-0371	1.3-6.3	42	12			
CP05-EAARS-CB-0372	4.9	65	45			
CP05-EAARS-CB-0372	10-14	20	0			
CP05-EAARS-CB-0373	4.5-9.5	80	34			
CP05-EAARS-CB-0373	9.5-14.5	26	17			-
CPOS-EAARS-CB-0374	5.5-10.5	51	26			-
CP05-EAARS-CB-0374	10.5-15.5	10	0			
CP05-EAARS-CB-0375	4595	58	28			
CP05-EAARS-CB-0376	4595	73	73			
CP05-EAARS-CB-0377	5.5-10.5	60	47			-
CP05-EAARS-CB-0377	10.5-15.5	40	20			
CP05-EAARS-CB-0378	6-11	64	35			-
CP05-EAARS-CB-0379	1.5-6.5	16	0			-
A REAL PROPERTY AND ADDRESS OF THE PARTY AND A		54	9	-		
CP05-EAARS-CB-0379	7-12					
CP05-EAARS-CB-0380	5.5-10	88	20			
CPOS-EAARS-CB-0380	10-12	38	50	-		-
CP05-EAARS-CB-0381	5-10		0			-
CP05-EAARS-CB-0381				-		-
CP05-EAARS-CB-0382	5.5-10	5	0	_		
CP05-EAARS-CB-0382	10-15	30	26	_		-
CP05-EAARS-CB-0383	1.5-6.5	38	20			-
CP05-EAARS-('B-0383	6.5-11.5	0	0			-
CP05-EAARS-CB-0384	2-7	34	10			
CP05-EAARS-CB-0384	7-12	34	0			-
CP05-EAARS-CH-0385	2-7	16	0			-
CP05-EAARS-CB-0385	7-12	2	- 0			-
CP05-EAARS-CB-0386	1.1-6.1	36	7			
CP05-EAARS-CB-0387	2-7	28	1			
CP05-EAARS-CB-0387	7-12	12	0			
CP05-EAARS CB-0388	0-5	38	13			GIF
CP03-EAARS-CB-0388	5-10	24	14			
CPRS-EAARS-CB-0389	1-76	42	1N			

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Table 4-1	Continued - Capr	ock Laborator	v Testing and	Core Data
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Boring Number	Run Depth (feet)	Core Recovery (percent)	RQD (percent)	Bulk Specific Gravity	Absorption (percent)	UCS (psi)
CP05-EAARS-CB-0389	8-13	0	0			
CP05-EAARS-CB-0390	0.5-5.5	.68	20			
CP05-EAARS-CB-0390	5.5-10.5	12	0			
CP05-EAARS-CB-0391	5.5-10.5	40	20			1
CP05-EAARS-CB-0392	0-5	48	30			1805
CP05-EAARS-CB-0392	5-10	0	0			
CP05-EAARS-CB-0393	0-5	40	22			
CP05-EAARS-CB-0393	5-10	6	0			
CP05-EAARS-CB-0394	2-7	50	15			
CP05-EAARS-CB-0394	7-12	38	15			
CP05-EAARS-CB-0395	8-13	36	10			
CP05-EAARS-CB-0395	13-18	18	13			
CP05-EAARS-CB-0396	3-8	80	28			
CP05-EAARS-CB-0397	2-7	24	8			
CP05-EAARS-CB-0397	7-12	6	0			
CP05-EAARS-CB-0398	5-7	25	0			
CP05-EAARS-CB-0398	7-12	60	45			
CP05-EAARS-CB-0398	12-17	29	19			
CP05-EAARS-CB-0399	6-9	67	19			1
CP05-EAARS-CB-0399	9-13	31	13			
CP05-EAARS-CB-0400	3-9	40	0.			
CP05-EAARS-CB-0401	5-10	10	0			
CP05-EAARS-CB-0402	5.3-10	44	32			
CP05-EAARS-CB-0402	10-15	10	0.			
CP05-EAARS-CB-0403	5-10	55	25			
CP05-EAARS-CB-0404	1.5-6.5	42	28			
CP05-EAARS-CB-0404	6.5-11.5	6	0			
CP05-EAARS-CB-0405	4-9	45	13			
CP05-EAARS-CB-0405	9-14	34	9			
CP05-EAARS-CB-0406	0-5	46	30			
CP05-EAARS-CB-0406	5-10	52	35			
CP05-EAARS-CB-0407	5,5-10,5	50	18			
CP05-EAARS-CB-0416	12-17	62	49			

RQD = Rock Quality Designation as a percentage

UCS = Unconfined Compressive Strength in pounds per square inch (psi)

4.2.2 Fort Thompson Sand

The silty sand below the caprock is composed primarily of calcite grains. The carbonate content test was determined in accordance with the Florida Department of Transportation (FDOT) test procedure. Results ranged from 76.6 percent to 91.9 percent with an average of 83.6 percent carbonate content. The grains are platy and angular and many, when viewed with a magnifier, have a fluted surface on one of the plate sides. Most if not all the sand grains appear to be shell fragments. One corrosivity test series (FDOT) was performed on sand from the Fort Thompson Formation (RB-0282, 5 to 10- foot depth):

Electrical resistivity – 6,100 Ohm-cm

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- pH 8.9
- Chlorides 90 parts per million (ppm)
- Sulfates 60 ppm.

SPT samples were assigned group symbols in accordance with ASTM D2487. Samples tested (90 tests) were mostly SM (53) with some SW-SM (12), SP-SM (6), and GP-GM (4) and with occasional GM (3), SP (3), GW (1), GW-GM (2), CL-ML (2), ML (2), and SW (1). Percent passing the 200 sieve ranged from 2 to 53 percent with an average of 19.9 percent. Moisture content (ASTM D2216) ranged from 6 percent to 63 percent with an average of 22.7 percent. Hydrometer analyses (ASTM D422) on the fines content of the two samples tested indicate them to be mostly silt with clay content of 5.8 to 8.8 percent. (Table 4-3)

The gravel content of the samples included shell fragments and limestone chips. Densities ranged from loose to very dense, Samples with high gravel content, especially limestone chips, generally correlate with high SPT blow counts. Intervals of hard drilling as judged from drilling rate, drill vibration and drill bit chatter also correlate with high SPT blow counts and limestone gravel content.

Table 4-2 Fort Thompson Formation Laboratory Soil Testing

Boring Number	Depth (feet)	Moisture (percent)	ASDM D2487 Class	-200 Sieve (percent)	Clay (percent)	CO ₃ (percent)
CP05-EAARS-CB-0168	5.5					87.4
CP05-EAARS-CB-0169	1.8.5		SP-SM	114		
CP05-EAARS-CB-0170	13,5		SM	28.1		
CP05-EAARS-CB-0170	8.5					82.6
CP05-EAARS-CB-0171	13.5		GM	20.1	5.8	
CP05-EAARS-CB-0171	6					87.7
CP05-EAARS-CB-0171	18.5					82.7
CP05-EAARS-CB-0172	8.5					86.2
CP05-EAARS-CB-0173	13.5		SM	39,8		1
CP05-EAARS-CB-0173	6					84.7
CP05-EAARS-CB-0174	6					83
CP05-EAARS-CB-0174	18.5					84.8
CP05-EAARS-CB-0175	28.5		SM	12.2		
CP05-EAARS-CB-0175	8.5					85.9
CP05-EAARS-CB-0175	13.5					83.7
CP05-EAARS-CB-0176	13.5		SM	29		
CP05-EAARS-CB-0176	23.5					78.5
CP05-EAARS-CB-0177	13.5					83.5
CP05-EAARS-CB-0179	8.3		SM	22.1		
CP05-EAARS-CB-0180	13.5		SM	26.6		
CP05-EAARS-CB-0181	6.5	-	SM	22.1	8.8	
CP05-EAARS-CB-0183	18.5		5M	13.1		
CP05-EAARS-CB-0183	8	172				
CP05-EAARS-CB-0186	23.5	26.2				
CP05-EAARS-CB-0187	18.5		SM	44.9		
CP05-EAARS-CB-0188	9					82.3
CP05-EAARS-CB-0189	5.5	15	SM	22		

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Table 4-2 Continued - Fort		Thompson			Cour restin	0
Boring Number	Depth (feet)	Moisture (percent)	ASDM D2487 Class	-200 Sieve (percent)	Clay (percent)	CO ₃ (percent
CP05-EAARS-CB-0189	13.5	21	SM	25		
CP05-EAARS-CB-0189	8.5		177			79
CP05-EAARS-CB-0190	13.5	26	SM	23		
CP05-EAARS-CB-0190	18.5					91.9
CP05-EAARS-CB-0191	13.5	21	SM	25		
CP05-EAARS-CB-0191	18.5					87.1
CP05-EAARS-CB-0192	18.5					80.6
CP05-EAARS-CB-0193	8.5		GM	13.6		
CP05-EAARS-CB-0193	28.5		GP-GM	7.3		
CP05-EAARS-CB-0193	11:		GW - GM	5.5		
CP05-EAARS-CB-0193	13.5		GW -GM	10.8		
CP05-EAARS-CB-0193	5.5		SP	3.2		
CP05-EAARS-CB-0194	11					78.6
CP05-EAARS-CB-0195	8,5		SM	43		
CP05-EAARS-CB-0195	-16					89.5
CP05-EAARS-CB-0255	9	33	SM	24		
CP05-EAARS-CB-0255	13.5	27	SM	32		
CP05-EAARS-CB-0255	18.5	19	SW-SM	11		
CP05-EAARS-CB-0256	14.5	63	GP-GM	12		
CP05-EAARS-CB-0256	21.5	21	SM	23		
CP05-EAARS-CB-0256	23.5	29	SM	19		
CP05-EAARS-CB-0266	11.5	24	SM	28		
CP05-EAARS-CB-0266	14.5	3.4	SM	14		
CP05-EAARS-CB-0266	16	36	SM	25		
CP05-EAARS-CB-0266	20.5	28	SM	27	-	
CP05-EAARS-CB-0266	5,5	13	SW-SM	8		76.6
CP05-EAARS-CB-0266	26.5	22	SW-SM	13	-	10,0
CP05-EAARS-CB-0268	18.5	22	GP-GM	9	-	
CP05-EAARS-CB-0268	13.5	30	SM	27		
	7	30	SIVI	47		81.6
CP05-EAARS-CB-0268	13.5	32	P3.4	25		01.0
CP05-EAARS-CB-0269	8.5	24	SM-SM	12		
CP05-EAARS-CB-0269	-	23		10	_	-
CP05-EAARS-CB-0269	18.5		SW-SM	35		
CP05-EAARS-CB-0270	13.5	36	SW			-
CP05-EAARS-CB-0270	18.5	26	SW-SM	11	-	
CP05-EAARS-CB-0271	10.5	24	SM	18	-	
CP05-EAARS-CB-0271	18.5	25	SM	25		
CP05-EAARS-CB-0271	23.5	22	SW-SM	12	-	0.00
CP05-EAARS-CB-0271	13.5	100	651	10		81.8
CP05-EAARS-CB-0272	5.1	19	SM	15		
CP05-EAARS-CB-0272	10	20	SM	24	-	-
CP05-EAARS-CH-0272	18.5	- 11	SW-SM	9		
CP05-EAARS-CB-0272	23.5	15	SW-SM	7		
CP05-EAARS-CB-0275	7.5	25	SM	17		
CP05-EAARS-CB-0275	13.5	18	SM	19		
CP05-EAARS-CB-0275	5.5	24	SW-SM	10		
CP05-EAARS-CB-0275	23.5	14	SW-SM	5		
CP05-EAARS-CB-0276	13.5	25	SM	13		
CP05-EAARS-CH-0276	23.5	74	SP	- 4		

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Boring Number	Depth (feet)	Moisture (percent)	ASDM D2487 Class	-200 Sieve (percent)	Clay (percent)	CO ₃ (percent)
CP05-EAARS-CB-0276	18.5	26	SP-SM	9		
CP05-EAARS-CB-0276	8	17	SW-SM	12		
CP05-EAARS-CB-0277	23.5	25	SM	16		
CP05-EAARS-CB-0278	18.5	21	SM	14		
CP05-EAARS-CB-0278	23,5	24	SM	19		
CP05-EAARS-CB-0280	7	18	GW	2		
CP05-EAARS-CB-0317	14	23	ML	52		
CP05-EAARS-CB-0326	14	23	SP-SM	11		
CP05-EAARS-CB-0329	8.5	17	SM	14		
CP05-EAARS-CB-0329	13.5	22:	SM	14		-
CP05-EAARS-CB-0329	18.5	13	SP-SM	12		
CP05-EAARS-CB-0333	8.5	24	SM	24		
CP05-EAARS-CB-0346	10	10	SP-SM	9		il.
CP05-EAARS-CB-0358	13	21	SM	20		
CP05-EAARS-CB-0360	8.5	26	GP-GM	- 8		
CP05-EAARS-CB-0360	13.5	31	SM	46		
CP05-EAARS-CB-0360	18.5	24	SM	32		
CP05-EAARS-CB-0365	11.5	29	SM	19		
CP05-EAARS-CB-0365	18.5	23	SM	41	-	
CP05-EAARS-CB-0365	23.5	23	SM	36		
CP05-EAARS-CB-0372	14,5	19	ML	53		
CP05-EAARS-CB-0373	19	25	SM	21		
CP05-EAARS-CB-0373	29	22	SM	19		13-5-
CP05-EAARS-CB-0377	15.5	18	SM	19		
CP05-EAARS-CB-0377	19	26	SM	25		
CP05-EAARS-CB-0377	24	19	SM	18		
CP05-EAARS-CB-0377	20	15	SM	17		
CP05-EAARS-CB-0406	23.5	26	SM	28		
CP05-EAARS-RB-0282	5	6	GM	14		
CP05-EAARS-RB-0282	10	21	SM	27		
CP05-EAARS-RB-0282	15	20	SM			
CP05-EAARS-RB-0283	5	10	CL-ML	39		
CP05-EAARS-RB-0283	10	23	CL-ML	41		
CP05-EAARS-RB-0284	10	18	SM	26		
CP05-EAARS-RB-0284	20	26	SP	4		
CP05-EAARS-RB-0286	15	18	SM	20		

4.2.3 Lower Fort Thompson Limestone

A cemented zone often occurs along the base of the Fort Thompson Formation or along the contact between the Fort Thompson Formation and the underlying Caloosahatchee or Pincerest Sand. The cemented zone forms a limestone layer varying from 1 to 6-feet thick, and the top was penetrated between 19 and 39-foot depth in the borings around the perimeter of the EAA Reservoir A-1, primarily on the northern, eastern, and southern sides. It was only found north of boring CB-0280 on the western side of the EAA Reservoir A-1. It was deepest along the southern half of the eastern side and shallower to the north or along the south side. No attempt was made to trace it into the proposed EAA Reservoir A-1 interior.

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For the most part, the limestone ranges from soft and weak to moderately hard, moderately strong and porous, consisting of sand sized grains cemented only at the contacts. Thin, strong, hard, dense beds are found within the interval, all less than one foot thick. Some of the intervals near the bottom contain fine, subrounded, quartz sand similar to that in the underlying Caloosahatchee or Pinecrest Sand Member.

Attempts to core this lower limestone were made in 28 borings. Core recoveries ranged from 0 to 100 percent with an average of 45 percent. RQD ranged from 0 to 64 percent with an average of 18.5 percent. Selected samples were pulled from the core runs, wrapped for protection and sent to a laboratory for specific gravity and absorption (ASTM D6473), and unconfined compressive strength testing (ASTM D2938). Specific gravity ranged from 1.5 to 2.96 with an average of 2.17. Absorption ranged from 2.05 to 15.1 percent and averaged 8.05 percent. Unconfined compressive strength ranged from 960 to 5,920 psi and averaged 2,780 psi. Again as with the caprock, it must be emphasized that because of the large core sample losses these results probably represent the upper limits of the true range. (Table 4-3)

Table 4-3 Limestone Laboratory Testing and Core Data in the Fort Thompson Below

	1	Capro	-K	- ·		
Boring Number	Run Depth (feet)	Core Recovery (percent)	RQD (percent)	Bulk Specific Gravity	Absorption (percent)	UCS (psi)
CP05-EAARS-CB-0168	23.5-28.5	26	16			
CP05-EAARS-CB-0170	25-29	22	13			
CP05-EAARS-CB-0287	25-30	84	62	2,47	2.05	2400
CP05-EAARS-CB-0289	15-20	0.	0.			
CP05-EAARS-CB-0289	20.5-25.5	57	14	2.46	3.56	
CP05-EAARS-CB-0290	23.5-28.5	0	0			
CP05-EAARS-CB-0290	34-39	.28	14	2.31	5.26	2215
CP05-EAARS-CB-0291	23,5-28.5	15	0			
CP05-EAARS-CB-0291	36-40	65	17			
CP05-EAARS-CB-0293	37-42	92	47	2.14	9,02	1053
CP05-EAARS-CB-0304	14.5-19.5	36	12			
CP05-EAARS-CB-0304	19.5-23.5	15	0			
CP05-EAARS-CB-0310	30-35	56	24	2,96	6.1	5920
CP05-EAARS-CB-0311	13.5-18.5	10	-0.			
CP05-EAARS-CB-0311	23.5-28.5	72	18	1.91	13.28	
CP05-EAARS-CB-0312	23.5-28.5	40	0			
CP05-EAARS-CB-0312	28.5-33.5	66	14	1.501	7.38	
CP05-EAARS-CB-0312	33.5-36.5	33	0	-		
CP05-EAARS-CB-0313	25-30	68	52			
CP05-EAARS-CB-0314	25-30	60	48			2970
CP05-EAARS-CB-0314	30-35	96	64			4916
CP05-EAARS-CB-0316	21.5-25	56	28			
CP05-EAARS-CB-0316	25-30	76	49	1-		
CP05-EAARS-CB-0318	19-25	100	46	1.93	15.1	960
CP05-EAARS-CB-0318	25-30	100	53.5	2.19	7.85	3095
CP05-EAARS-CB-0320	15-20	90	46			148
CP05-EAARS-CB-0323	16-21	74	40			
CP05-EAARS-CB-0323	21-23.75	40	0			
CP05-EAARS-CB-0323	23.75-28.5	- 8	.0			
CPOS-EAARS-CB-0324	16-21	82	56:	2.39	24	
CP05-EAARS-CB-0324	21-26	32	-8		-	

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Boring Number	Run Depth (feet)	Core Recovery (percent)	RQD (percent)	Bulk Specific Gravity	Absorption (percent)	UCS (psi)
CP05-EAARS-CB-0326	17.5-22.5	80	52	2.01	II	Ale and
CP05-EAARS-CB-0326	22.5-27.5	36	0			
CP05-EAARS-CB-0327	18-23	78	28	1.901	9.63	
CP05-EAARS-CB-0327	23-28	-44	16	1.97	11.57	
CP05-EAARS-CB-0330	10-15	12	0			
CP05-EAARS-CB-0330	19-24	34	0			
CP05-EAARS-CB-0332	15-20	24	0			
CP05-EAARS-CB-0332	23.5-28.5	48	12			
CP05-EAARS-CB-0333	22-27	10	6			
CP05-EAARS-CB-0341	21.5-26.5	26	- 0			
CP05-EAARS-CB-034T	26.5-31.5	42	- 8			
CP05-EAARS-CB-0341	31.5-36.5	40	10			
CP03-EAARS-CB-0395	30-35	29	13			
CP05-EAARS-CB-0395	35-40	40	-24			
CP05-EAARS-CB-0396	25-30	11	0			
CP05-EAARS-CB-0396	30-35	26	10			-
CP05-EAARS-CB-0400	9-15	45	0			
CP05-EAARS-CB-0400	15-20	80	25			
CP05-EAARS-CB-0400	20-25	26	0			
CP05-EAARS-CB-0405	25-30	10	0			
CP05-EAARS-CB-0405	30-35	- 0	0		To the second	
CP05-EAARS-CB-0407	28.5-33.5	28	14			
CP05-EAARS-CB-0407	33.5-38.5	74	-48			

4.2.4 Caloosahatchee Formation and Pinecrest Sand

Below the Fort Thompson Formation the borings penetrated shelly, fine, uniform, subrounded, quartz sand with local cemented zones. In the rotosonic drilled borings, the cemented zones were recovered as gravel sized aggregates of the sand and shell fragments. The sand belongs to the Caloosahatchee Formation and/or the Pinecrest Sand Member of the Tamiami Formation that cannot be differentiated as noted above. The top of the sand in the recovered samples ranged from 18.5 to 43.5 feet and averaged 29 feet. Borings generally between about 30 and 50-foot deep ended in the sand. Only the borings of 100-foot depth or deeper penetrated into the underlying Ochopee Limestone Member of the Tamiami Formation. Along the eastern end of the south side of the EAA Reservoir A-1, borings CB-0267 through CB-0270 and CB-0283 penetrated a layer of silty sand near the base of the Tamiami Formation.

Densities ranged from loose to very dense. Samples sent for laboratory testing were classified as per USCS as mostly SP-SM (43), SM (23), and SW-SM (15) and occasionally SP (8), GP-GM (5), GP (1), and GM (1). Percent passing the 200 sieve ranged from 1 to 39.5 percent with an average of 10.7 percent. Moisture content and carbonate content ranged from 2 to 30 percent and 10.9 to 77.1 percent, averaging 21.3 percent and 36.1 percent, respectively. The clay content from hydrometer testing ranged from 1.4 to 6.6 percent with an average of 3.4 percent. (Table 4-4)

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Table 4-4 Caloosahatchee and Pinecrest Sand Laboratory Soil Testing

Boring Number	Depth (feet)	Moisture (percent)	ASDM D2487 Class	-200 Sieve (percent)	Clay (percent)	CO ₃
CP05-EAARS-CB-0168	48.5		SP-SM	7.5		
CP05-EAARS-CB-0168	28.5					29,1
CP05-EAARS-CB-0168	43.5					10.9
CP05-EAARS-CB-0170	48.5		SM	21.9	3.6	
CP05-EAARS-CB-0170	29					40.5
CP05-EAARS-CB-0170	43.5					37.3
CP05-EAARS-CB-0171	48.5		SP-SM	6.4		
CP05-EAARS-CB-0171	33.5					31.4
CP05-EAARS-CB-0172	28.5		SP-SM	11,4		36.8
CP05-EAARS-CB-0172	38.5					31.4
CP05-EAARS-CB-0173	28.5					27.6
CP05-EAARS-CB-0173	48,5					34.6
CP05-EAARS-CB-0174	43.5		SP-SM	8.4		
CP05-EAARS-CB-0174	38.5					28.5
CP05-EAARS-CB-0175	38.5					32
CP05-EAARS-CB-0176	43.5		SM	39.5	6,6	
CP05 EAARS-CB-0176	38.5					25.3
CP05-EAARS-CB-0177	33.5					26.7
CP05-EAARS-CB-0178	48.5		SP-SM	10.4	1.9	
CP05-EAARS-CB-0178	38.5	24.5				
CP05-EAARS-CB-0181	48.5	26.8				
CP05-EAARS-CB-0182	48.5		SM	13.2		
CP05-EAARS-CB-0183	28.5		SP-SM	9.2	1.4	
CP05-EAARS-CB-0184	28.5		SP-SM	9.5		
CP05-EAARS-CB-0185	43.5		SM	28		
CP05-EAARS-CB-0185	53.5	17	SM	14		1
CP05-EAARS-CB-0185	58.5	25	SM	15		
CP05-EAARS-CB-0189	53.5	15	GM	13:		
CP05-EAARS-CB-0189	33.5	26	SP-SM	- 6		
CP05-EAARS-CB-0189	58.5					63.2
CP05-EAARS-CB-0189	28.5					14.5
CP05-EAARS-CB-0189	63.5					41.6
CP05-EAARS-CB-0190	48.5	26	SM	15		
CP05-EAARS-CB-0190	33.5					56.5
CP05-EAARS-CB-0191	48.5		SM			
CP05-EAARS-CB-0191	33.5	20	SP-SM	6		
CP05-EAARS-CB-0191	38.5					40,7
CP05-EAARS-CB-0191	43,5					25.3
CP05-EAARS-CB-0192	48.5		GP-GM	7.9		
CP05-EAARS-CB-0192	28.5		SP-SM	10		
CP05-EAARS-CB-0193	68.5		GP-GM	6.4		
CP05-EAARS-CB-0193	58.5		SM	16.2		
CP05-EAARS-CB-0193	33.5		SP	4.3		
CP05-EAARS-CB-0193	38.5		SP-SM	8.8		
CP05-EAARS-CB-0193	63,5		SW-SM	10.8		
CP05-EAARS-CB-0194	33.5		SM	14.4		23.3
CP05-EAARS-CB-0195	43.5		SM	163		
CP05-EAARS-CB-0195	53.5		SW-SM	9.6		

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Table 4-4 Continued - Caloosahatchee and Pincerest Sand Laboratory Soil Testing

38.5 44.5 55 50.5 31 55 44.5 37 49 61 65.5 68.5	28 29 23 19 22 30 25 27 21 30	SM SP-SM SP-SM SW-SM SM SM SP SW-SM	22 8 8 10 25 22 3		
55 50.5 31 55 44.5 37 49 61 65.5 68.5	23 19 22 30 25 27 21 30	SP-SM SW-SM SM SM SP SW-SM	8 10 25 22		
55 50.5 31 55 44.5 37 49 61 65.5 68.5	19 22 30 25 27 21 30	SP-SM SW-SM SM SM SP SW-SM	10 25 22		
31 55 44.5 37 49 61 65.5 68.5	22 30 25 27 21 30	SM SM SP SW-SM	25 22		
31 55 44.5 37 49 61 65.5 68.5	22 30 25 27 21 30	SM SM SP SW-SM	25 22		
55 44.5 37 49 61 65.5 68.5	25 27 21 30	SP SW-SM	22		
44.5 37 49 61 65.5 68.5	27 21 30	SP SW-SM	3		
37 49 61 65.5 68.5	27 21 30	SW-SM			
49 61 65.5 68.5	30		-9		
61 65.5 68.5	30	SW-SM	7		
65.5 68.5		SW-SM	H		
68.5	22	SW-SM	9		
	22	SW-SM	8	1	
78.5	26	SM	39		
33.5	18	SW-SM	9		
33.5	16	SP	3		
38.5	21	SW-SM	6		
38.5	24	SP-SM	10		
	29	SP	5		
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	29	SW-SM	8	-	11.7
			12		34.7
33.5	16:	SP-SM			
23.5		SM			-
28.5	21	SM	31		
33.5	13	SP-SM	11		
	23.5 28.5	58.5 21 33.5 16 28.5 15 33.5 25 38.5 21 48.5 21 73.5 22 58.5 23 63.5 28 28.5 27 33.5 27 28.5 24 33.5 12 28.5 12 28.5 12 28.5 13 33.5 28 38.5 24 43.5 25 53.5 23 68.5 23 48.6 29 33.5 34 25 23.5 34 25 23.5 26 33.5 16 23.5 22 28.5 21	58.5 21 SP-SM 33.5 16 SW-SM 28.5 15 SP-SM 33.5 25 SP-SM 38.5 21 SP-SM 48.5 21 SP-SM 73.5 22 SP-SM 58.5 23 SW-SM 28.5 27 SP-SM 28.5 27 SP-SM 28.5 24 SM 33.5 24 SP-SM 28.5 12 SW-SM 28.5 12 SW-SM 33.5 24 SP-SM 28.5 13 GP-GM 33.5 28 SM 38.5 24 SP-SM 43.5 25 SP-SM 53.5 23 SP-SM 48.5 29 SW-SM 33.5 23 SP-SM 48.5 29 SW-SM 33.5 26 SP-SM 33.5	58.5 21 SP-SM 5 33.5 16 SW-SM 5 28.5 15 SP-SM 7 33.5 25 SP-SM 7 38.5 21 SP-SM 4 48.5 21 SP-SM 6 73.5 22 SP-SM 10 58.5 23 SW-SM 9 63.5 28 SW-SM 7 28.5 27 SP-SM 6 33.5 27 SP-SM 9 28.5 24 SM 13 33.5 30 SM 16 33.5 12 SW-SM 9 28.5 12 SW-SM 9 28.5 12 SW-SM 6 33.5 24 SP-SM 9 28.5 12 SW-SM 9 28.5 13 GP-GM 9 33.5 28 SM 15	58.5 21 SP-SM 5 33.5 16 SW-SM 5 28.5 15 SP-SM 7 33.5 25 SP-SM 7 38.5 21 SP-SM 4 48.5 21 SP-SM 6 73.5 22 SP-SM 6 73.5 22 SP-SM 9 63.5 28 SW-SM 9 28.5 27 SP-SM 6 33.5 27 SP-SM 9 28.5 24 SM 13 33.5 24 SP-SM 6 28.5 12 SW-SM 9 28.5 13 GP-GM 9 33.5 28 SM 15

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Table 4-4 Continued - Caloosabatchee and Pinecrest Sand Laboratory Soil Testing

Boring Number	Depth (feet)	Moisture (percent)	ASDM D2487 Class	-200 Sieve (percent)	Clay (percent)	CO ₃
CP05-EAARS-CB-0406	33.5	13	SP-SM	13		
CP05-EAARS-RB-0282	30	8	SM	17		
CP05-EAARS-RB-0282	45	14	SP	2		
CP05-EAARS-RB-0282	60		SP-SM	8		
CP05-EAARS-RB-0283	40	17	SP-SM	6		-
CP05-EAARS-RB-0283	45		GP-GM	6		
CP05-EAARS-RB-0283	60	8	SP-SM	8		
CP05-EAARS-RB-0283	70		SP-SM	15		
CP05-EAARS-RB-0284	40	2	GP	4		
CP05-EAARS-RB-0284	45		GP-GM	6		
CP05-EAARS-RB-0284	55	16	SP-SM	9		
CP05-EAARS-RB-0285	40		SP-SM	11		
CP05-EAARS-RB-0285	45		SP-SM	7		
CP05-EAARS-RB-0286	30	16	SP-SM	6		
CP05-EAARS-RB-0286	65	21	SM	16		
CP05-EAARS-RB-0286	35	24	SP-SM	5		Y-
CP05-EAARS-RB-0286	40		SP	1		
CP05-EAARS-RB-0286	45		SP	3		
CP05-EAARS-RB-0286	50		5M	18		
CP05-EAARS-RB-0286	60	.14	SP-SM	11		77.1

4.2.5 Ochopee Limestone

The top of the Ochopee Limestone Member of the Tamiami Formation was penetrated by the 100-foot long borings, and the total thickness was penetrated by the rotosonic drilled borings. The top of the Ochopee Limestone, as judged from the topmost SPT samples recovered and from the top in the rotosonic drilled borings, ranged from 63.5 to 89.3-foot depth, with an average of 74 feet. It averaged about 90 feet in the rotosonic drilled borings. In the borings, the Ochopee Limestone consisted of variable proportions of fine, subrounded quartz sand and fine to medium, angular to subrounded calcitic sand. Gravel sized aggregate clasts of the sand are common especially in the rotosonic drilled borings.

Density in the Ochopee Limestone as judged by SPT blow counts to 100-foot depth was mostly medium dense to dense with lesser instances of very dense zones, or refusal on apparently hard, cemented zones. Samples sent for laboratory testing were were classified as per USCS as mostly SP-SM (38), SM (24), and SW-SM (17) with a few GP-GM (4) and GW-GM (2). Percent passing the 200 sieve ranged from 7 to 30 percent with an average of 11.7 percent. Moisture content and carbonate content ranged from 7 to 28 percent and 32.8 to 78.8 percent, averaging 18.5 percent and 65.8 percent, respectively. (Table 4-5).

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Table 4-5 Ochopee Limestone Laboratory Soil Testing

Boring Number	Depth (feet)	Moisture (percent)	ASDM D2487 Class	-200 Sieve (percent)	Clay (percent)	CO ₃
CP05-EAARS-CB-0189	68.5	18	GW-GM	11		
CP05-EAARS-CB-0189	88.5	25	SM	17		
CP05-EAARS-CB-0189	73.5	24	SP-SM	-8		
CP05-EAARS-CB-0189	83.5	22	SP-SM	10	1	
CP05-EAARS-CB-0189	78.5					45,9
CP05-EAARS-CB-0189	73.5					69.1
CP05-EAARS-CB-0190	93.5	22	SM	14		
CP05-EAARS-CB-0190	68.5	19	SP-SM	12		
CP05-EAARS-CB-0190	78.5	22.	SP-SM	12		
CP05-EAARS-CB-0190	73.5					6.5
CP05-EAARS-CB-0190	98.5					67.7
CP05-EAARS-CB-0191	68,5	20	SM	13		
CP05-EAARS-CB-0191	93.5	23	SM	12		
CP05-EAARS-CB-0191	73.5					68.9
CP05-EAARS-CB-0191	78.5					67.8
CP05-EAARS-CB-0192	78,5		SW-SM	9.2		
CP05-EAARS-CB-0194	83.5		SM	16.2		
CP05-EAARS-CB-0194	73.5		SP-SM	8.9		
CP05-EAARS-CB-0194	53,5		SW-SM	8.9		
CP05-EAARS-CB-0195	78.5		SP-SM	9.3		
CP05-EAARS-CB-0195	83.5		SW-SM	9.7		
CP05-EAARS-CB-0256	91	21	SM	17		
CP05-EAARS-CB-0256	95.5	19	SM	20		
CP05-EAARS-CB-0256	98.5	22	SM	20		
CP05-EAARS-CB-0256	70	28	SP-SM	13		
CP05-EAARS-CB-0256	76	28	SP-SM	11		
CP05-EAARS-CB-0256	80,5	26	SP-SM	10		-
CP05-EAARS-CB-0256	65.5	21	SW-SM	14		
CP05-EAARS-CB-0256	85	26	SW-SM	10		
CP05-EAARS-CB-0256	89.5	22	SW-SM	9		
CP05-EAARS-CB-0266	85	[4	GW-GM	6		
CP05-EAARS-CB-0266	73	21	SW-SM	10:		
CP05-EAARS-CB-0266	76	18	SW-SM	- 11		
CP05-EAARS-CB-0266	89.5	14	SW-SM	8		
CP05-EAARS-CB-0267	88.5	21	SM	29		
CP05-EAARS-CB-0275	83.5	20	SW-SM	- 11		
CP05-EAARS-CB-0275	93.5	18	SW-SM	12		
CP05-EAARS-CB-0280	73.5	26	SM	12		
CP05-EAARS-CB-0280	83.5	22	SM	19		
CP05-EAARS-CB-0280	88.5	23	SM	12		
CP05-EAARS-CB-0280	93.5	24	SM	15		
CP05-EAARS-CB-0280	78.5	19	SP-SM	8		
CP05-EAARS-CB-0280	98.5	24	SP-SM	11		
CP05-EAARS-RB-028Z	70	7	GP-GM	7		7.2.3
CP05-EAARS-RB-0282	- 65	14	GP-GM	7		
CP05-EAARS-RB-0282	80		SP-SM	7		1

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Boring Number	Depth (fcet)	Moisture (percent)	ASDM D2487 Class	-200 Sieve (percent)	Clay (percent)	CO ₃
CP05-EAARS-RB-0282	85		SP-SM	8		
CP05-EAARS-RB-0282	115	20	SM	27		
CP05-EAARS-RB-0282	120	22	SM	26		
CP05-EAARS-RB-0282	125		SM	19		
CP05-EAARS-RB-0282	130	21	SM	25		32.8
CP05-EAARS-RB-0282	135	21	SM	21	1	
CP05-EAARS-RB-0282	150		SP-SM	7		
CP05-EAARS-RB-0282	155		GP-GM	6		
CP05-EAARS-RB-0282	95	22.	SP-SM	6	U .	
CP05-EAARS-RB-0282	145	13	SP-SM	11		
CP05-EAARS-RB-0283	75		SP-SM	8		
CP05-EAARS-RB-0283	110	18	SM	14		75.5
CP05-EAARS-RB-0283	155		SP-SM	11		
CP05-EAARS-RB-0283	100	11	SP-SM	9		71.7
CP05-EAARS-RB-0283	140	19	SP-SM	10		53
CP05-EAARS-RB-0283	115	16	SW-SM	12		75.9
CP05-EAARS-RB-0283	125	13	SW-SM	7		77.7
CP05-EAARS-RB-0283	150	13	SW-SM	8		68.3
CP05-EAARS-RB-0283	160	14	SW-SM	0		67.2
CP05-EAARS-RB-0284	100	11	GP-GM	6		
CP05-EAARS-RB-0284	120	13	SM	13		
CP05-EAARS-RB-0284	70	9	SP-SM	7		78.8
CP05-EAARS-RB-0284	75		SP-SM	6		
CP05-EAARS-RB-0284	90	15	SP-SM	7		72.1
CP05-EAARS-RB-0284	110	1	SP-SM	.9		
CP05-EAARS-RB-0284	115		SM	13		
CP05-EAARS-RB-0284	125	10	SP-SM	8		
CP05-EAARS-RB-0284	140	9	SP-SM	7		55.4
CP05-EAARS-RB-0284	150		SP-SM	9		
CP05-EAARS-RB-0285	70	1	SM	13		
CP05-EAARS-RB-0285	75		SP-SM	11		
CP05-EAARS-RB-0285	110		SM	16		
CP05-EAARS-RB-0285	115		SP-SM	11		
CP05-EAARS-RB-0286	75	13	SM	13		72
CP05-EAARS-RB-0286	110	20	SM	30		
CP05-EAARS-RB-0286	70		SP-SM	11		
CP05-EAARS-RB-0286	80	12	SP-SM	7		
CP05-EAARS-RB-0286	95	17	SP-SM	8		
CP05-EAARS-RB-0286	105	14	SP-SM	7		
CP05-EAARS-RB-0286	115		SP-SM	10		
CP05-EAARS-RB-0286	125	17	SP-SM	12		
CP05-EAARS-RB-0286	140	15	SP-SM	7		58.7
CP05-EAARS-RB-0286	150	12	SP-SM	10		300
CP05-EAARS-RB-0286	155		SP-SM	1 11		
CP05-EAARS-RB-0286	88.5	21	SW-SM	12		
CP05-EAARS-RB-0280	98.5	15	SW-SM	12		

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4.2.6 Unnamed Sand Formation

The unnamed sand formation was encountered in the rotosonic drilled borings between the Ochopee Limestone and the Fort Pearce Formation It consists mostly of shelly, uniform, fine grained, subrounded quartz sand similar to that of the Pinecrest Sand Member, but it is silty. It is identified primarily by the yellow-gray color.

Samples sent for laboratory testing were assigned USCS classifications of SM (8), SP-SM (4), and CL-ML (1). Percent passing the 200 sieve ranged from 11 to 41 percent with an average of 24.4 percent. The moisture content on samples tested ranged between 12 percent and 21 percent and averaged 18.1 percent. Carbonate content ranged from 36.6 to 61.6 percent with an average of 47.6 percent. (Table 4-6)

Table 4-6 Laboratory Soil Testing for the Unnamed and Peace River Formations

Boring Number	Depth (feet)	Moisture (percent)	ASDM D2487 Class	-200 Sieve (percent)	Clay (percent)	CO ₃
Unnamed Formation	12					
CP05-EAARS-RB-0282	180	20	SM	20		54.5
CPOS-EAARS-RB-0283	185	20	CL-ML	41		53.6
CP05-EAARS-RB-0283	165	20	SM	23		41.2
CP05-EAARS-RB-0283	180	20	SM	29		37.2
CPOS-FAARS-RB-0283	195	17	SM	27		36.6
CP05-EAARS-RB-0284	175	17	SM	22	-	48.2
CP05-EAARS-RB-0284	185	15	SM	35		
CP05-EAARS-RB-0284	155	12.	SP-SM	- 11	0	
CP05-EAARS-RB-0285	150	10	SM	32	0	
CP05-EAARS-RB-0285	155	18	SM	19	3	
CPOS-EAARS-RB-0285	175	. 18	SM	41	7	
CP05-EAARS-RB-0285	190	28	5M	47		
CP05-EAARS-RB-0286	175	18	SM	19		
CP05-EAARS-RB-0286	180	- 21	SM	37		
CP05-EAARS-RB-0286	160	20	SP-SM	12		61.6
Peace River Formation					1	
CP05-EAARS-RB-0282	225	36	ML	75		
CP05-EAARS-RB-0282	200	20.	SM	3.4		
CP05-EAARS-RB-0282	215	28	SM	29		
CP05-EAARS-RB-0283	200	25	SM	22		23.9
CP05-EAARS-RB-0283	205	26	SM	25		30.5
CP05-EAARS-RB-0284	225	75	ML	99		
CP05-EAARS-RB-0284	205	23	SM	23		
CPOS-EAARS-RB-0284	210	26	SM	-43		
CP05-EAARS-RB-0285	205	32	SM	39	13	
CP05-EAARS-RB-0285	210	34	SM	41	4	
CP05-EAARS-RB-0285	215	43	CL	- 71		
CP05-EAARS-RB-0285	215	43	MH	89	49	
CP05-EAARS-RB-0285	220	101	MB	38	88	
CP05 EAARS-RB-0285	220	88	CL	62		
CP05-EAARS-RB-0285	225	148	C11	86		
CP05-EAARS-RB-0283	245	151	MH	08	-51	
CPGS-EAARS-RB-0286	215	25	MI	. 53		
CP05-EAARS-RH-0286	195	19	554	51	7	

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4.2.7 Peace River Formation

The top of the Peace River Formation was penetrated in the rotosonic drilled borings and all ended in the formation. The top of the formation ranged from 191 to 200-foot depth, and averaged 197 feet. In the borings, it mostly consisted of very fine, silty sand, grading to more finely grained with depth. Samples sent to the laboratory were USCS classified as SM (7), ML (3), CL (2), and CH (1). Percent passing the 200 sieve ranged from 22 to 99 percent with an average of 50.2 percent. Two carbonate content tests returned 23.9 to 30.5 percent. Moisture content ranged between 19 and 148 percent with an average of 44.8 percent. (Table 4-6)

4.3 HYDRAULIC INTERVAL TESTING RESULTS

At two of the locations (RB-0238 and RB-0286) the static water level below ground level was progressively lower as the hole was drilled deeper. At RB-0285 this trend was reversed. In RB-0284 the trend was for the water levels to lower with depth apart from the uppermost (40 to 50 feet) which was lower than the static water level at the 70 to 80-foot interval. At RB-0282 the levels were generally the same with depth until 155-160 feet where the static level was higher than the strata above.

The pH, temperature, and conductivity of effluent stream were generally checked twice during the pumping of each interval in borings RB-0284 and RB-0284, once about one-half way through pump testing and once near the end. The results of the chemistry monitoring are listed in Table 4-7.

RB-0283 RB-0284 Interval **Parameters** Depth (feet) Late data Early data Late data Early data 40-50 7.43 pH 75.0 25.4 24.9 cmp 927 892 653 644 Conductivity (microSiemens) 7.39 7.28 7.41 70-80 pH 7.43 25.0 24.9 Temp "(003 1017 Conductivity 2410 2440 (microSiemens) 7.31 7.65 7.44 7.46 pH 24.8 24.9 25.2 Temp 'C 4700 Conductivity 2690 2680 4660 (microSiemens) 150-160 pH 25.3 25.1 24 9 Temp (7240 7290 4680 4700 Conductivity (microSiemens)

Table 4-7 Groundwater Chemistry Monitoring Results

The conductivity results indicate that the water quality decreases with depth in both boreholes from non brackish at the top to brackish at depth. This suggests that there is not much vertical movement and mixing of the groundwater.

Comparison of the same depth intervals between the two boreholes indicates that there are significant horizontal variations in quality.

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4.3.1 Aquifer Responses

The pumping time/drawdown data have been analyzed using the Cooper-Jacob straight line method (semilog plot) and the Hantush curve matching method (log-log plot). Where the recovery data were suitable, they have been analyzed using the Cooper-Jacob straight line method. The units of transmissivity are feet squared per day (feet day). Results are presented in Table 4-8 through Table 4-12.

There are large differences between the transmissivities determined for the pumping data by the semilog and log-log plot methods. The transmissivities determined with the semilog method are generally characteristic of the types of earth materials encountered. The transmissivities determined by the log-log method are too low for the types of earth materials encountered, and the data curves did not generally fit the type curve well. The transmissivities determined by the semilog method are more representative of the in situ materials.

Where available, the transmissivities determined from the recovery data generally are comparable to the transmissivities determined for the pumping data by the semilog method, just slightly higher. The drawdown in pumped wells is generally higher than the drawdown in the aquifer, so analyses of the data generally underestimates the transmissivity. The analysis of recovery data tends to mitigate the problem and thus gives results that are more representative of the aquifer characteristics. Where available, the transmissivities determined by the recovery analyses should be used, and the transmissivities determined from the pumping data by the semilog method should be used otherwise.

The results tabulated below include the pumping rates and the measured drawdowns at 30 minutes of pumping. Since the pumping rates for each interval were similar, the drawdowns are inversely related to the calculated transmissivities, and the greatest part of the total drawdown was achieved in the first 30 minutes of pumping in each interval. The total drawdown in RB-0285, 40 to 50 feet was less than 0.1 foot, too small to provide data suitable for analysis. No transmissivity is given for that interval. However, considering the small drawdown, the transmissivity is probably higher than that in RB-0283, 40 to 50 feet. The data for RB-0283, 40 to 50 feet, is also questionable because of the small drawdown, but the data at least shows an apparent linear trend. Both of these tests indicate high transmissivity intervals, but quantitative determinations cannot be made.

The complete pumping test data and analyses are contained in Appendix 4. It should be noted that a partial collapse often occurred in the open hole intervals during the pumping tests and was detected by measuring the hole depth again after the testing (see Table 4-8).

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Table 4-8 Aquifer Characteristics for RB-0282

RB-0282 Interval Depth (feet)	Pumping rate (gpm) and duration (minutes)	Draw- down at 30 minutes of pumping (feet)	Recovery monitoring period (minutes)	Semilog plot (time vs. drawdown) (feet²/day)	Log - log plot (time vs. draw- down) (feet²/day)	(feet*/day)	Recovery Semilog plot (time vs. drawdown) (feet/day)
60-70	- 18	2,44		C	L		
	90		14	5300	540		5765
80-90	18	0.29		C	L	U (early)	100
	50		30	25500	1.750	9900	Too fast
120-130	2.44	18.52		C	L	-	
	120		60	6.3	0.002		4.1
150-160	18	1		C	L		
	60		45	18500	930		9900

C = Confined or unconfined aquifer without delayed yield

L = Leaky confined

U = Unconfined aquifer with delayed yield

gpm = Gallons per minute

feet2/day = Transmissivity units in feet squared per day

Table 4-9 Aquifer Characteristics for RB-0283

RB-0283 Interval Depth (feet)	Pumping rate (gpm) and duration (minutes)	Draw- down at 30 minutes of pumping (feet)	Recovery monitoring period (minutes)	Semilog plot (time vs. drawdown) (feet*/day)	Log - log plot (time vs. draw- down) (feet²/day)	(feet ² /day)	Recovery Semilog plot (time vs. drawdown) (feet/day)
40-50	18.75	0.05		C	L		
	30		10	63200	5700		Too fast
70-80	18.5	2.28		C	L		
	75		30	7300	105		7200
110-120	19.5	3,8		C	L		
	70		30	5900	70		8600
150-160	18.5	2.61		C	L		
	65		30	6700	100		7250

C = Confined or unconfined aquifer without delayed yield

L = Leaky confined

U = Unconfined aquifer with delayed yield

gpm = Gallons per minute

feet /day = Transmissivity units in feet squared per day

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Table 4-10 Aquifer Characteristics for RB-0284

RB-0284 Interval Depth (feet)	Pumping rate (gpm) and duration (minutes)	Draw- down at 30 minutes of pumping (feet)	Recovery monitoring period (minutes)	Semilog plot (time vs. drawdown) (feet*/day)	Log - log plot (time vs. draw- down) (feet²/day)	(feet²/day)	Recovery Semilog plot (time vs. drawdown) (feet /day)
40-50	18.5	0.105		C	L	1	
	30		10	Bad Data	8900		Too fast
70-80	18.5	0.65		C	L		
	60		30	27000	800		32500
110-120	18.5	1.74		C	L		
	30		10	14200	580		Foo fast
150-160	18.5	2.67		C	L		
	60		30	10400	370		Too fast

C = Confined or unconfined aquifer without delayed yield

Table 4-11 Aquifer Characteristics for RB-0285

RB-0285 Interval Depth (feet)	Pumping rate (gpm) and duration (minutes)	Draw- down at 30 minutes of pumping (feet)	Recovery monitoring period (minutes)	Semilog plot (time vs. drawdown) (feer ² /day)	Log - log plot (time vs. draw- down) (feet /day)	(feef/day)	Recovery Semilog plat (time vs. drawdown) (feet /day)
40-50	Stepped	5.34					
	16 max		14				166
70-80	Stepped	12.07			7		
	15.6 (max)		14				56
110-120	20.5	1.08		C	L		
	60		14	20800	1800	7	Too fast
150-160	22.5	2.92		C	L		
	120		14	10800	400		10600

C = Confined or unconfined aquifer without delayed yield

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L = Leaky confined

U = Unconfined aquifer with delayed yield

gpm = Gallons per minute

feet2/day = Transmissivity units in feet squared per day.

L = Leaky confined

U = Unconfined aquifer with delayed yield

gpm = Gallons per minute

feet day = Transmissivity units in feet squared per day

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Table 4-12 Aquifer Characteristics for RB-0286

RB-0286 Interval Depth (feet)	Pumping rate (gpm) and duration (minutes)	Draw- down at 30 minutes of pumping (feet)	Recovery monitoring period (minutes)	Semilog plot (time vs. drawdown) (feet²/day)	Log — log plot (time vs. draw- down) (feet²/day)	(feet*/day)	Recovery Semilog plot (time vs. drawdown) (feet/day)
40-50	18	14.06		C	L		
	120		30	60	20		60
70-80	18	1.37		C	L	U (early)	
	75		30	2400	300	400	7050
110-120	18,75	4.31		C	L		
	75		30	4200	60		6606
150-160	18	0.91		C	L	U (early)	
	75		30	10400	280	50	12400

C = Confined or unconfined aquifer without delayed yield

L = Leaky confined

U = Unconfined aquifer with delayed yield

gpm = Gallons per minute

ft /day = Transmissivity units in feet squared per day

4.4 GROUNDWATER MONITORING RESULTS

The groundwater levels in the three piezometers installed during the summer 2005 drilling program were determined on November 22, 2005. The data are tabulated in Table 4-13. The depths are measured from the top of the flush mounted protective casing.

Table 4-13 Groundwater Depths

Boring	Interval Depth (feet)	Depth to Water (feet)		
RB-0283	108 to 121.5	2.3		
RB-0284	68 to 81	1.2		
RB-0286	148 to 161	1.4		

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5.0

REFERENCES

BLACK & VEATCH

South Florida Water Management District EAA Reservoir A-I Geotechnical Data Report

March 2006

5.0 REFERENCES

- Harvey, J.W., Krupa, S.L., Gefvert, C.G., Mooney, R.H., Choi, J., King, S.A., and Giddings, J.B. Interactions Between Surface Water and Ground Water and Effects on Mercury Transport in the North-Central Everglades. U.S. Geological Survey Water-Resources Investigations Report 02-4050, 82 pages. 2002.
- McCollum, S.H., Cruz, E., Stem, L.T., Wittstruck, W.H., Ford, R.D., Watts, F.C. Soil Survey of Palm Beach County Area, Florida. USDA/NRCS in cooperation with University of Florida Agricultural Experiment Station, 1978.
- Miller, James A., Hydrogeologic Framework of the Floridan Aquifer System in Florida and in Parts of Georgia, Alabama, and South Carolina, U.S. Geological Survey Professional Paper 1403-B, United States Government Printing Office, Washington, D.C. 1986.
- Miller, Wesley L., 1987. Lithology and Base of the Surficial Aquifer System, Palm Beach County, Florida: USGS, Water-Resources Investigations Report 86-4067, 1 sheet.
- Missimer, T.M. Stratigraphic Relationships of Sediment Facies within the Tamiami Formation of Southwestern Florida: Proposed Intraformational Correlations. In: Scott, T.M., and Alman, W.D., eds., The Plio-Pleistocene Stratigraphy and Paleontology of Southern Florida: Florida Geological Survey, Special Publication 36, p. 63-92, 1992.
- Reese, R. S. and Cunningham, K. J. Hydrogeology of the Grav Limestone Aquifer in Southern Florida. United States Geological Survey Water-Resources Investigations Report 99-4213, United States Government Printing Office, Washington, D.C. 2000.
- Reese, R. S. and Memberg, S. J. Hydrogeology and Distribution of Salinity in the Floridan Aquifer System, Palm Beach County, Florida. United States Geological Survey Water-Resources Investigations Report 99-4061, United States Government Printing Office, Washington, D.C. 2000.
- Schroeder, M.C., Milliken, D.L., and Love, S.K., 1954. Water Resources of Palm Beach County, Florida: Florida Geological Survey, Water Resources Studies, Report of Investigations #13, 63 p.
- Scott, T. M., The Lithostratigraphy of the Hawthorn Group (Miocene) of Florida. Bulletin No. 59, Florida Geological Survey, Tallahassee, Florida, 1988.
- Text to Accompany the Geologic Map of Florida. Open-File Report No. 80, Florida Geological Survey, Tallahassee, Florida. 2001.
- —— Personal Communication, 2005.

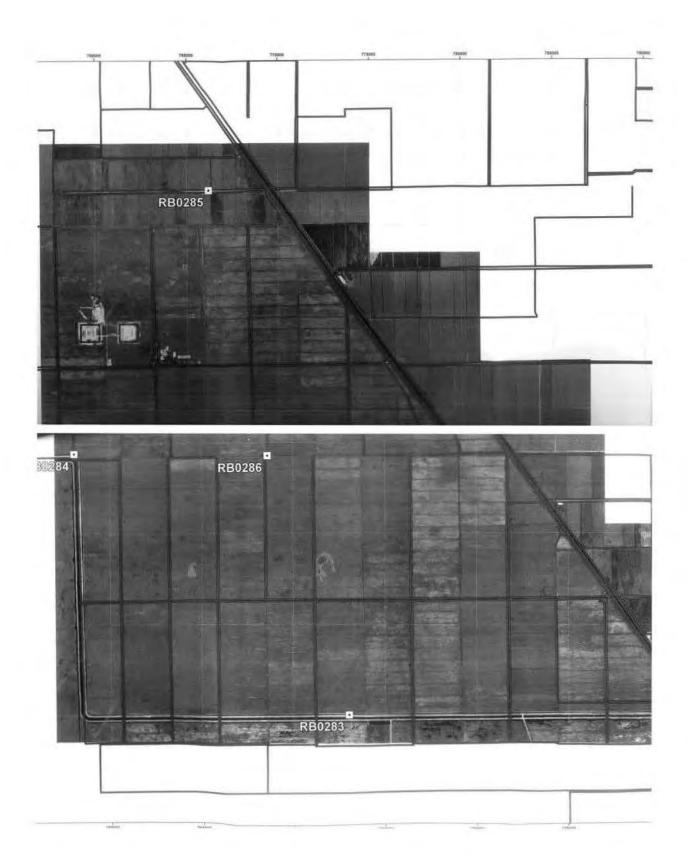
BLACK & VEATCH

March, 2006

- Slack, J.J., Planning Aid Letter to Dennis W. Barnett, Acting Chief, Planning Division, USACE dated 11March 2005.
- White, William A., The Geomorphology of the Florida Peninsula, Bulletin No. 51, Florida Bureau of Geology, Tallahassee, Florida, 1970.
- Williams Earth Sciences, Geotechnical Field Exploration Summary Report; EAA Palm Beach County, Florida, prepared for South Florida Water Management District, June 11, 2004.
- Williams Earth Sciences, Addendum to Geotechnical Field Exploration Summary Report; EAA Palm Beach County, Florida, prepared for South Florida Water Management District, July 30, 2004.

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PLATE 1 EAA RESERVOIR A-1 BORING LOCATION PLAN



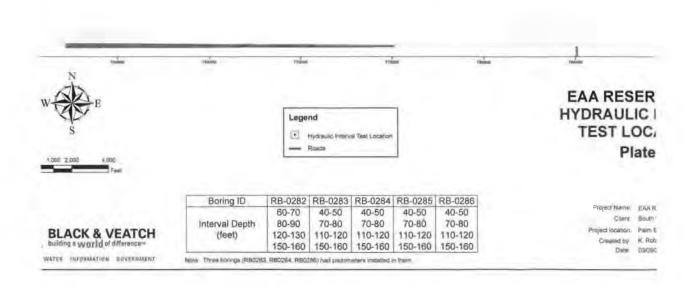
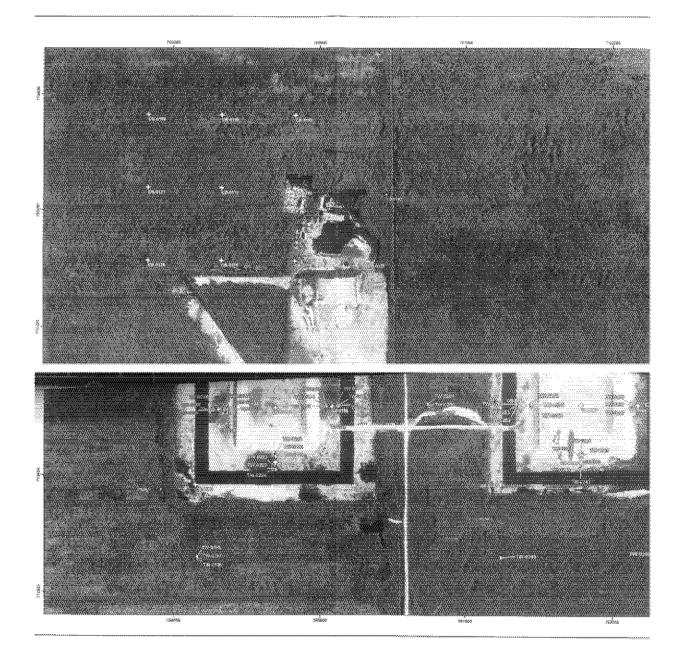


PLATE 2 EAA RESERVOIR A-1 TEST CELLS BORING AND PIEZOMETER LOCATION PLAN



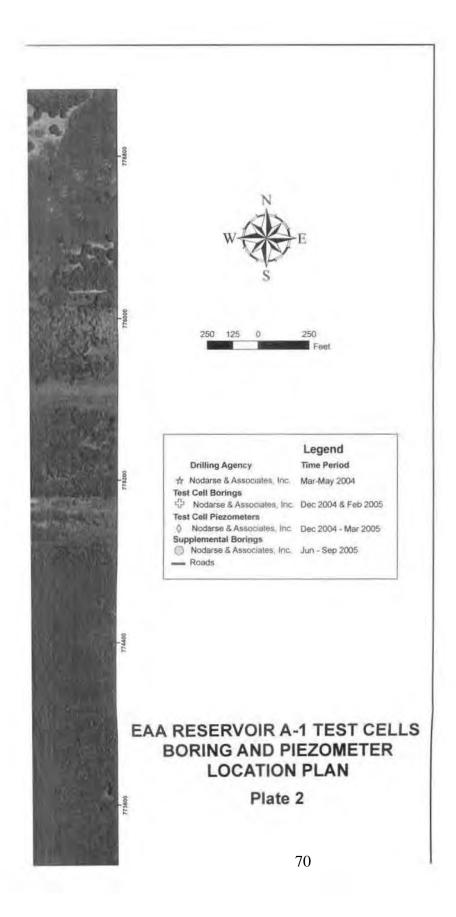


PLATE 3 EAA RESERVOIR A-1 HYDRAULIC INTERVAL TEST LOCATIONS

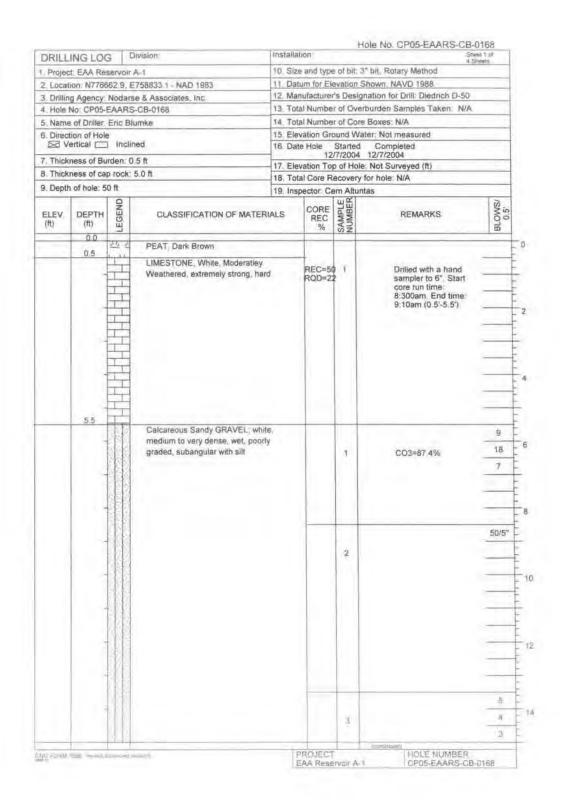
APPENDIX 1

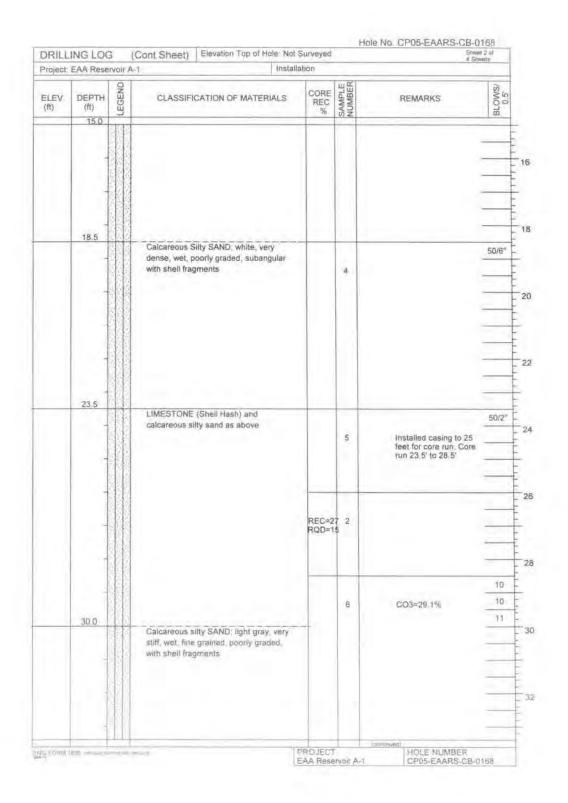
APPENDIX 1 TEST CELL BORINGS AND PIEZOMETER INSTALLATION LOGS: 168-180

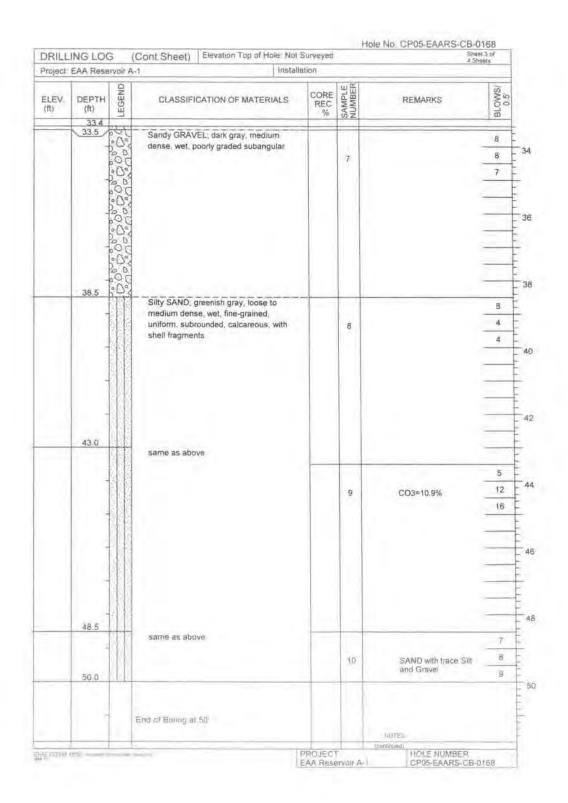
SOIL CLASSIFICATION CHART

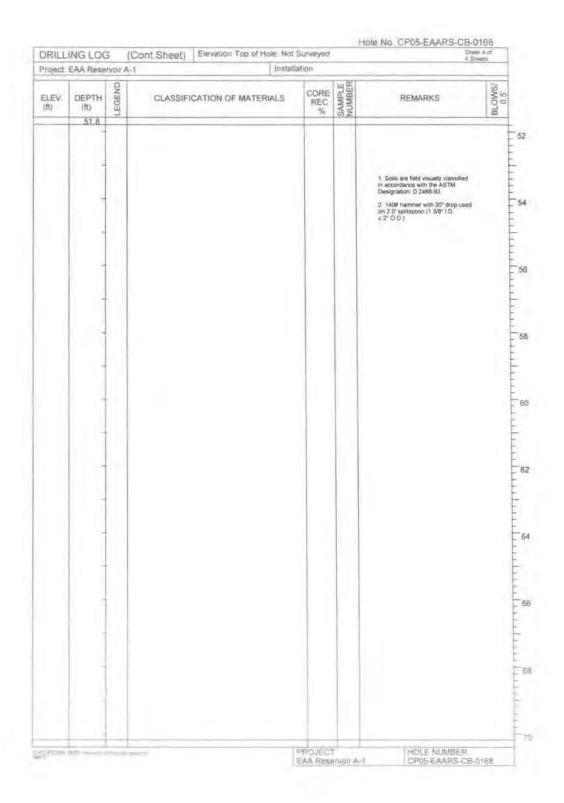
3.0	ONE	SYME	BOLS	TYPICAL	
IVI	AJOR DIVISION	OMS	GRAPH LETTER		DESCRIPTIONS
	GRAVEL AND	CLEAN GRAVELS		GW	WELL-GRADED GRAVELS, GRAVEL- SAND MIXTURES, LITTLE OR NO FINES
COARSE GRAINED SOILS	GRAVELLY SOILS	(LITTLE OR NO FINES)		GP	POORLY-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
	MORE THAN 50% OF COARSE	GRAVELS WITH FINES		GM	SILTY GRAVELS, GRAVEL - SAND - SILT MIXTURES
	FRACTION RETAINED ON NO. 4 SIEVE	(APPRECIABLE AMOUNT OF FINES)		GC	CLAYEY GRAVELS, GRAVEL - SAND CLAY MIXTURES
MORE THAN 50% OF MATERIAL IS LARGER THAN NO. 200 SIEVE SIZE	SAND	CLEAN SANDS		sw	WELL-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES
	SANDY SOILS	(LITTLE OR NO FINES)		SP	POORLY-GRADED SANDS, GRAVELLY SAND, LITTLE OR NO FINES
	MORE THAN 50% OF COARSE FRACTION	SANDS WITH FINES		SM	SILTY SANDS, SAND - SILT MIXTURES
	PASSING ON NO.	(APPRECIABLE AMOUNT OF FINES)		sc	CLAYEY SANDS, SAND - CLAY MIXTURES
		LIQUID LIMIT LESS THAN 50		ML	INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY OR CLAYEY FINE SANDS OR CLAYEY SILTS WITH SLIGHT PLASTICITY
FINE GRAINED SOILS	SILTS AND CLAYS			CL	INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS, LEAN CLAYS.
				OL	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY
MORE THAN 50% OF MATERIAL IS SMALLER THAN NO. 200 SIEVE				МН	INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SAND OR SILTY SOILS
SIZE	SILTS AND CLAYS	AND CREATER THAN 50		СН	INORGANIC CLAYS OF HIGH PLASTICITY
			он	ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, DRGANIC SILTS	
н	IGHLY ORGANIC	SOILS	70 A7 53 5 F TT CT 70 F TR TR TR	PI	PEAT, HUMUS, SWAMP SOILS WITH HIGH ORGANIC CONTENTS

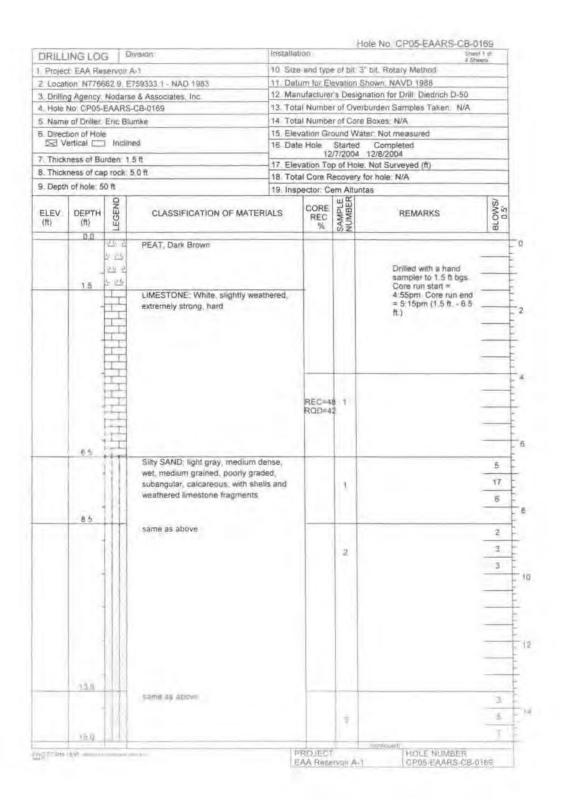
HOTE DUAL SYMBOLS ARE USED TO INDICATE BORDERLINE SOIL CLASSIFICATIONS

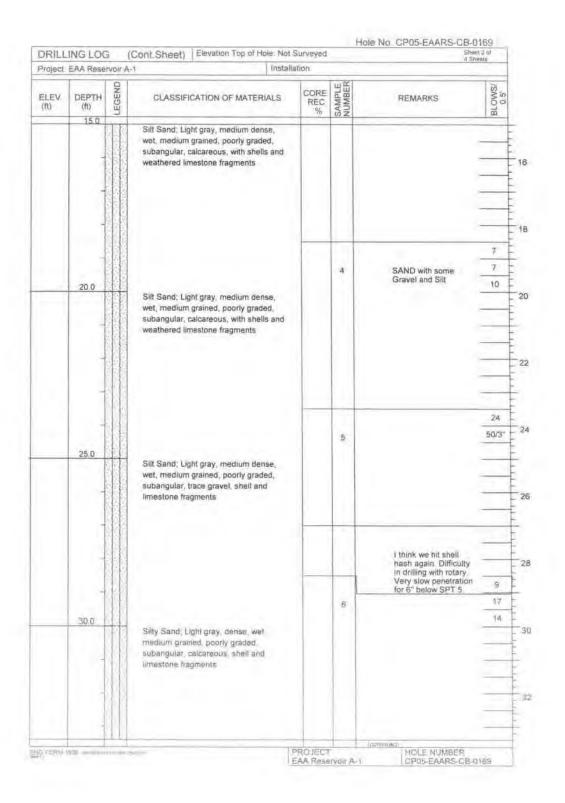


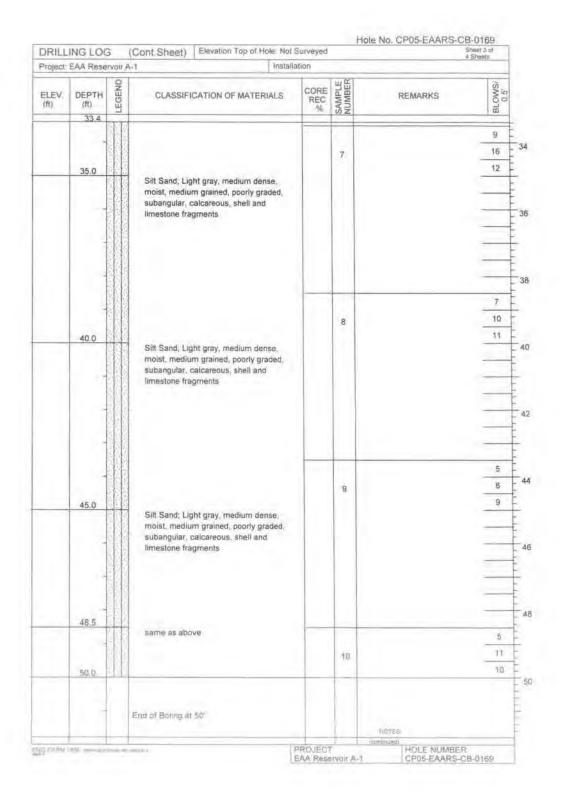


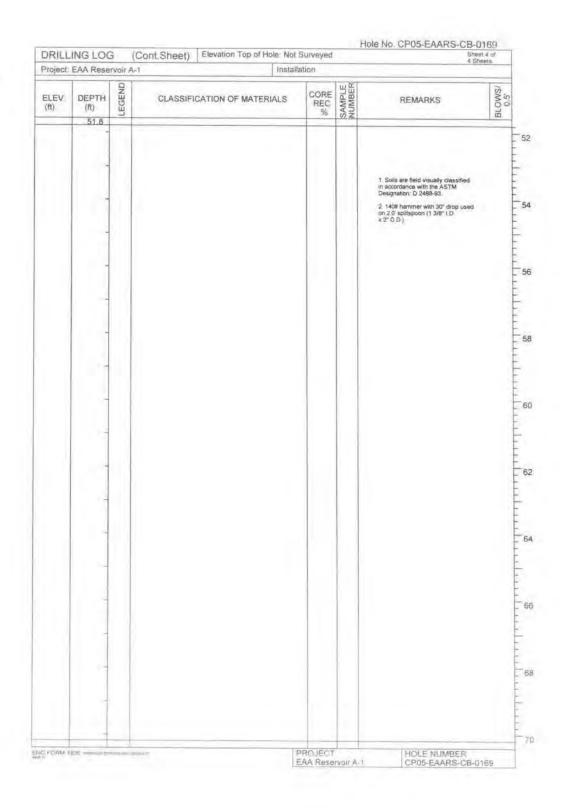




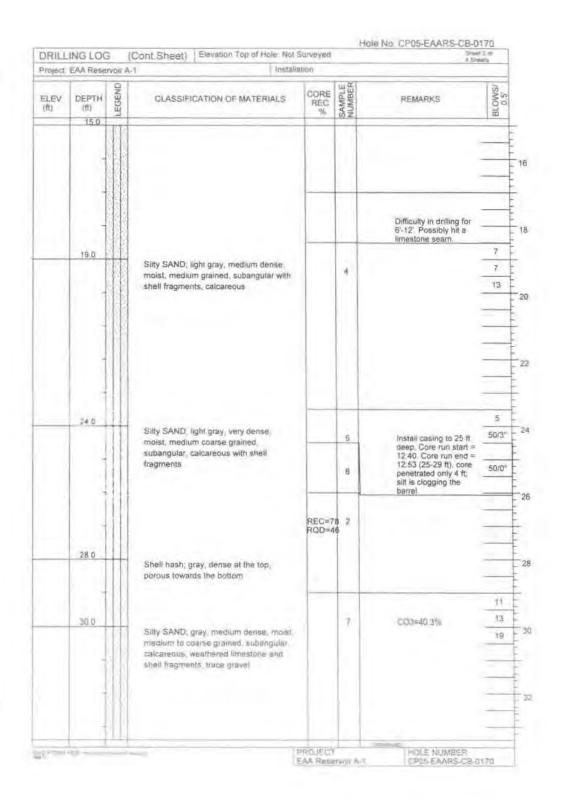




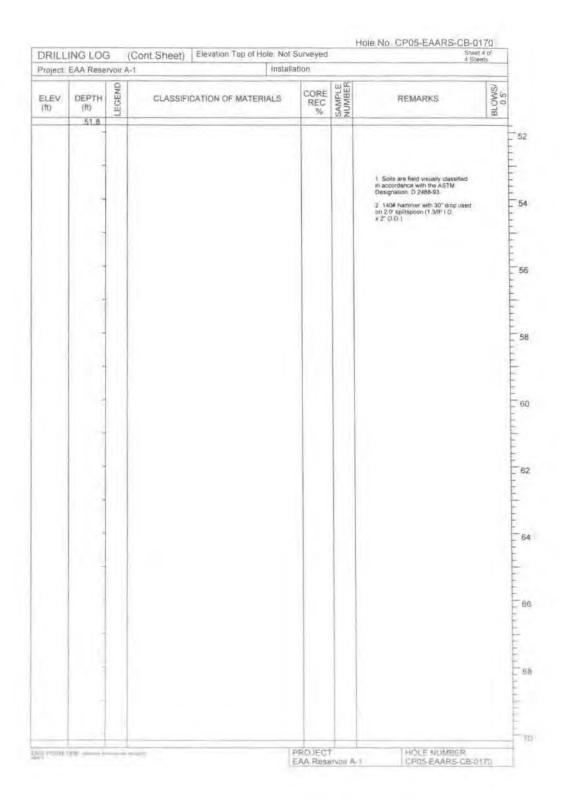




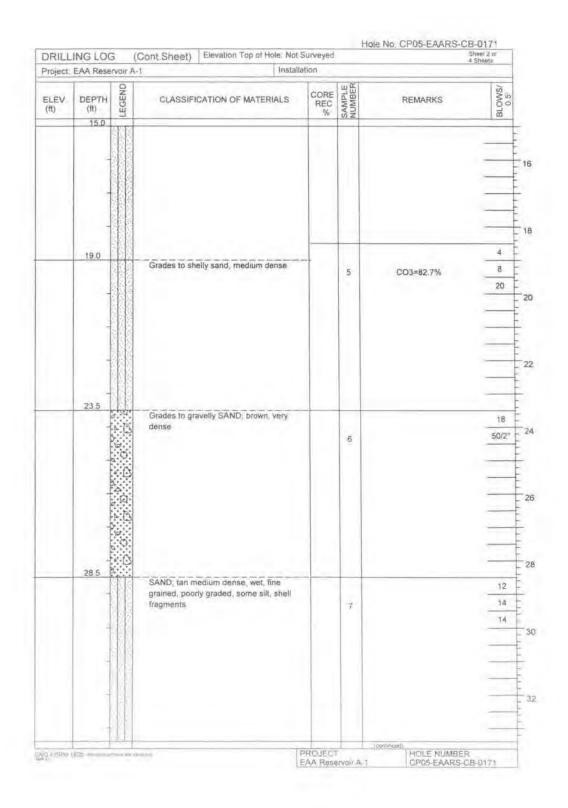
DRILL	ING LO	a D	livision: II	Hole No. CP05-EAARS-CB-0170 Sneet 1 of a Sneets				
1. Projec	EAA Re	servoir	A-1 1	10. Size and type of bit. 3" bit. Rotary Method				
				11. Datum for Elevation Shown, NAVD 1988				
3. Drillin	g Agency:	Nodars	se & Associates, Inc. 1	2 Manuf	acturer	s Design	nation for Drill: Diedrich D-50	
4. Hole	No: CP05-	EAARS	-CB-0170 1	3. Total	Number	of Over	burden Samples Taken: N/A	
5. Name	of Driller	Eric Bl	umke 1	4. Total	Number	of Core	Boxes: N/A	
	tion of Hale			5. Eleval	ion Gro	und Wa	ter: Not measured	
≥ V	ertical	Inclin	ned	6. Date I		Started	Completed 12/8/2004	
7_ Thick	ness of Bu	rden: 1	.O ft	7 Fleval			Not Surveyed (ft)	_
8. Thick	ness of ca	p rock:	508		-		for hole: N/A	
9. Depth	of hole: 5	0 ft	-	19. Insper		m Altunt		
ELEV_ (ft)	DEPTH (ft)	LEGEND	CLASSIFICATION OF MATERIAL	LS	CORE REC %	SAMPLE	REMARKS	BLOWS/ 0.5'
	0.0	100 11	DEAT Dad house			-		1
		to stile	PEAT, Dark brown					
	1.0	10.11					Drilled with hand	
		T	LIMESTONE, light gray, slightly to				sampler. Core run	
		T	moderately weathered, extremely				start = 10:00am. Core run finish = 10:14am	_
	-	1	strong, hard, vuggy	-			two mostl = destrictill	
		1						
		1		ic ic	REC=50	1		
					QD=28			
		T						_
		T						
		1						
		1						
	-	1						
		1						_
	6.0	7.5	Complete the compl					
		6	Gravelly SAND; light, medium dense.					4
		6 ()	wet, coarse grained, subangular, weathered limestone fragments			3		8
	-	0	CONTACTOR STATES			1		19
		0.0						10.
		00						
	8.5	0						
			Silty Sand Consolidated LIMESTON					2
			light gray, very dense, moist, medius grained, subangular, with shell	m		2	CO2=92 col	25
			fragments, calcareous			4	CO3=82.6%	_
	- 3		AND THE RESIDENCE OF THE PARTY					50/2"
								-
		MM						
	1							_
						-		
	14.0		And Assessed to the control of the c					7
		28.0	Silty SAND: lighter gray, loose, mois			3	SAND with some Sill	B
		201	medium grained subangular with shi fragments, calcareous	78(1)			and trace Gravel	0
		1. 11	administration manual contra				(con(reum)	-

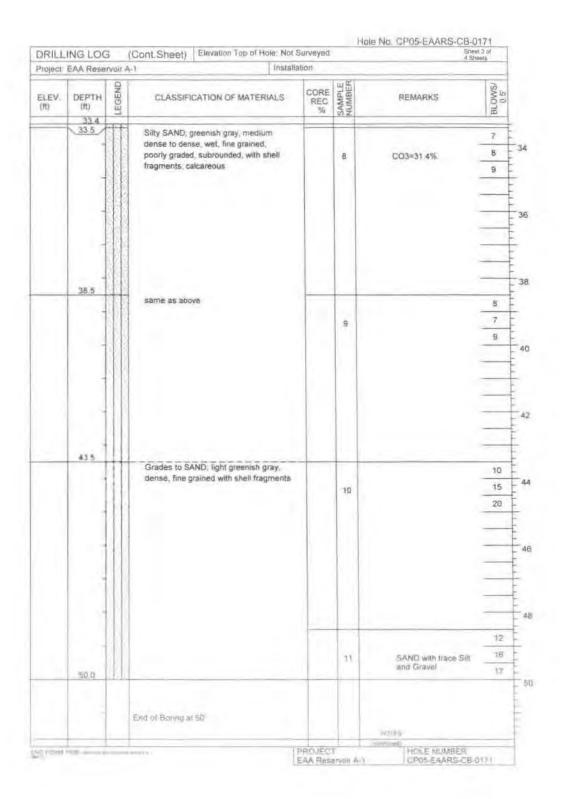


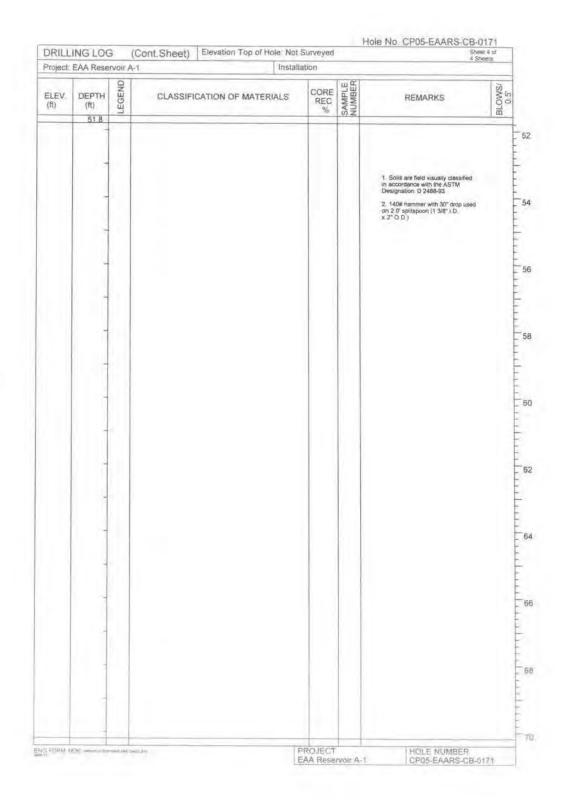
roject	ING LOC		(Cont.Sheet) Elevation Top of Hole: Not Install:	ation		4 She	443
ELEV (ft)	DEPTH (ff)	LEGEND	CLASSIFICATION OF MATERIALS	CORE REC %	SAMPLE	REMARKS	BLOWS/ 0.5'
							5
	34.0	11	Sitty SAND; gray, medium dense, moist,		8		7
			medium to coarse grained, subangular, calcareous, weathered limestone and		0		8
		shell fragments, trace gravel					
	39.0		CIB. CAND				3
			Silty SAND; gray, loose, moist, medium grained, subangular, calcareous,		9		2
			weathered limestone and shell fragments		4		
	***			-			3
	44.0		Silty SAND; gray, loose, wet, fine	10	10	10 CO3=37.3%	4
			grained, subrounded, calcareous with shell fragments		10		4
	- 4						
	-						
							-
	49.0	+	Silty SAND; greenishgray, loose, wet,				1
			fine grained, subrounded, calcareous		37	SAND with some Sill	5
	50.0	146	with shell fragments	+	1-1		5
			End of Boring at 50'				
_	-			PROJECT		NOTES (continent)	

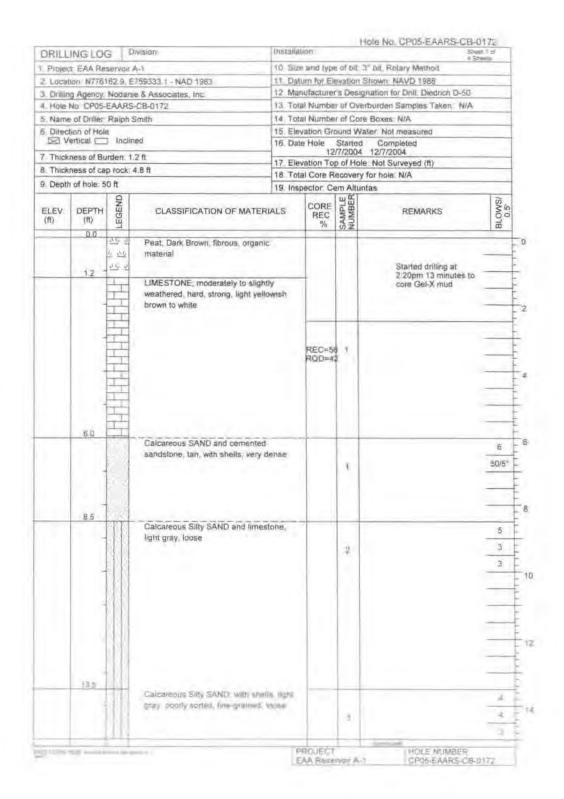


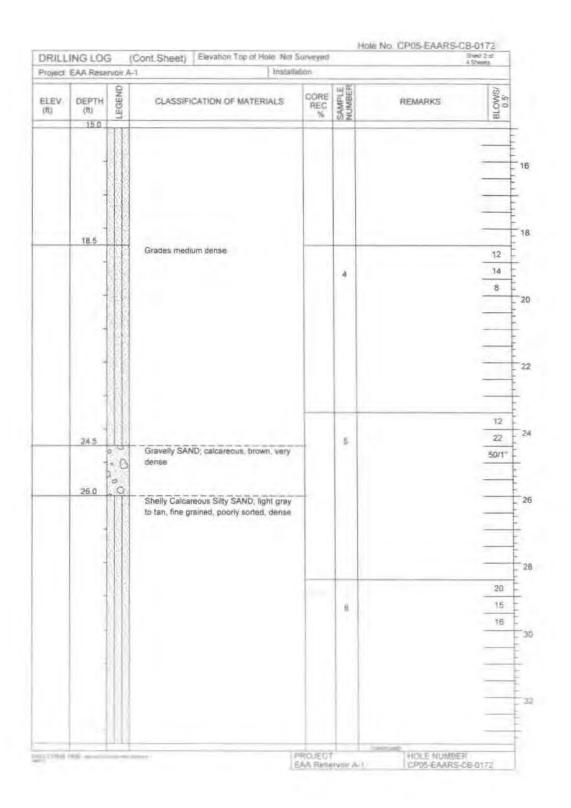
DRILL	ING LO	GE	Division	Installation			fole No. CP05-EAARS-CB-0 Sneet 4 Stock	16	
Projec	EAA R	BEIVOIL	A-1	10 Size and type of bit 3" bit, Rotary Method					
				11 Datum	for Ele	evation	Shown: NAVD 1988		
3 Drilling	g Agency	Nodars	CO. CO. T IN THE GRAND THE CO.	12. Manufacturer's Designation for Drill, Diedrich D-50					
4 Hole I	No CP05	EAARS	-CB-0171	13. Total Number of Overburden Samples Taken, N/A.					
5 Name	of Driller	Raiph		14 Total Number of Core Boxes N/A					
	ion of Ho				_		ater: Not measured		
	artical			16 Date H		Started 7/2004			
	ness of B			12/7/2004 12/7/2004 17 Elevation Top of Hole: Not Surveyed (ft)					
B. Thicks	ness of ca	p rock	E 2 B			-	for hole: N/A		
9 Depth	of hole:	50 ft		19. Inspec	tor. Ce		ntas		
ELEV (ft)	DEPTH (ft)	LEGEND	CLASSIFICATION OF MATERIA		ORE REC %	SAMPLE	REMARKS	BLOWS/ 0.5'	
	0.0	20.5	DEAT Me assessed						
		436	PEAT No sample						
	1.0	100 0					Gel-X drilling mud		
			LIMESTONE tan to light yellowish				Double tube core		
		1	brown, hard, strong, moderately				barrel	-	
			weathered, vuggy	-		-			
		T		R	EC=53	1			
		1			QD=3				
								_	
		1							
		1							
		T						_	
	83						Soft at 5.2 feet		
	6.2	111	Calcareous Sitty SAND: light gray, k	005E				-4	
		1510	to very dense, wet, fine to medium			7	CO3=87.7% Manual	6	
			grained, poorly graded, subangular,			Hammer Hammer		12	
			shell fragments and limestone seam	15	-			- 50	
								5	
		161				2		3	
								3	
								-	
								-	
								42	
						3		50(2"	
		1				3		1000	
		110							
								-	
								_	
	13.6								
			Sity SAND, light gray, medium dam	se to				6	
		1011	very dense with fine to medium grained, poorly graded, subarigular			4	Committee Contraction	D.	
		19	shall fragments			7	Gravelly SAND with		
			-,					24	
NO THEM I	15				UECT	ner A	L CP05 EAARS-CB C		

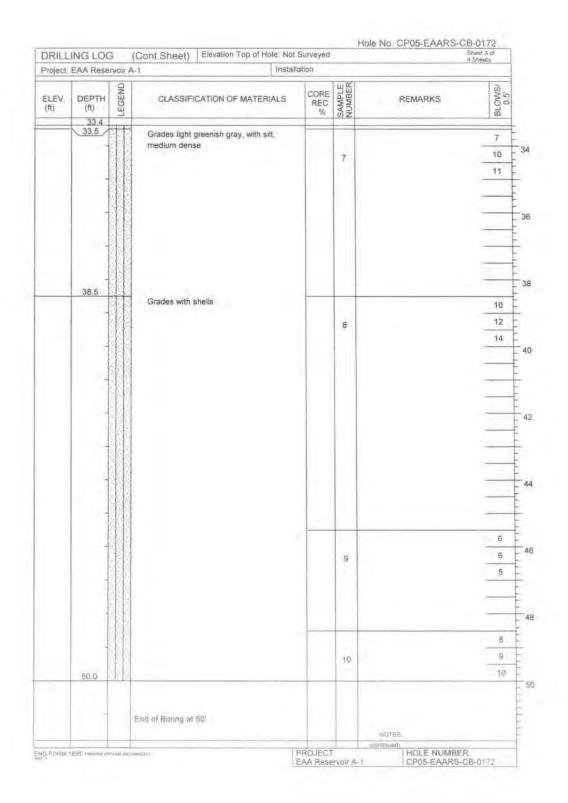


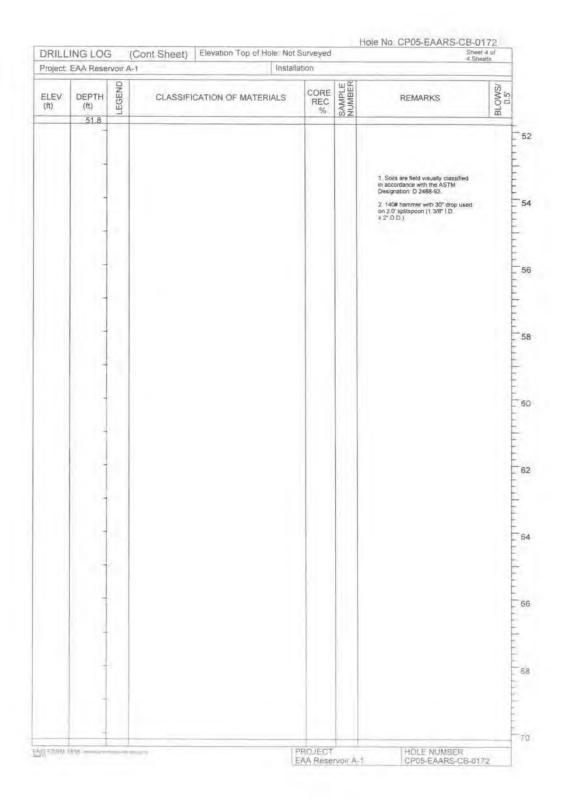


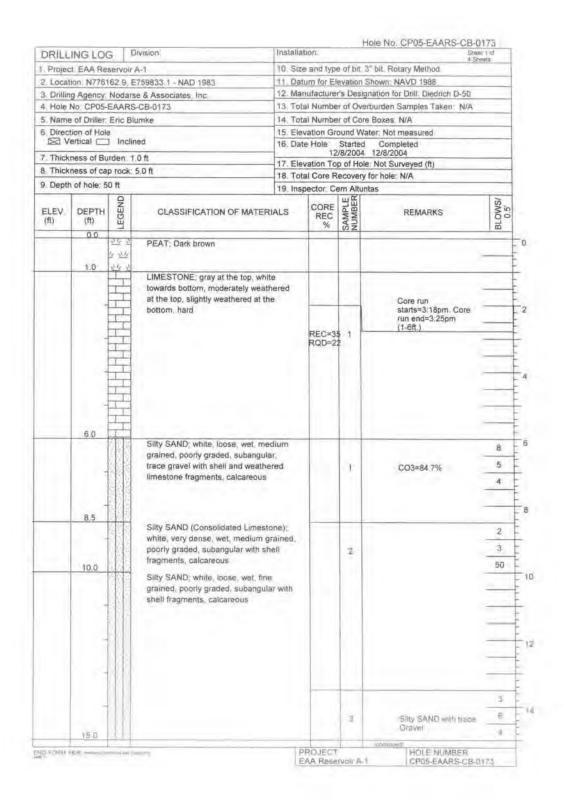


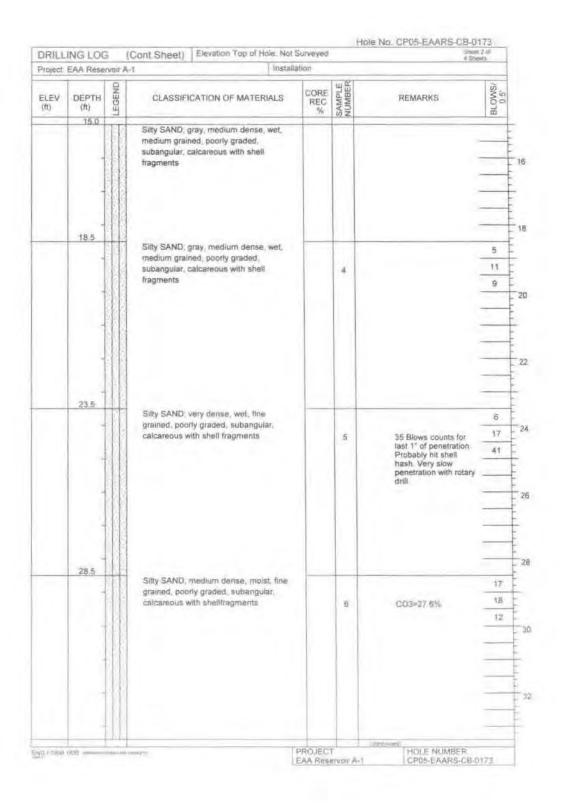


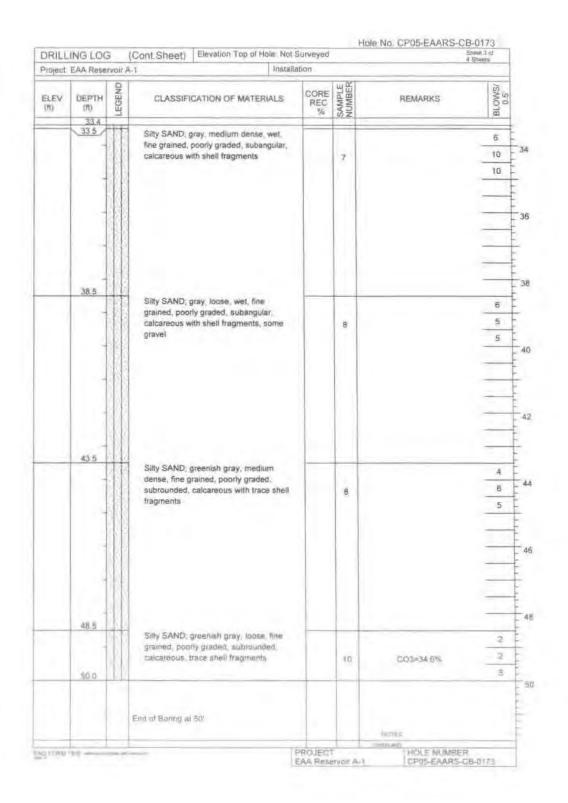


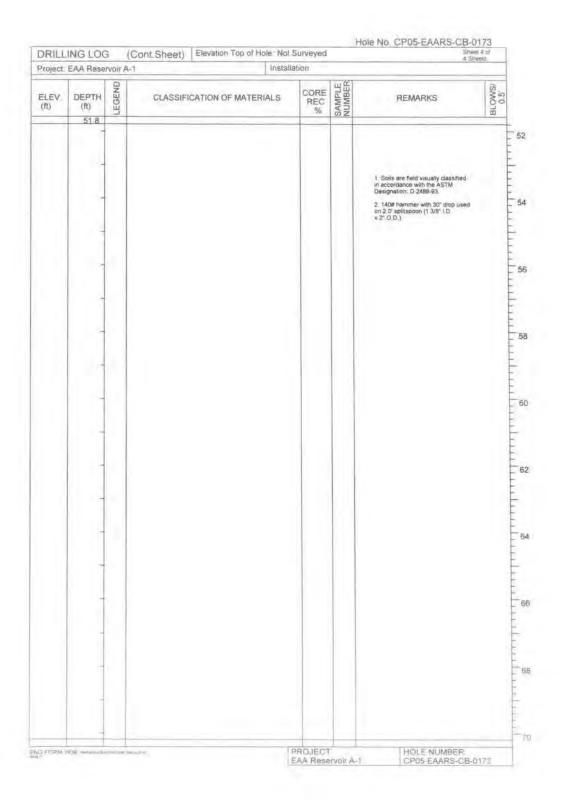




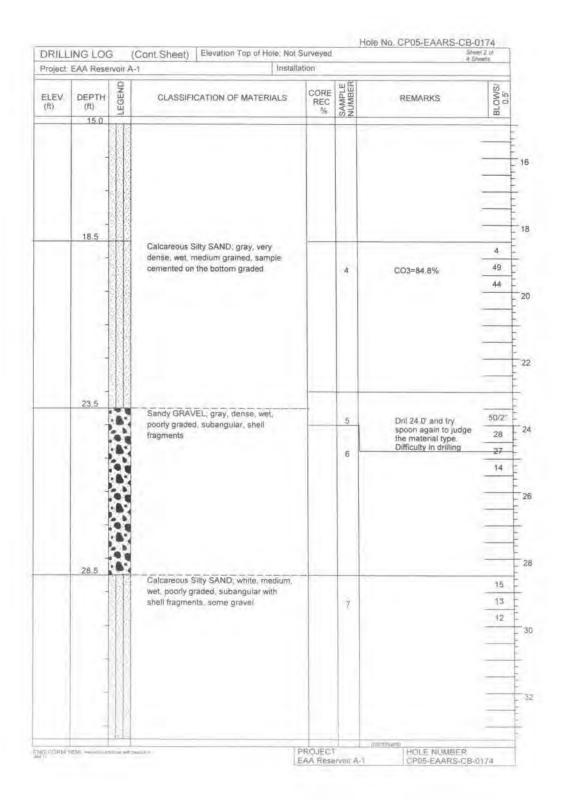


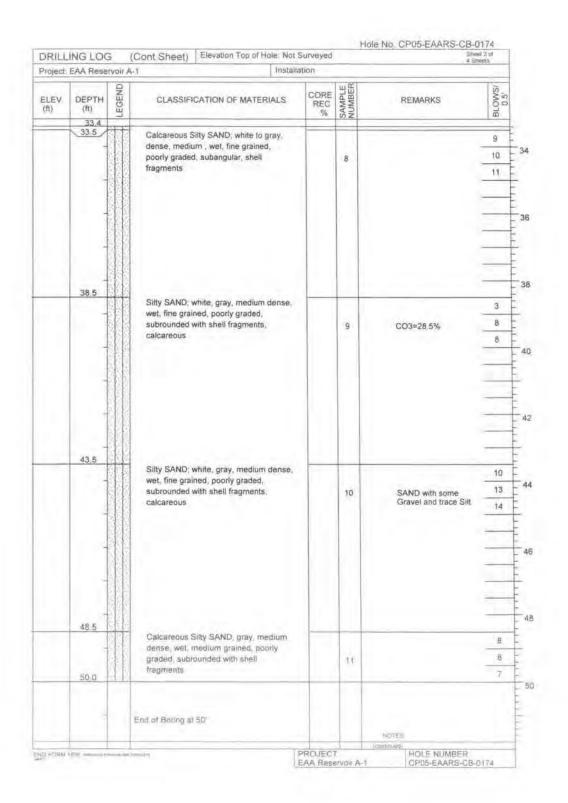


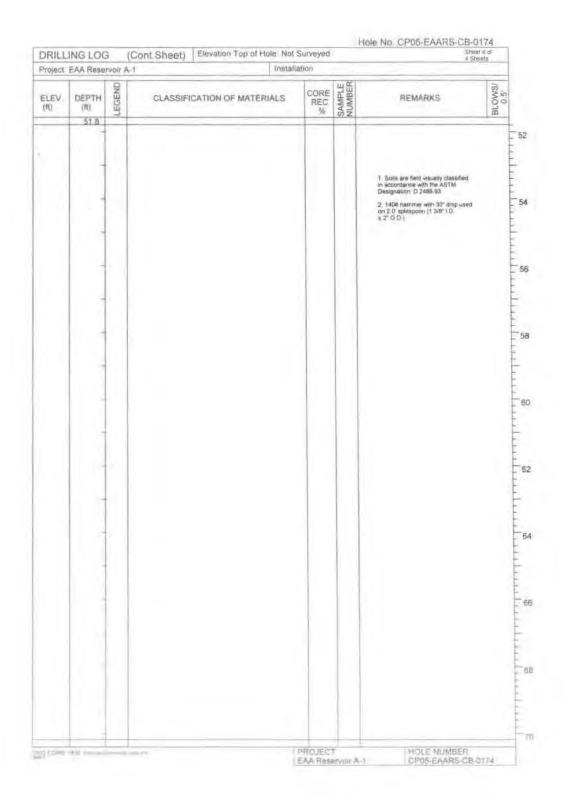




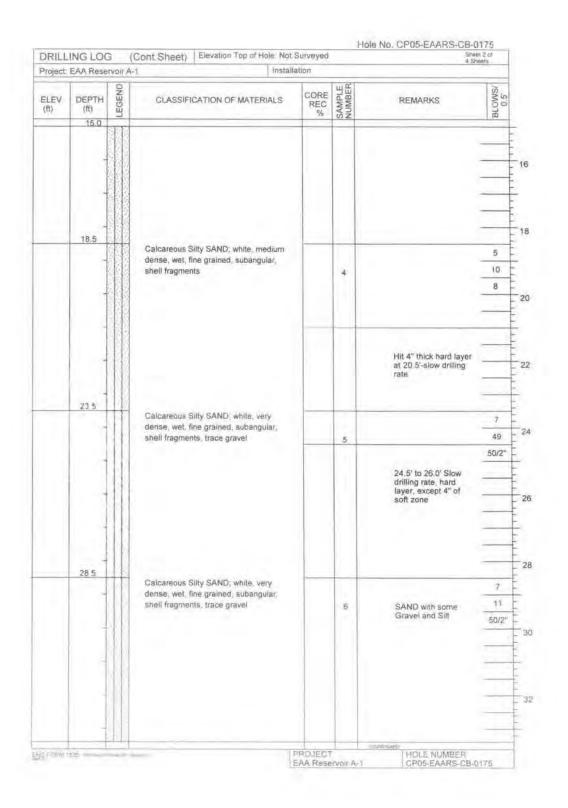
bed to be be	ING LO	3 D	ivision:	Installatio	in:		Hole No, CP05-EAARS-CB-01 Sheet 4 Sheet	Tipf	
f. Projec	t EAA Re	servoir A	4-1	10. Size and type of bit 3" bit, Rotary Method					
				11 Datum for Elevation Shown: NAVD 1988					
							gnation for Drill: Diedrich D-50		
	No: CP05-			13. Total Number of Overburden Samples Taken. N/A					
	e of Driller:						e Boxes: N/A		
	tion of Hole ertical			15, Elevation Ground Water, Not measured 16. Date Hole Started Completed					
7. Thick	ness of Bu	rden 1.	0 ft	17 Fleva			12/9/2004 e: Not Surveyed (ff)		
8. Thick	ness of ca	p rock: 5	S O FF	17. Elevation Top of Hole: Not Surveyed (ft) 18. Total Core Recovery for hole: N/A					
9. Depth	n of hole: 5	Oft	-	19. Inspe		m Altur			
ELEV.	DEPTH (ft)	LEGEND	CLASSIFICATION OF MATERIA	LS	CORE REC %	SAMPLE	REMARKS	BLOWS/	
	0.0				70	0Z		0	
	1.0	70 97 7 97 97 97	PEAT, Dark brown				Drilled with hand		
		H	LIMESTONE; gray, moderately weathered, vuggy and with shells, seams in yellow color, hard				sampler. Core run start =8:36am. Core run end=8:49am (1-6ft.)		
		吉			REC=32		(101.)	=	
		Ħ			RQD=7				
	1	1							
	100	I							
	6.0	1	Calcareous Silty SAND; white dense	0				-	
			wet, fine to medium grained, poorly					3	
			graded, subangular, trace gravel wit			1	CO3=83%	3	
			shell fragments					29	
	85	+	Calcareous Silty SAND, white to light	hi				23	
			gray, wet, fine to medium grained,			6		40	
			poorly graded, subangular, trace gra with shell fragments	avei		2		-	
			mor even negricina					17	
		ODE T							
			Calcareous Silty SAND; while to gra					ā	
			dense, well medium grained poorly	(
				(3	Difficulty in artilling,	4 5	

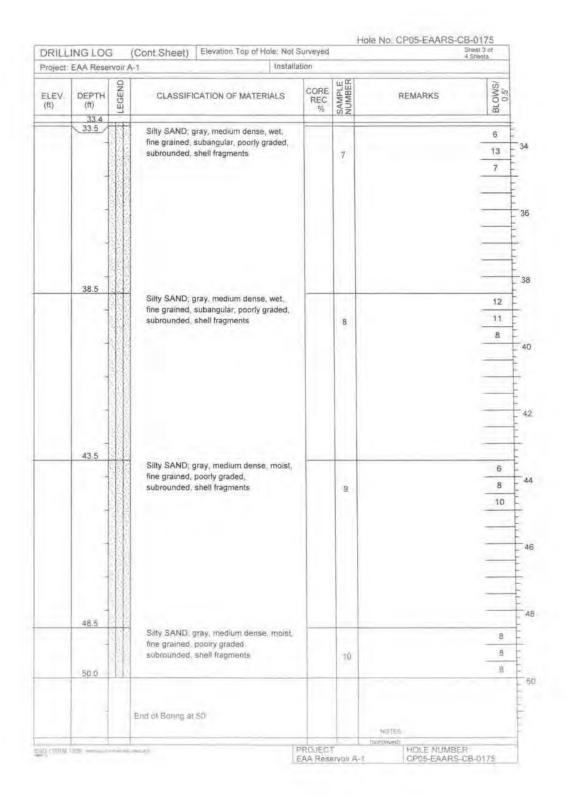


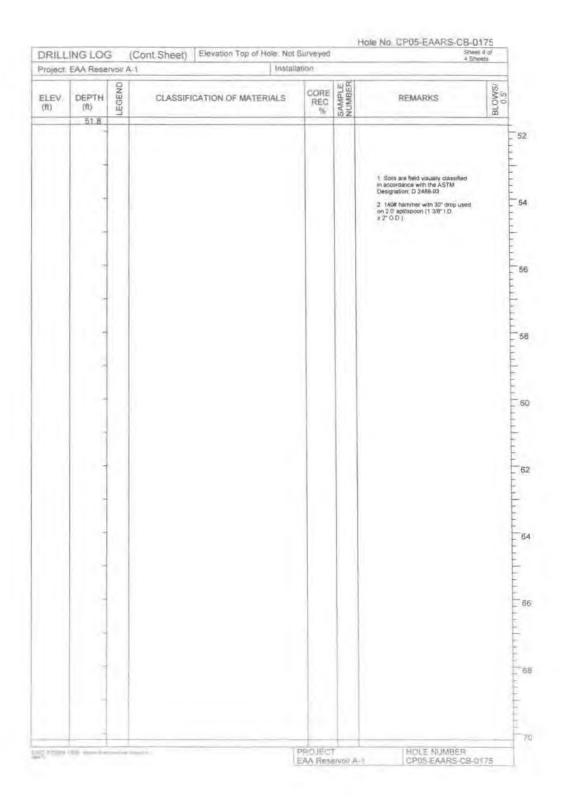


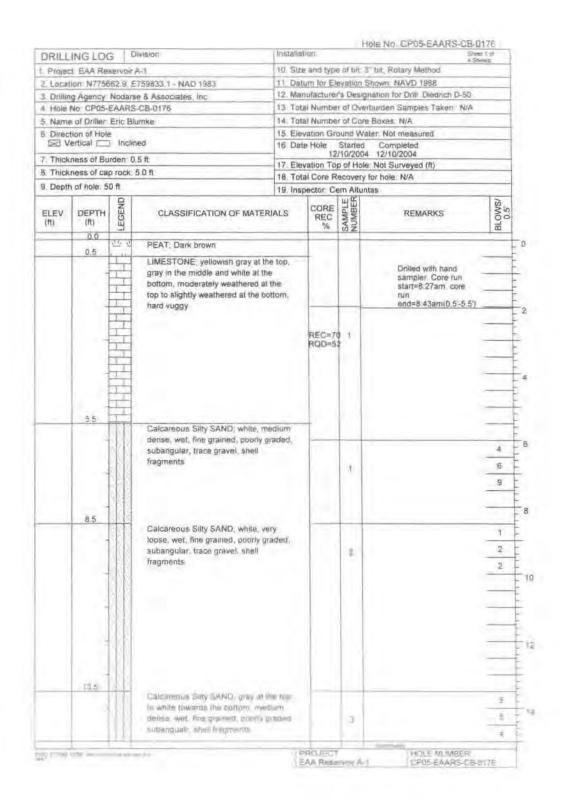


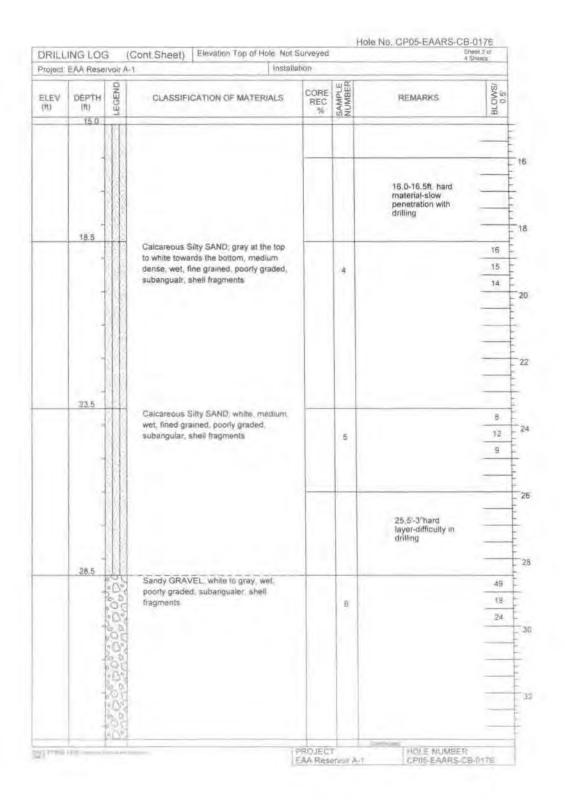
DRILL	ING LO	GE	livision	Installation Sheet f of a Sheet f of a Sheet f of a Sheets 10 Size and type of bit, 3" bit, Rotary Method 11 Datum for Elevation Shown: NAVD 1988					
1 Projec	EAA Re	servoir	A-1						
2 Local	tion: N775	662.9, E	759333.1 - NAD 1983						
			e & Associates, Inc.				nation for Drill Diedrich D-50		
4. Hole	No CP05	EAARS	-CB-0175	_			burden Samples Taken: N/A		
5 Name	e of Driller	Eric Bl	umke	14. Tot	al Number	of Core	Boxes: N/A		
	tion of Hol		Ou.	-			ter: Not measured		
≥⊴ v	ertical	T luciu	ied	16. Da		Started	Completed 12/9/2004		
7. Thick	ness of B	irden: 0	5 ft	17 Fie			: Not Surveyed (ft)		
8. Thick	ness of ca	p rock:	5.0 ft				for hole: N/A		
9. Depti	n of hole:	50 ft		-	pector: Ce	m Altunt	Committee of the Commit		
ELEV.	DEPTH (ft)	LEGEND	CLASSIFICATION OF MATE	RIALS	CORE REC %	SAMPLE	REMARKS	BLOWS/ 0.5'	
	0.0	31. 3	DEAT: Ded brown						
	0.5	- 12	PEAT; Dark brown						
LIMESTONE, yellowish gray at the middle and white at the vuggy, hard, moderately wea			LIMESTONE; yellowish gray at t at the middle and white at the bo vuggy, hard, moderately weathe the top to slightly weathered at t bottom	ottom, red at			Drilled with hand sampler. Core run start=1:17pm. Core run end=1:32pm (0.5-5.5ft.)		
					REC=67 RQD=52				
	5.5		Calcareous Silty SAND, white, in dense, wet, fine grained, poorly subangualer, shell fragments, se gravel	graded,		1		5 7	
	8.5 Calcareous Silty SAND; white	Calcareous Silty SAND; white, le					2		
	wet, fine grained, poorly graded, subangular, shell fragments, trace gravel, bottom 5" is plastic silty SANE gray, fine grained			ce		2	CO3=85.9%	3	
13.5	13,5								
		1.00	Calcareous Silty SAND, white, o wer, fine grained, poorly graded					3.	
			subangular shell fragments so			3	CO3=81.7W	3	
	()///		mayer shell fragments some				703=KI 7W	-	
		1797	TIL5 A 441					30	

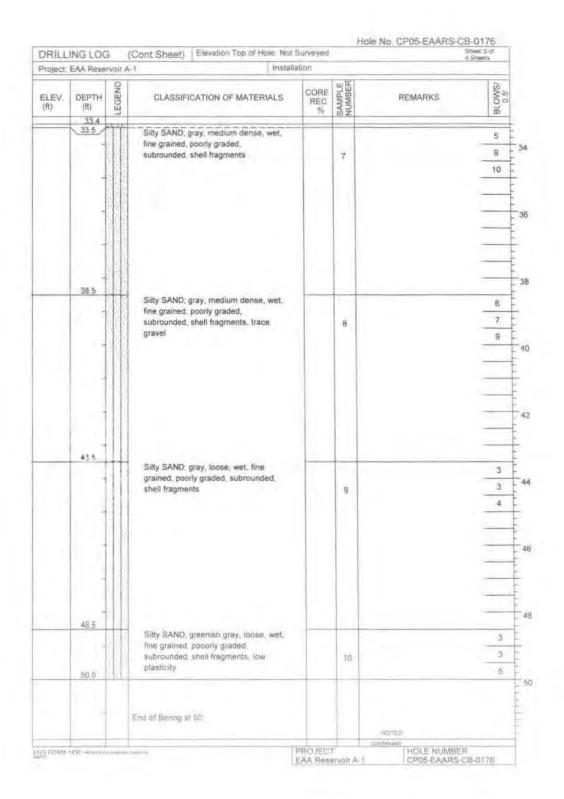


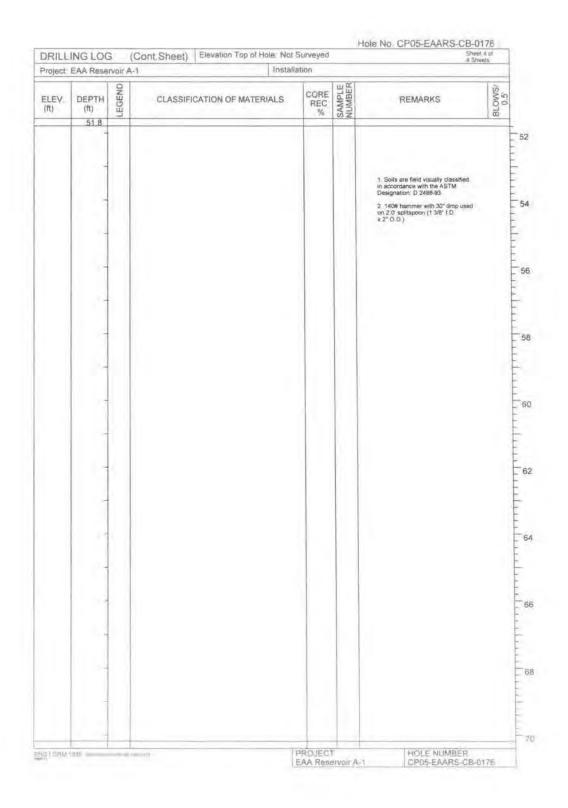


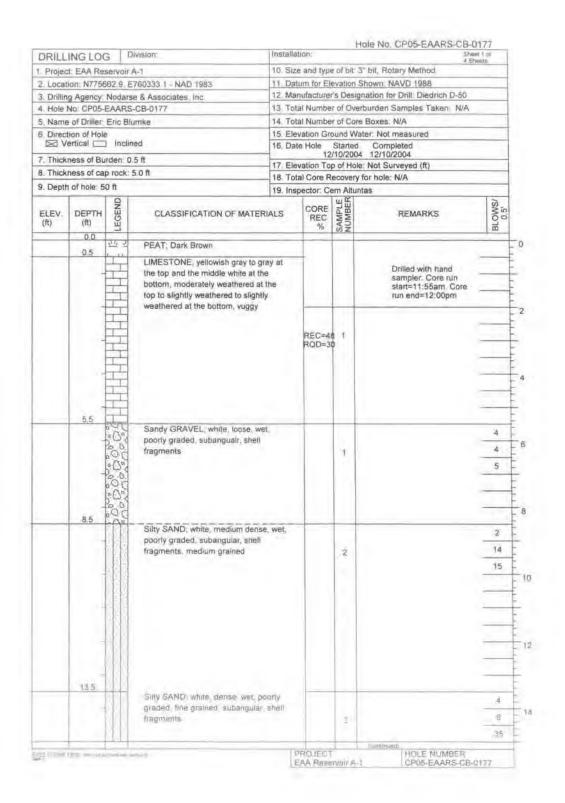


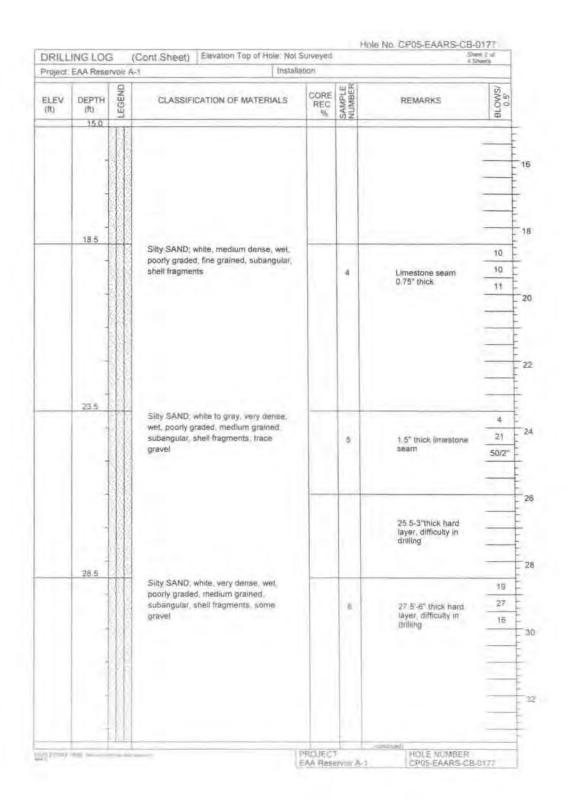


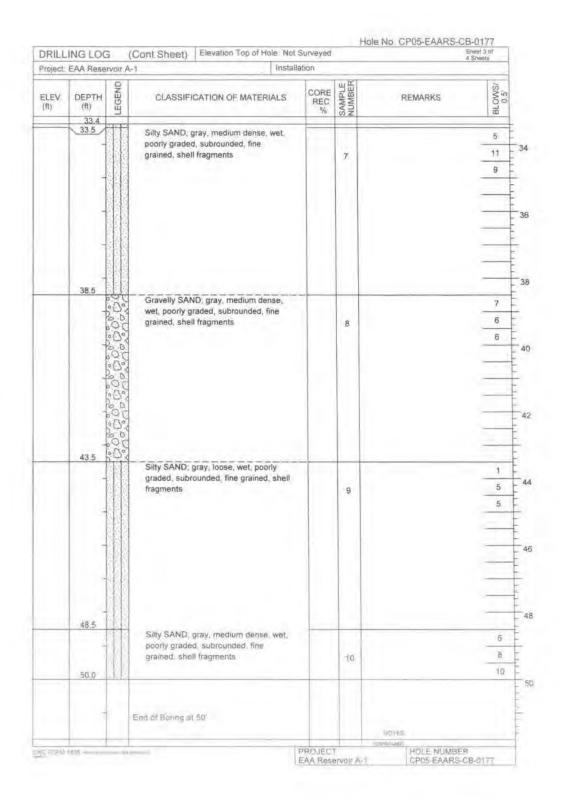


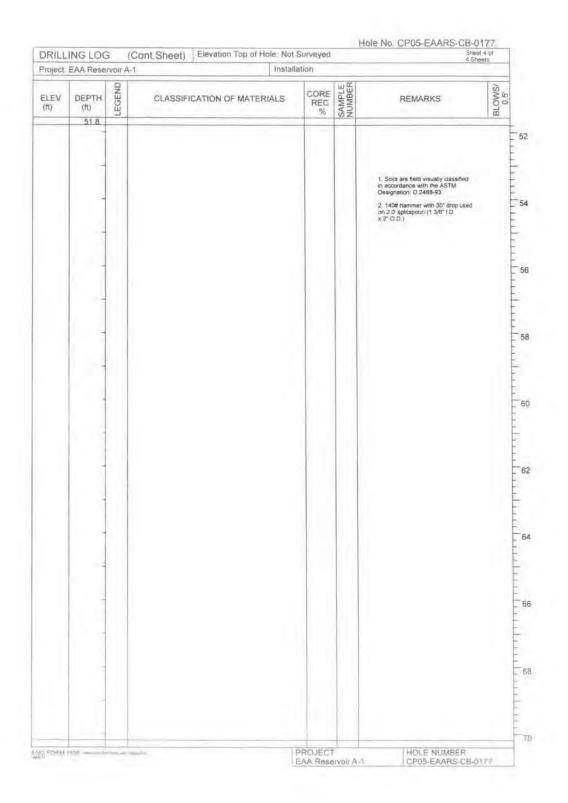


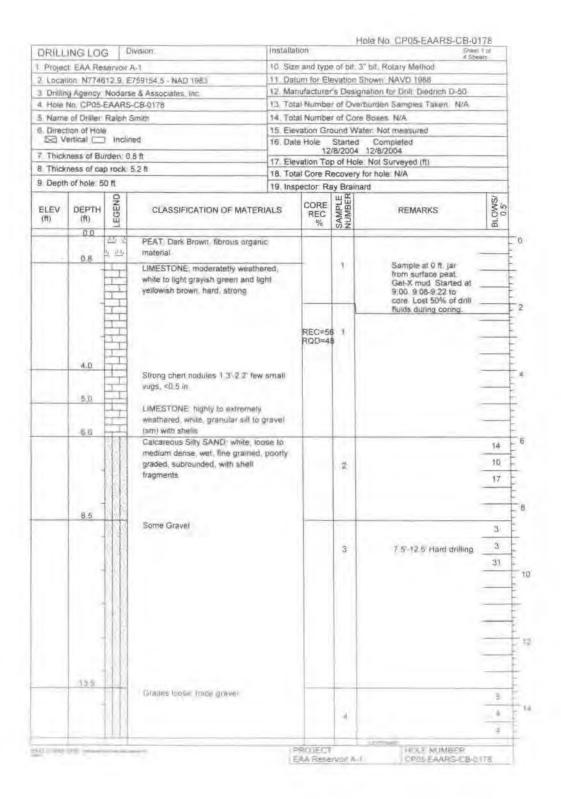


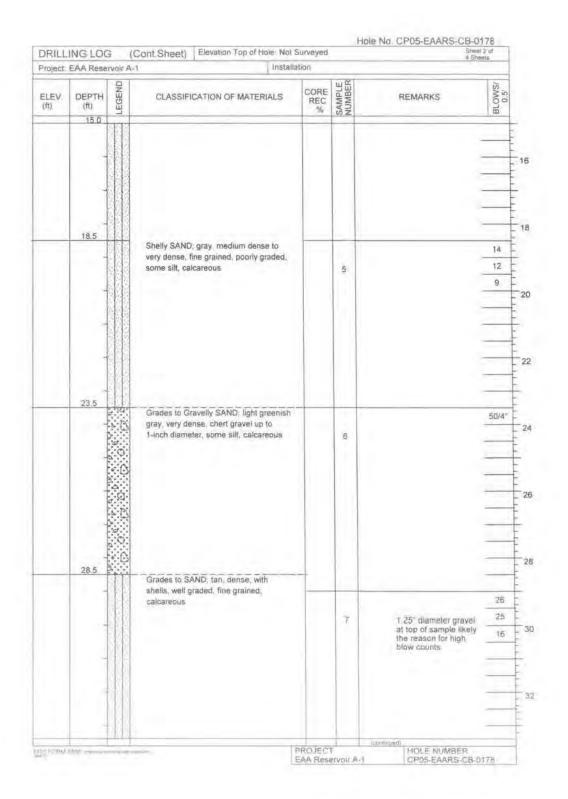


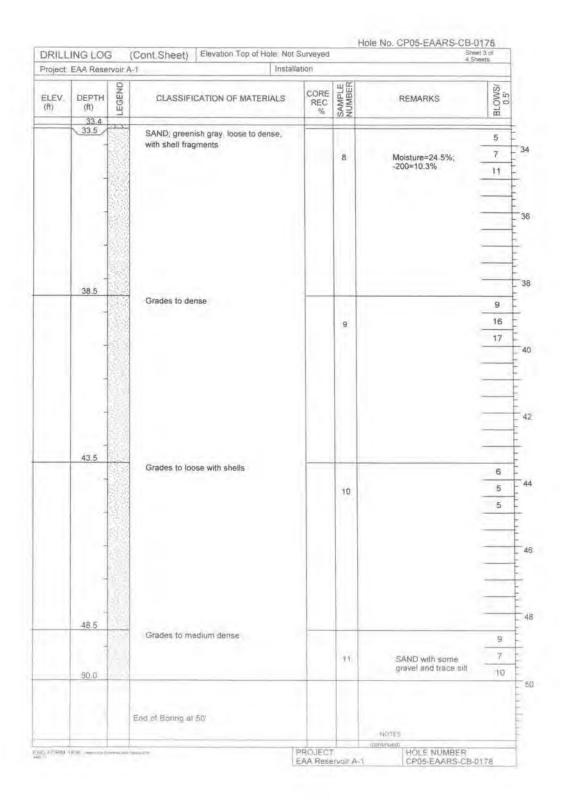


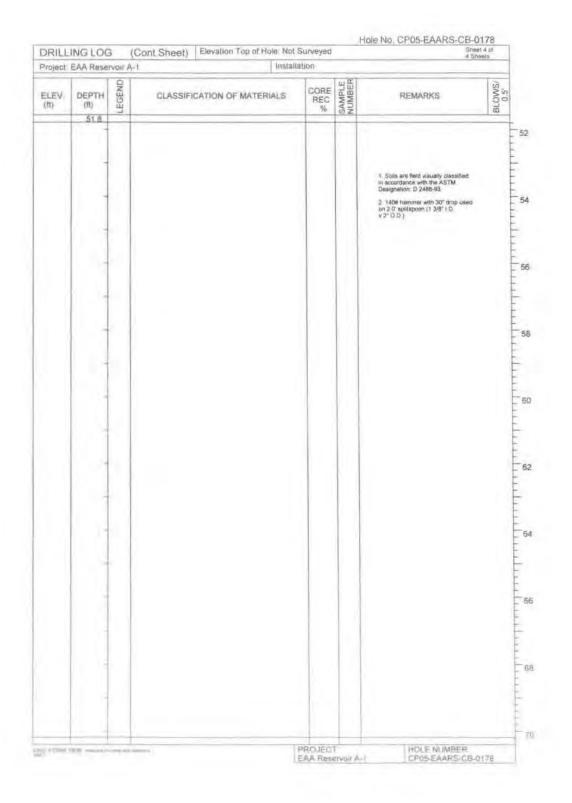


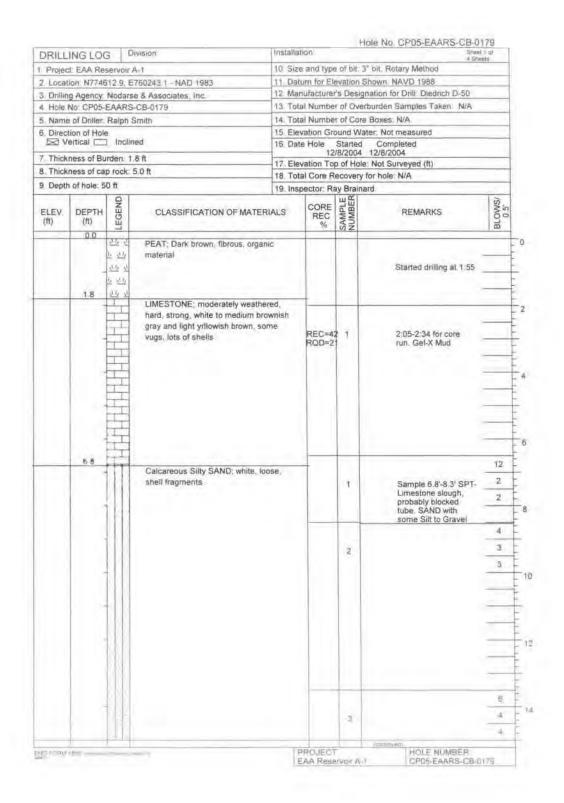


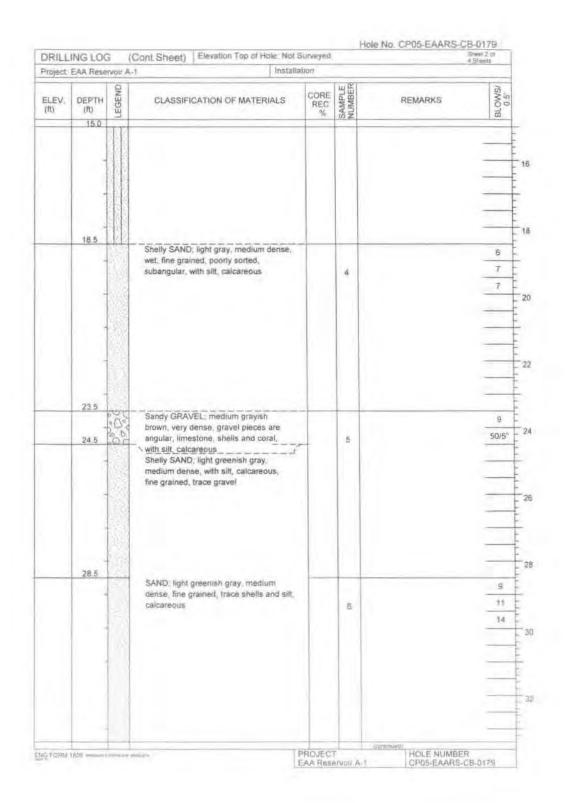


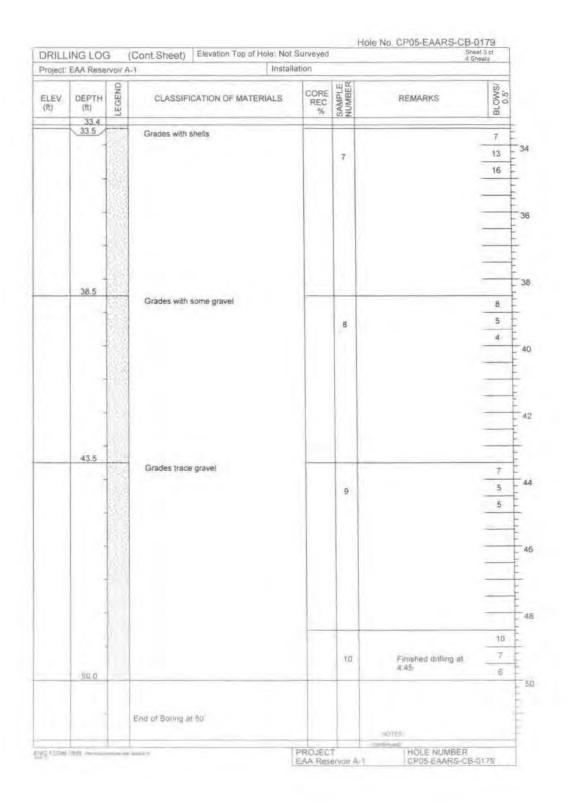


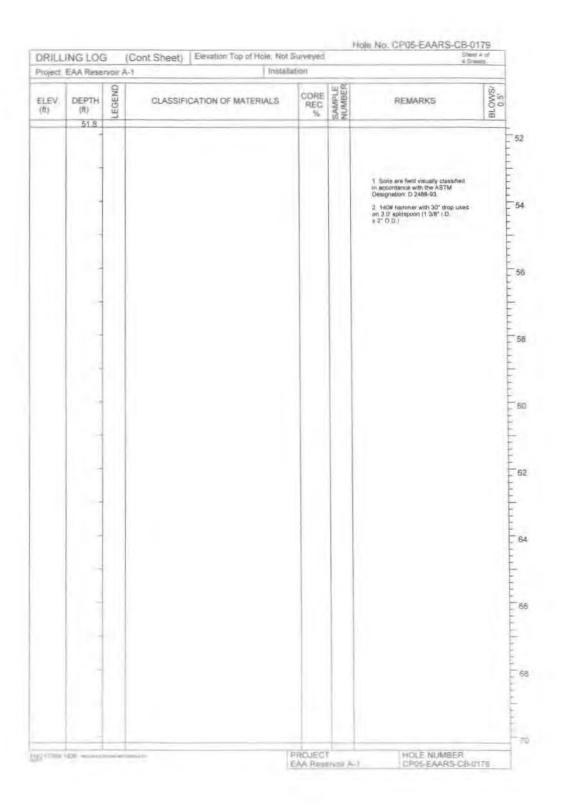


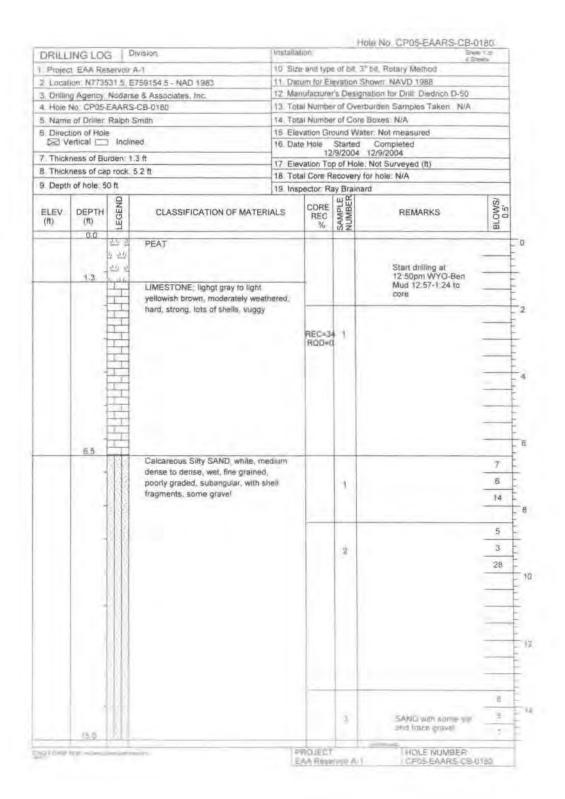


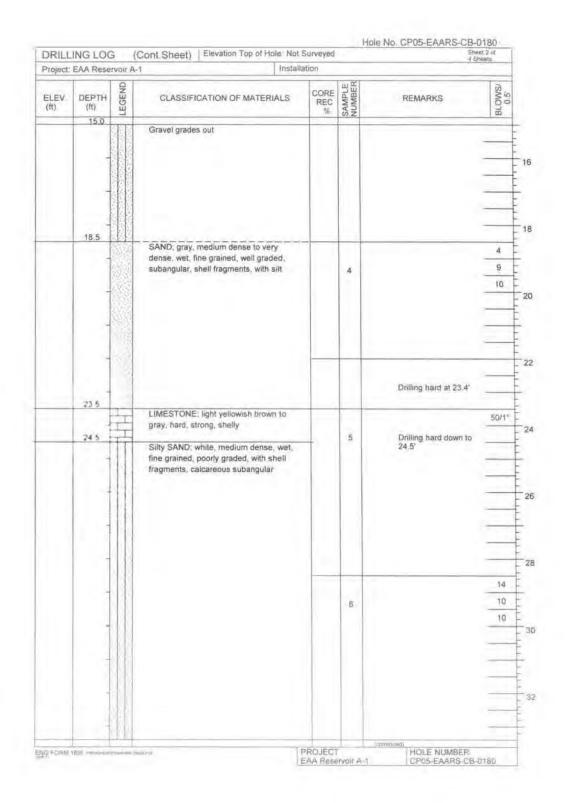


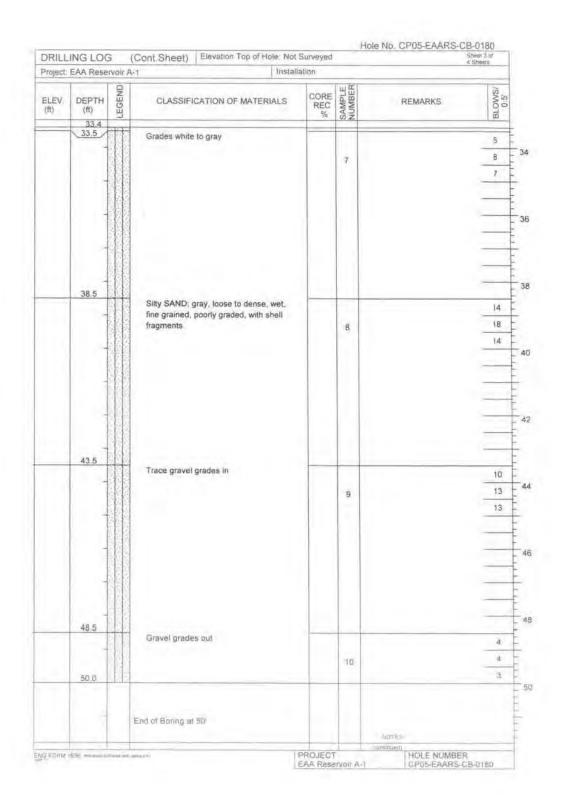


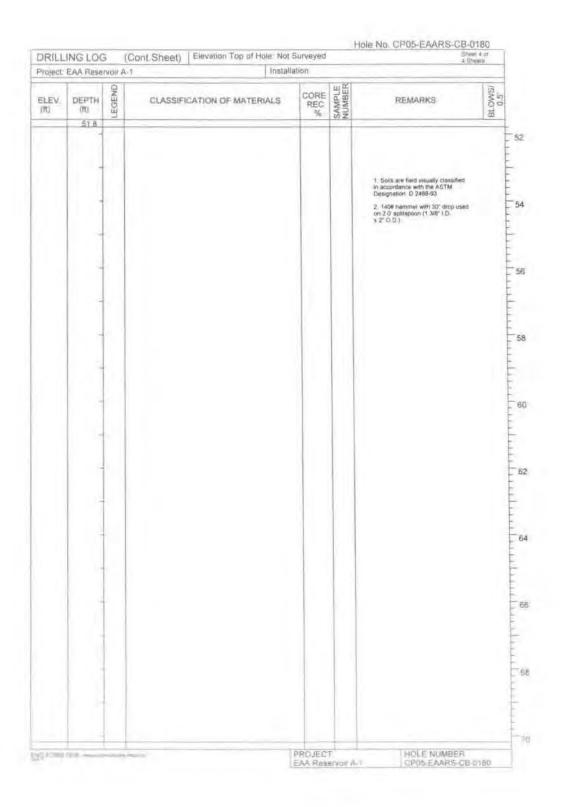




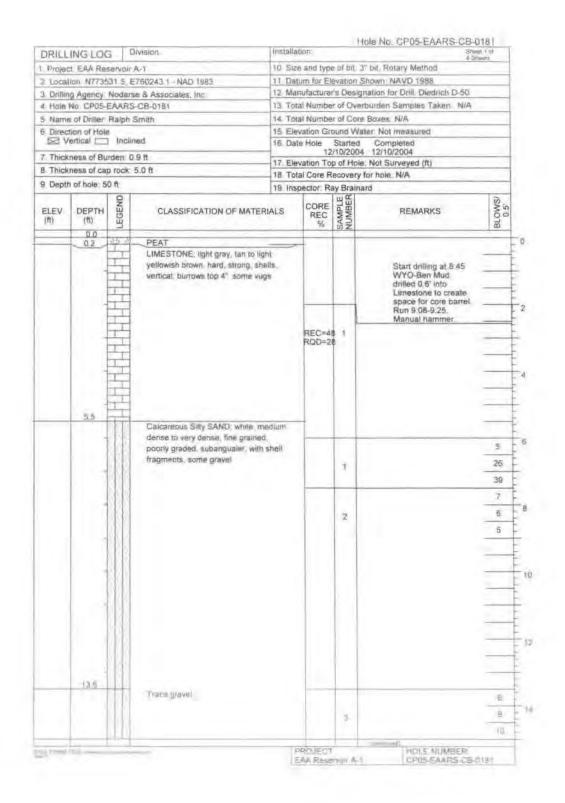


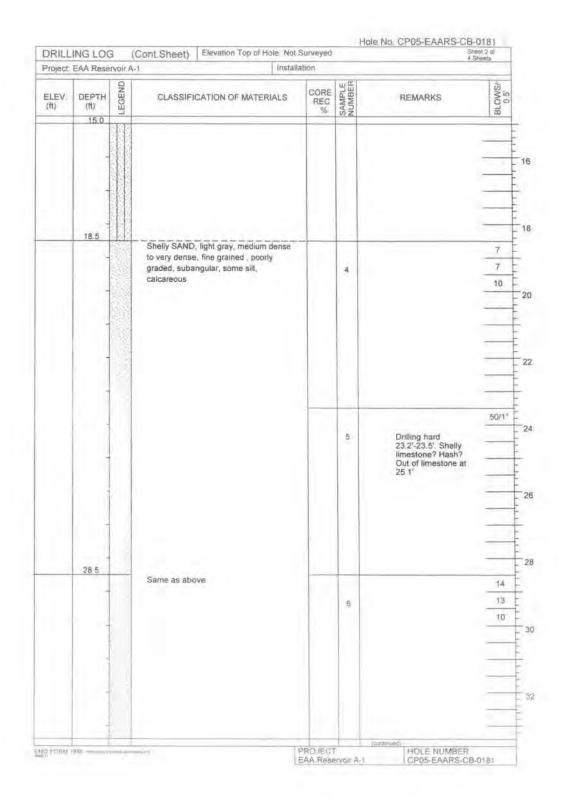


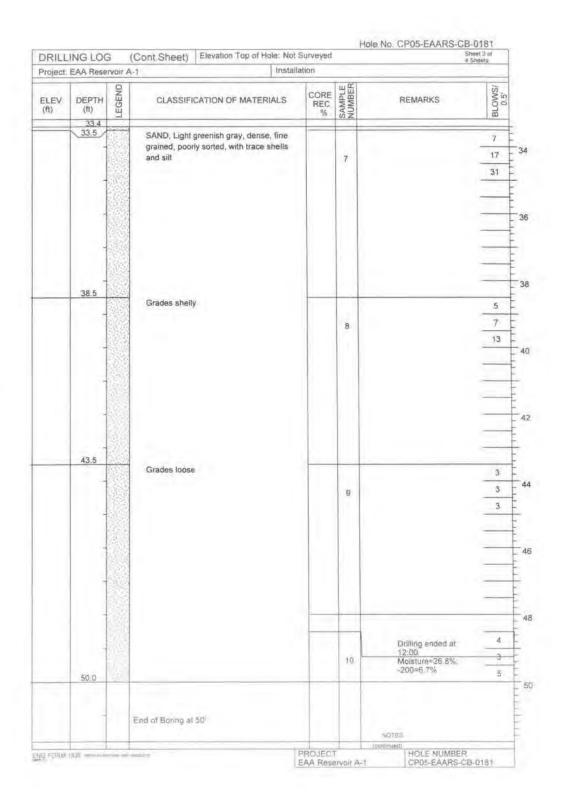


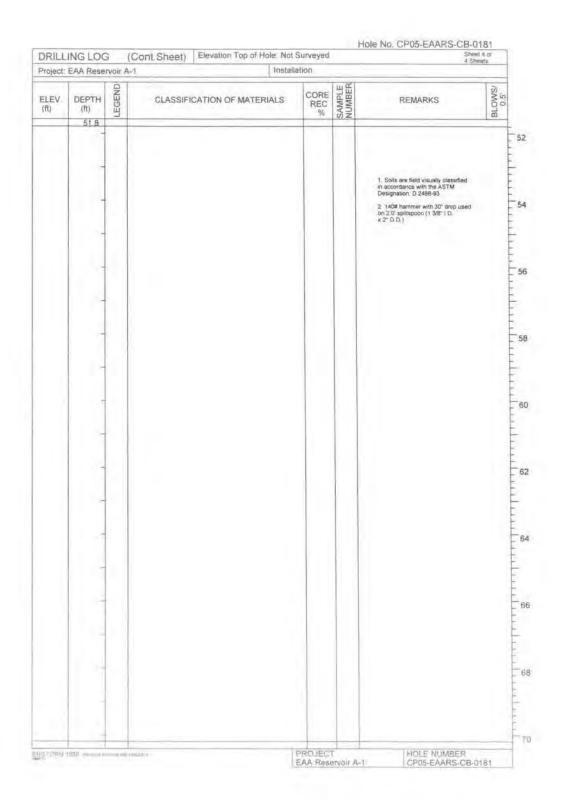


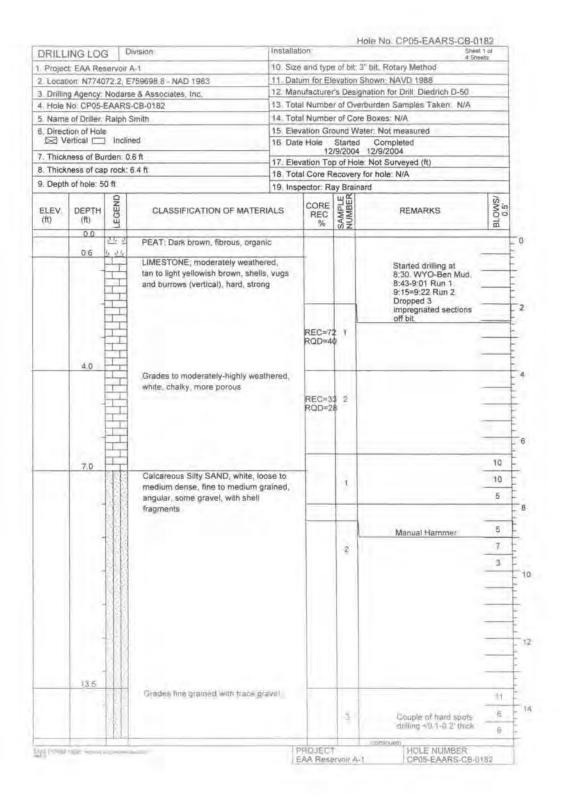
APPENDIX 1 TEST CELL BORINGS AND PIEZOMETER INSTALLATION LOGS: 181-200

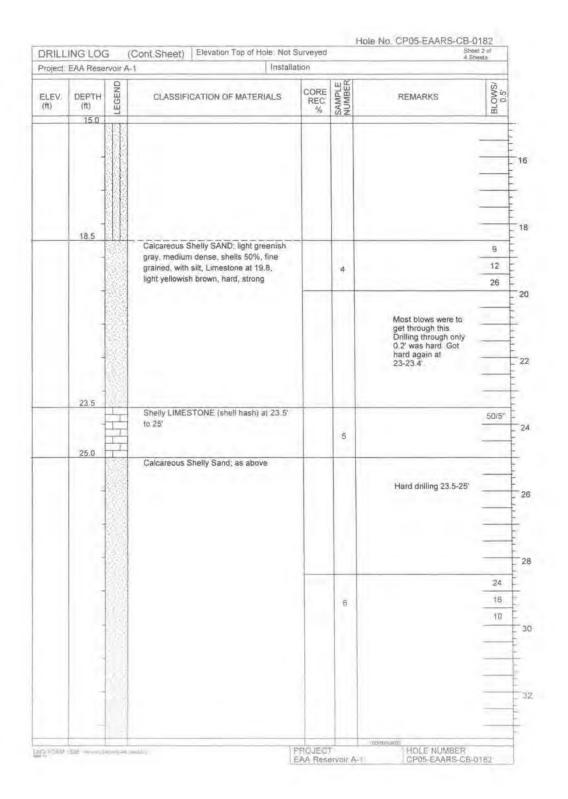


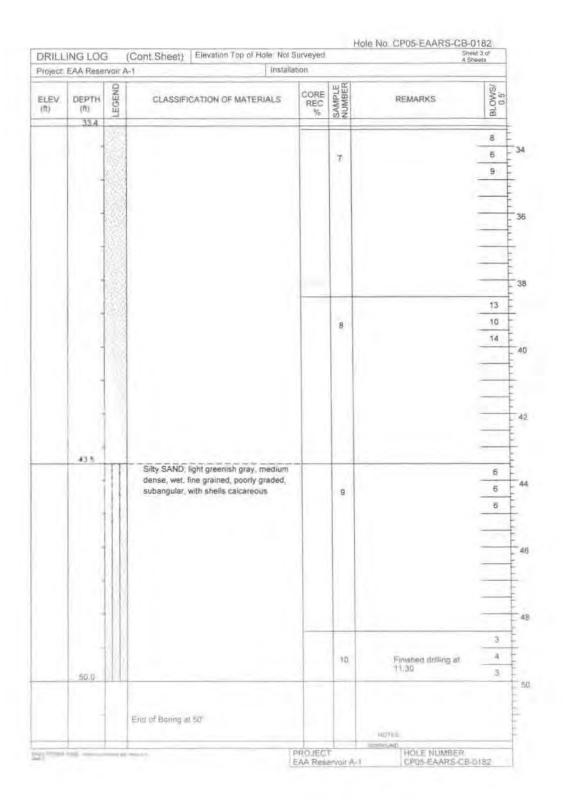


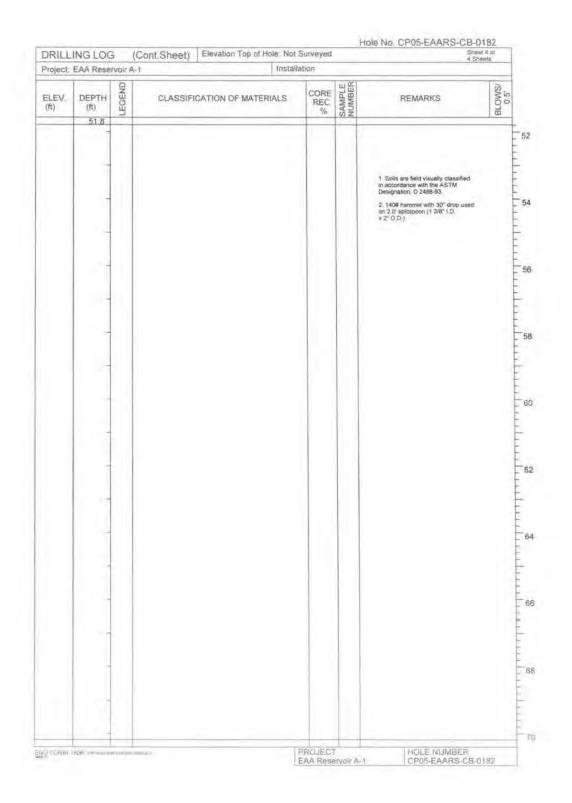


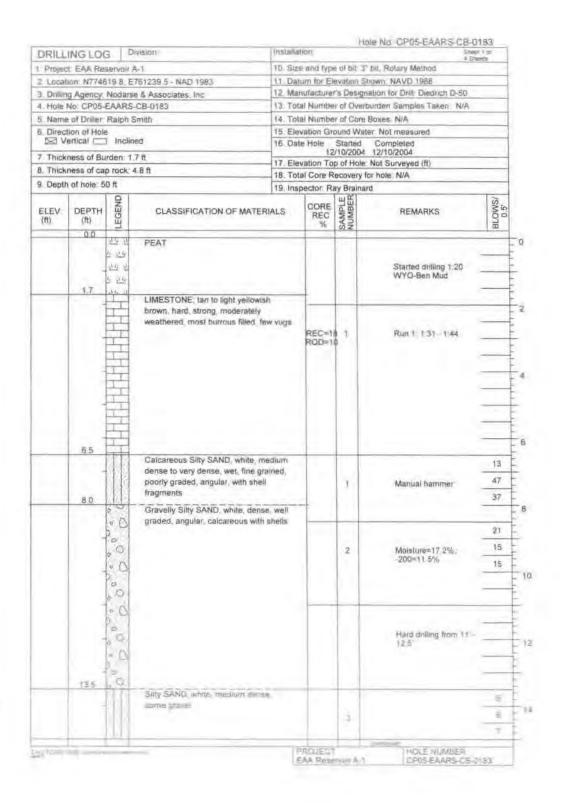


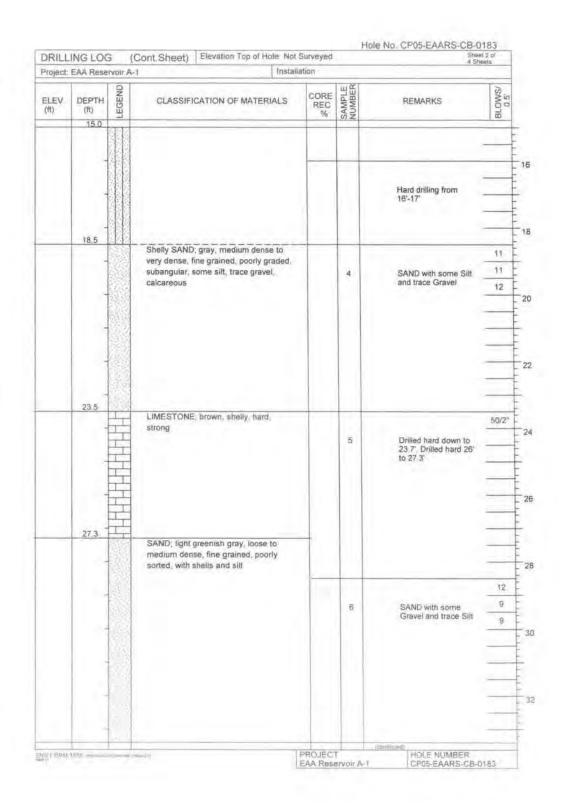


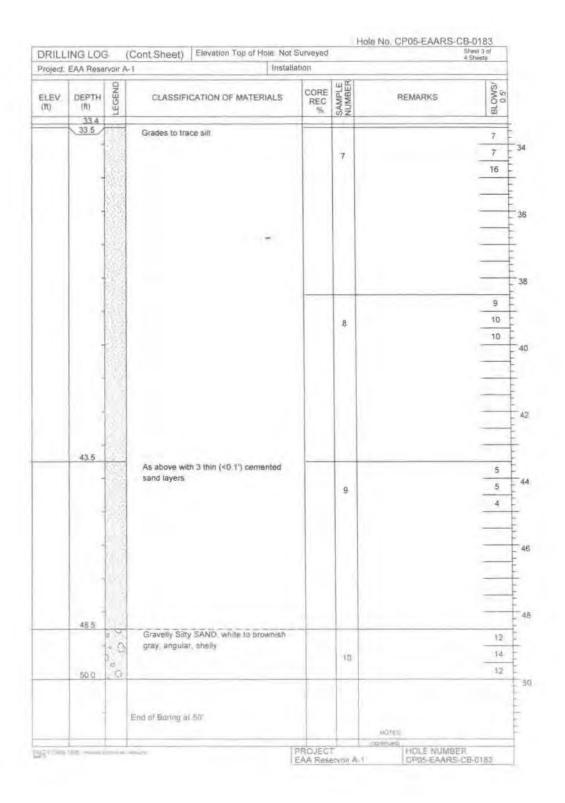


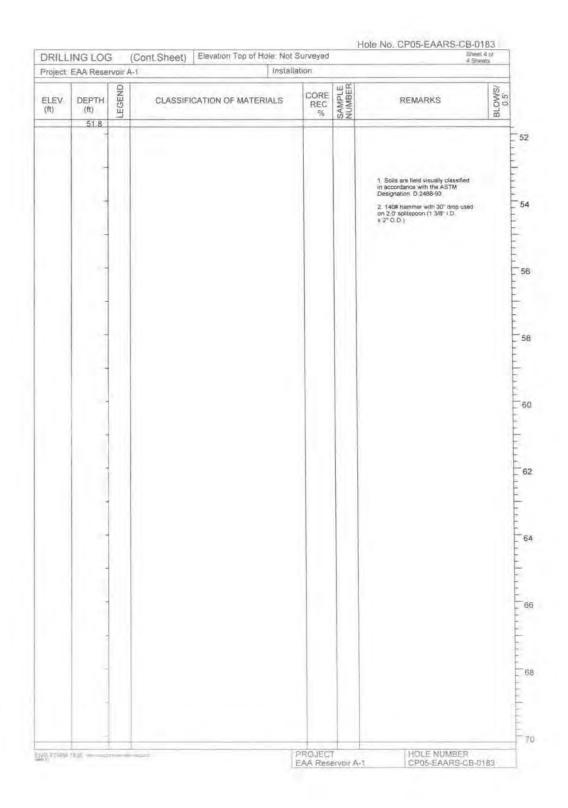


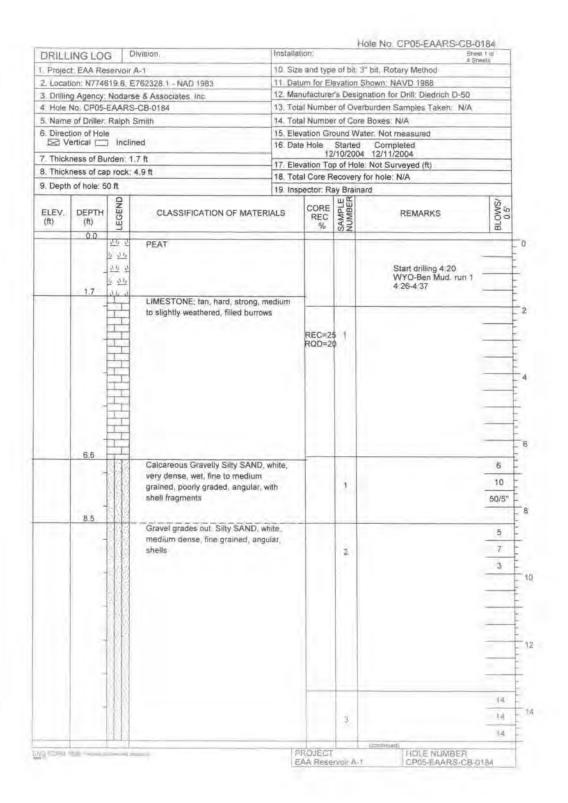


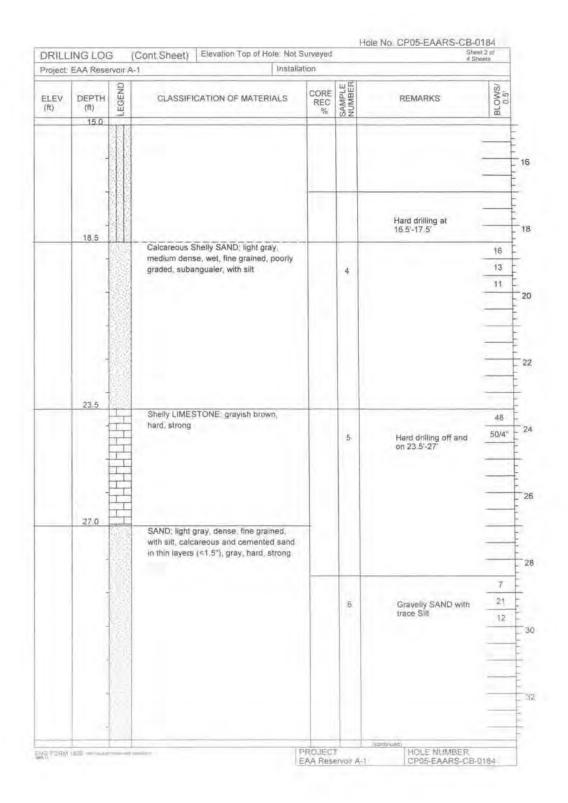


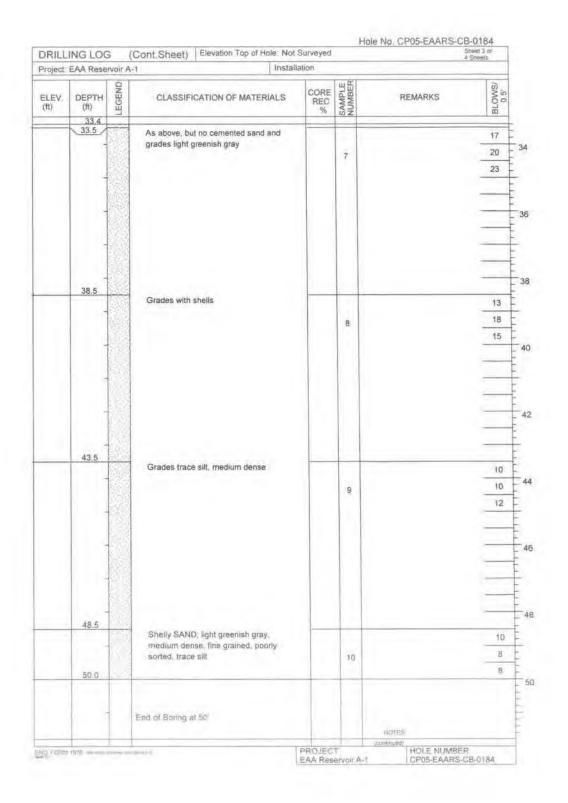


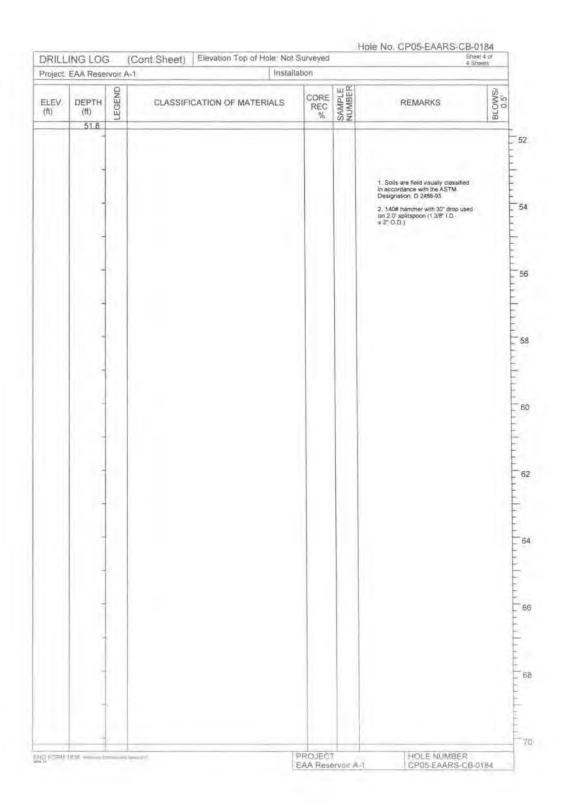


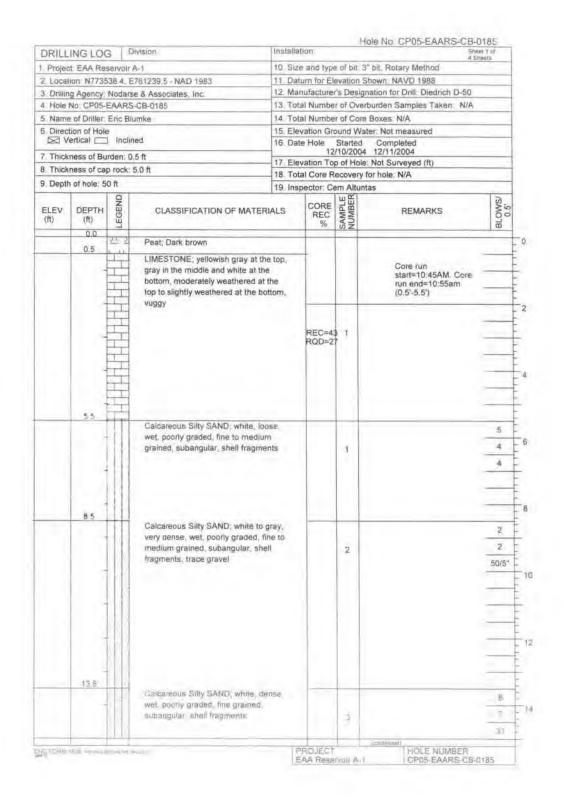


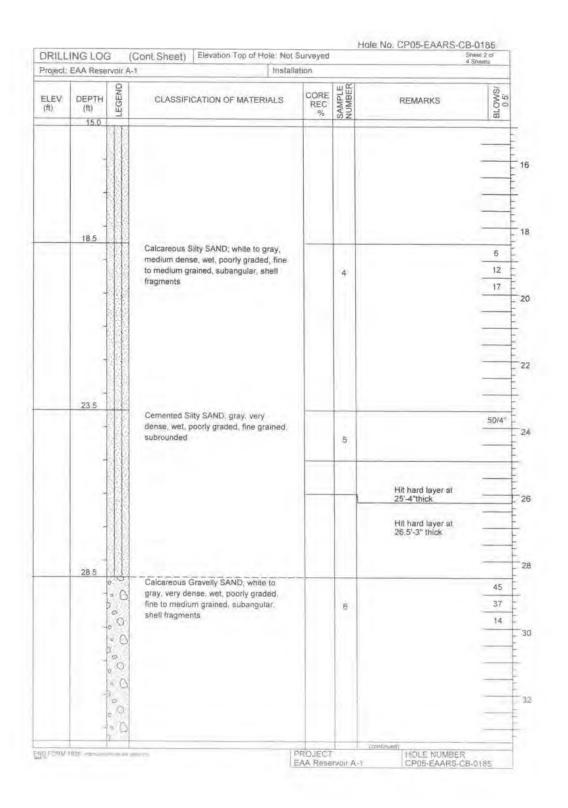


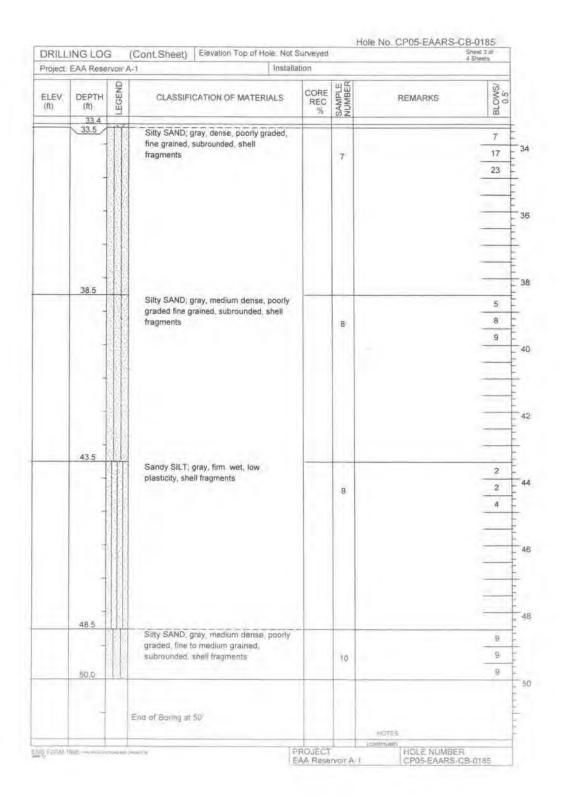


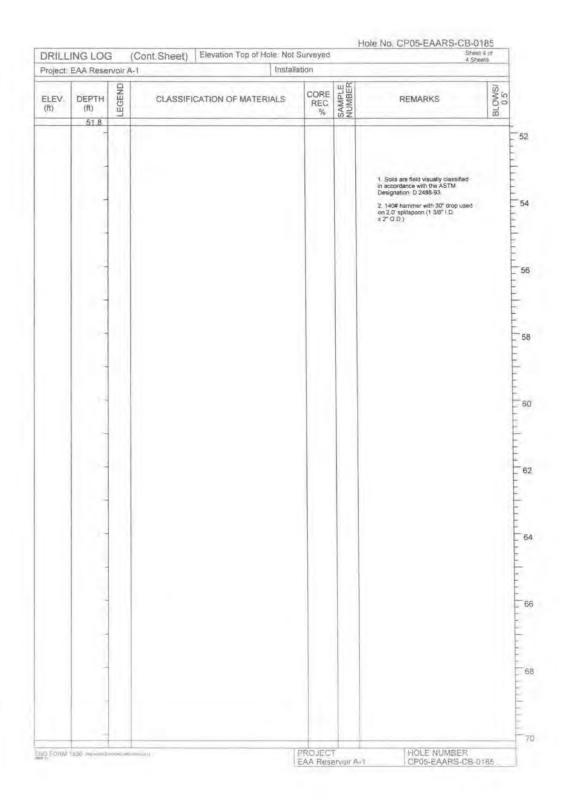


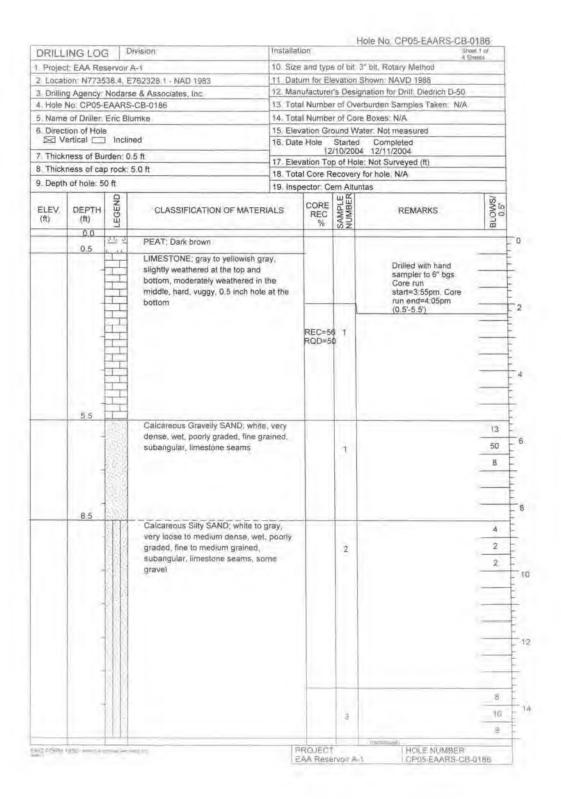


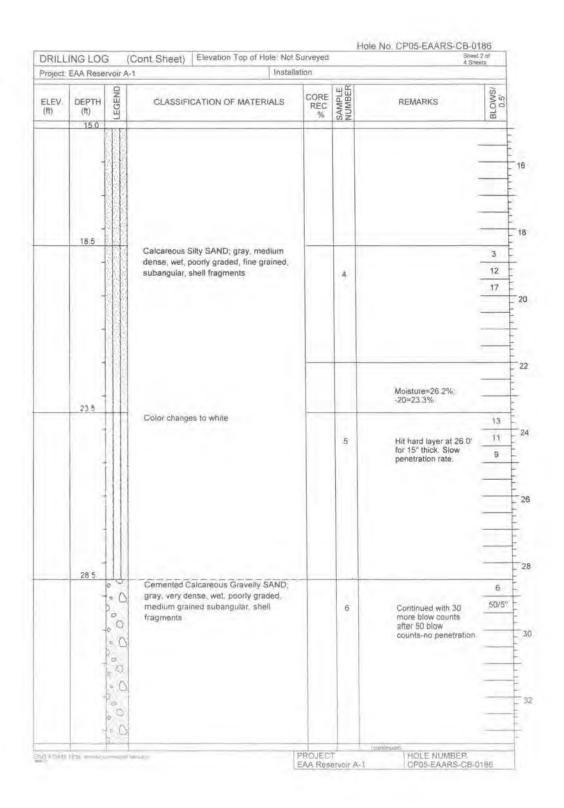


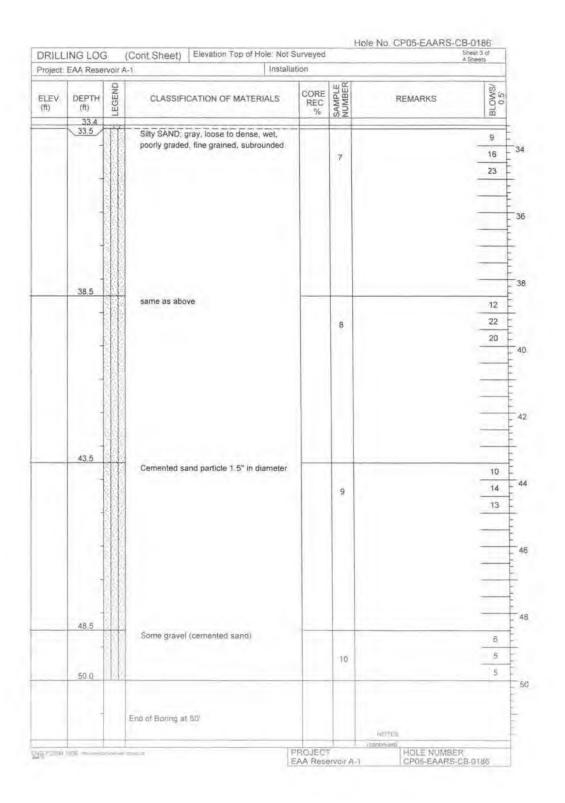


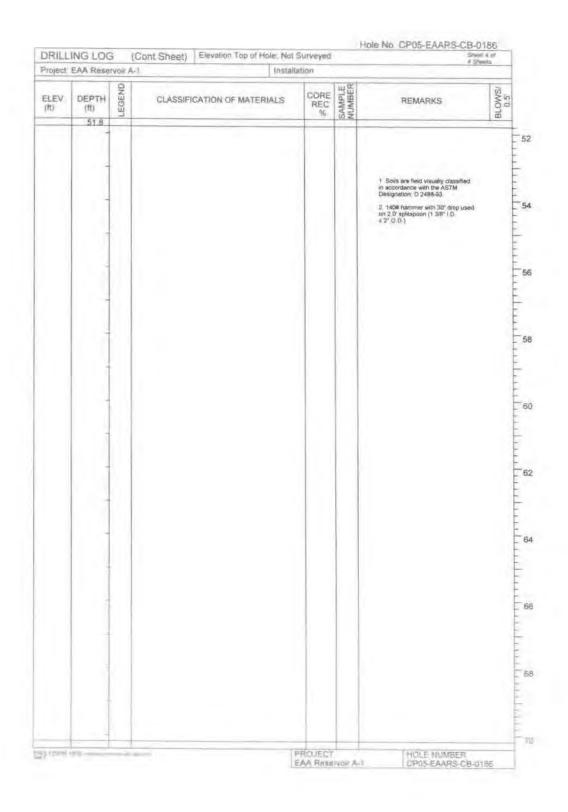


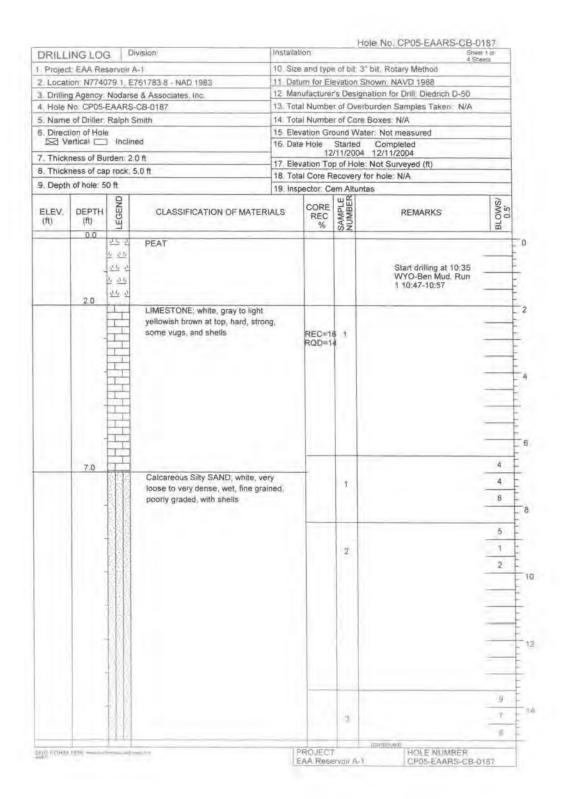


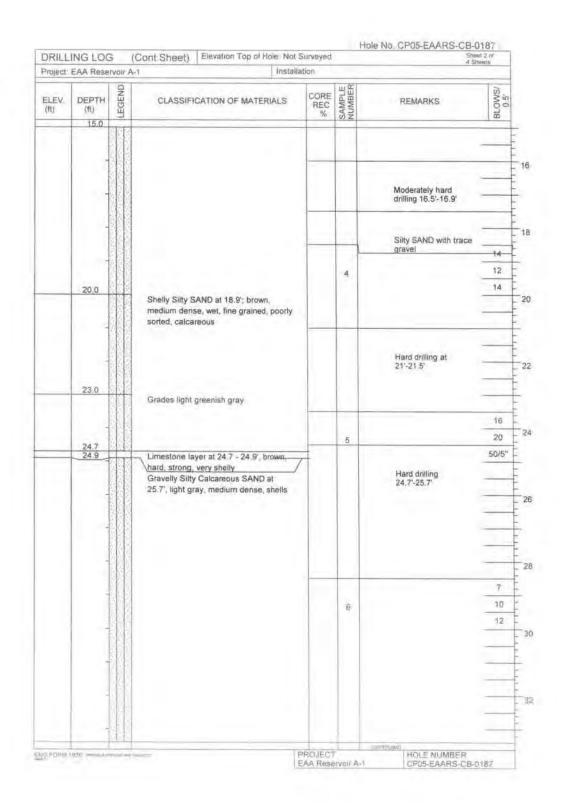


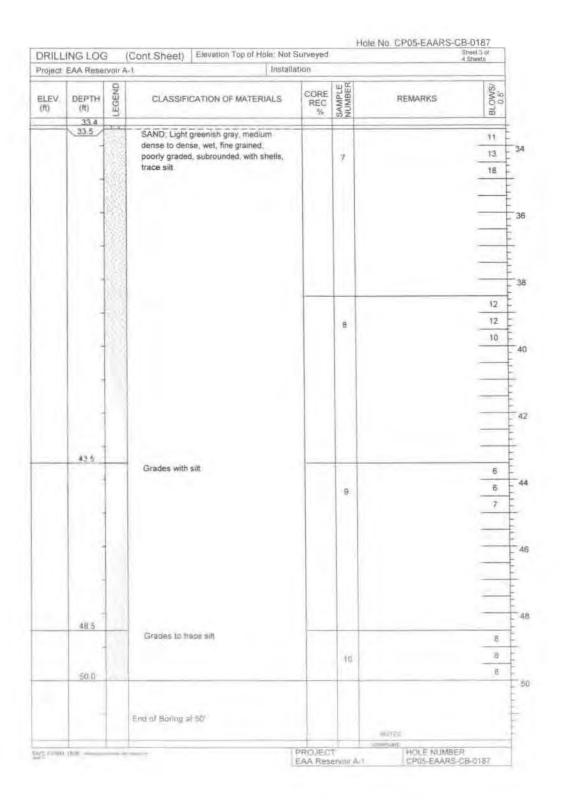


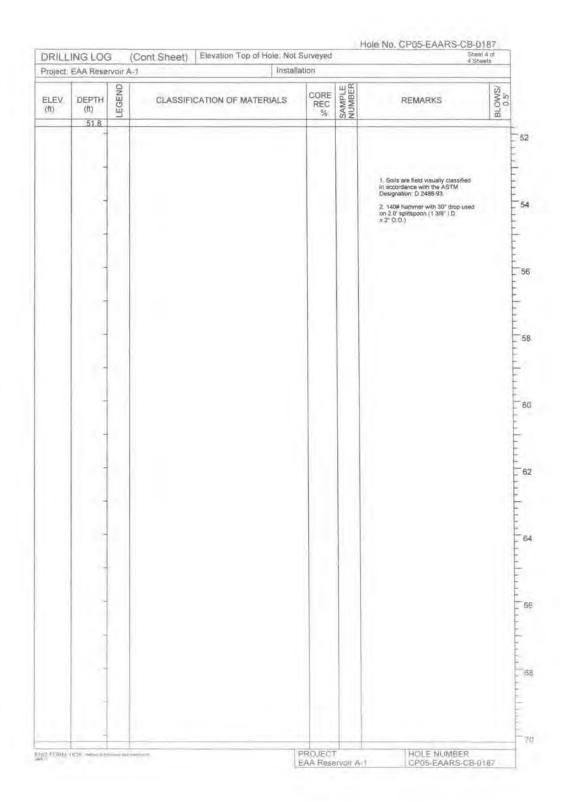


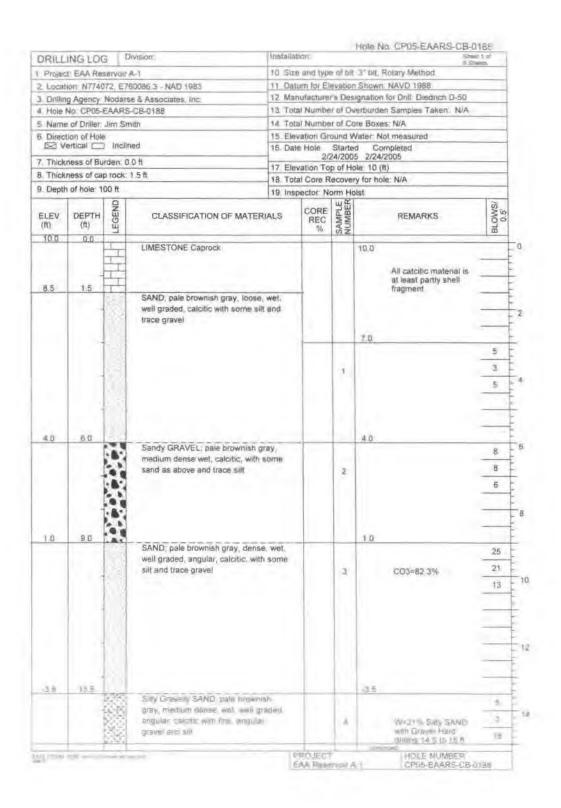


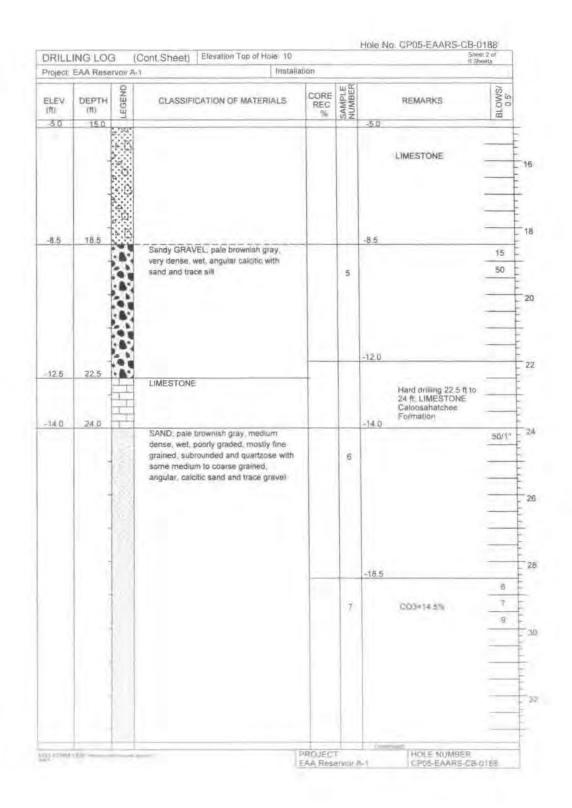


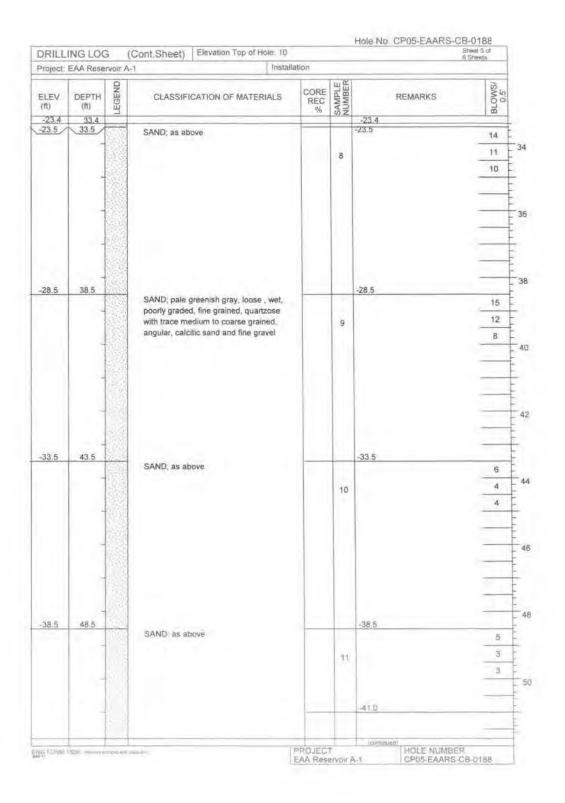


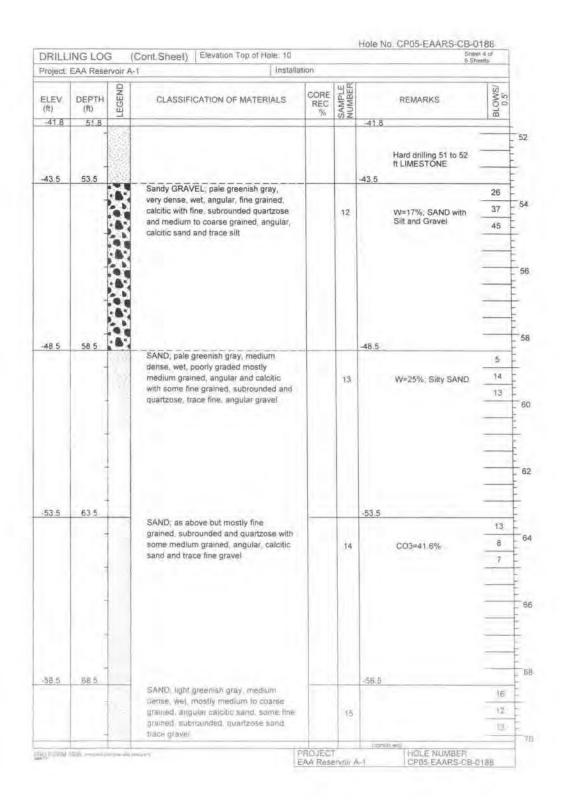


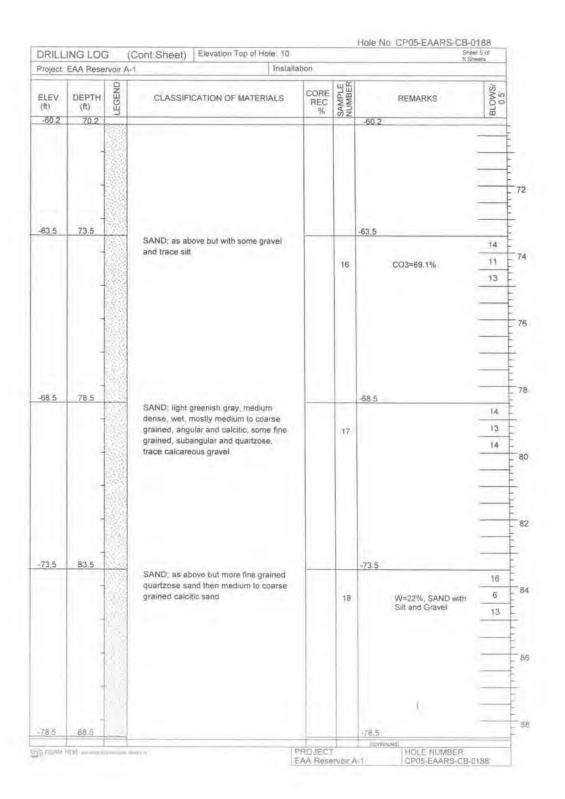


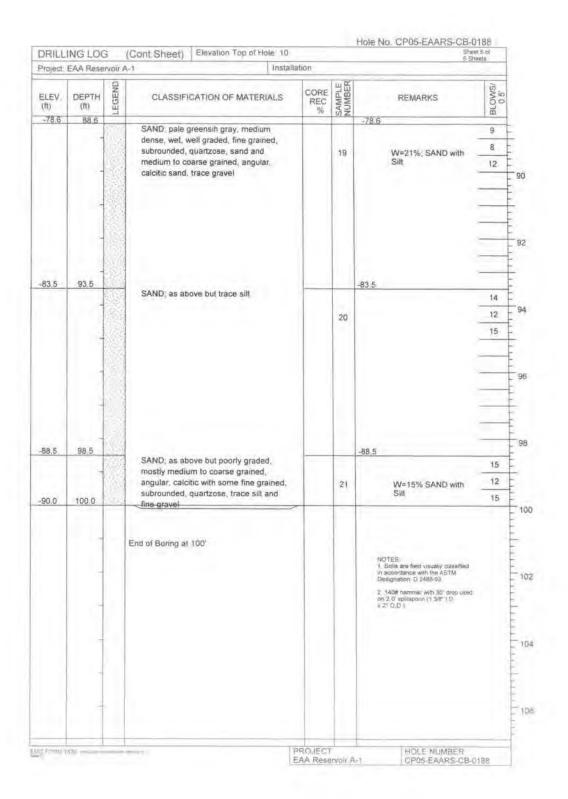


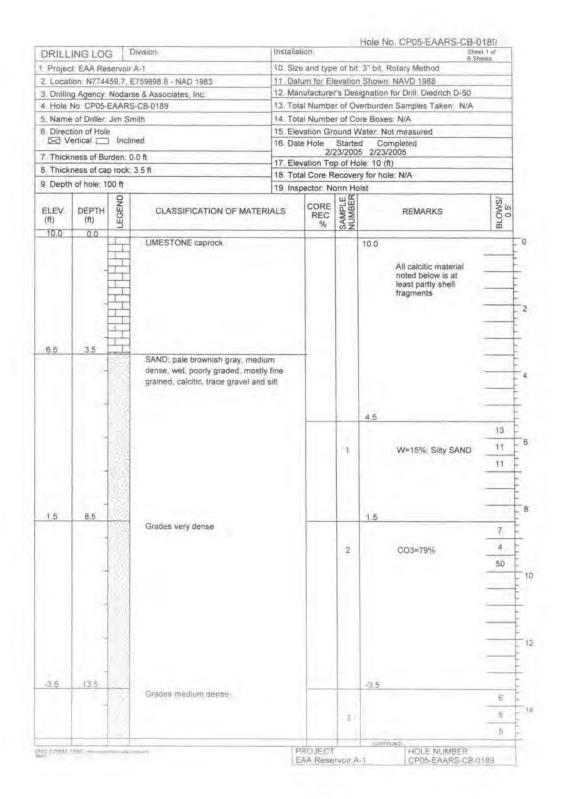


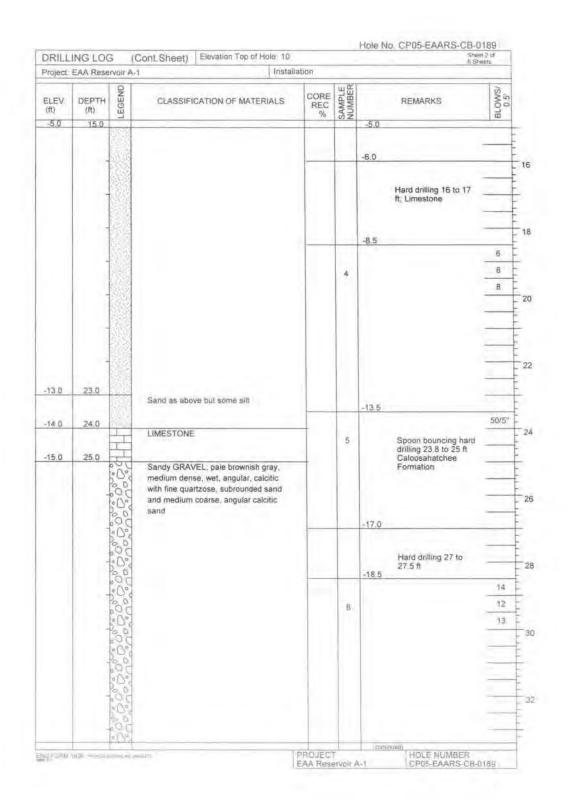


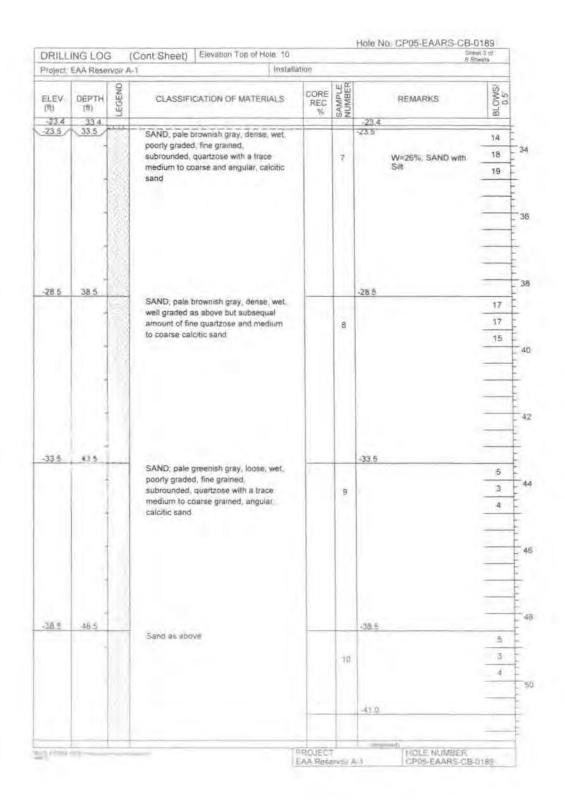


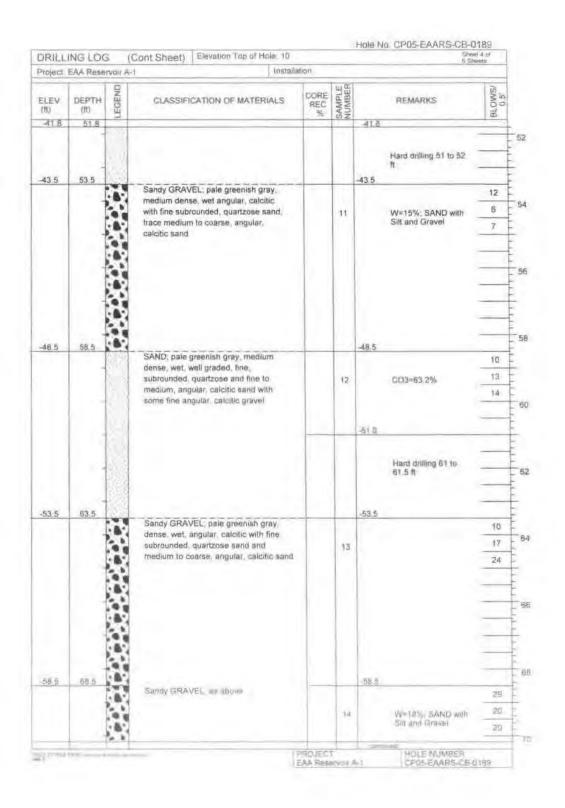


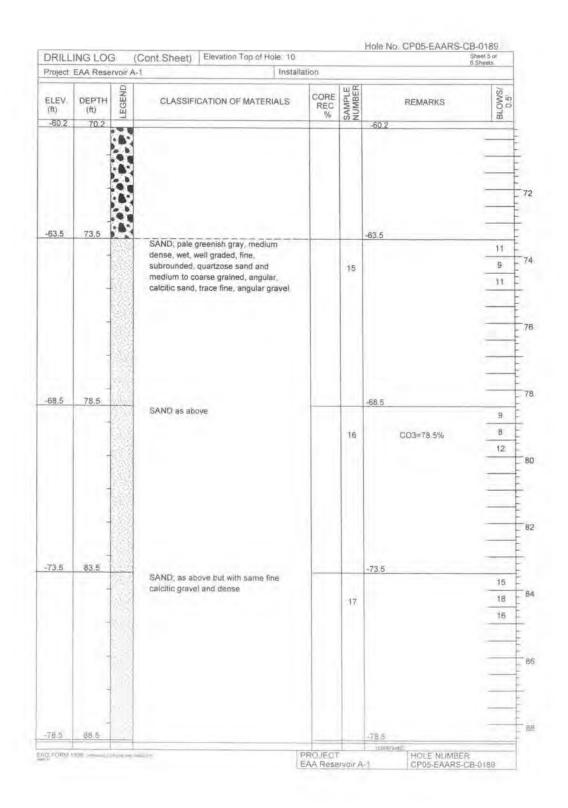


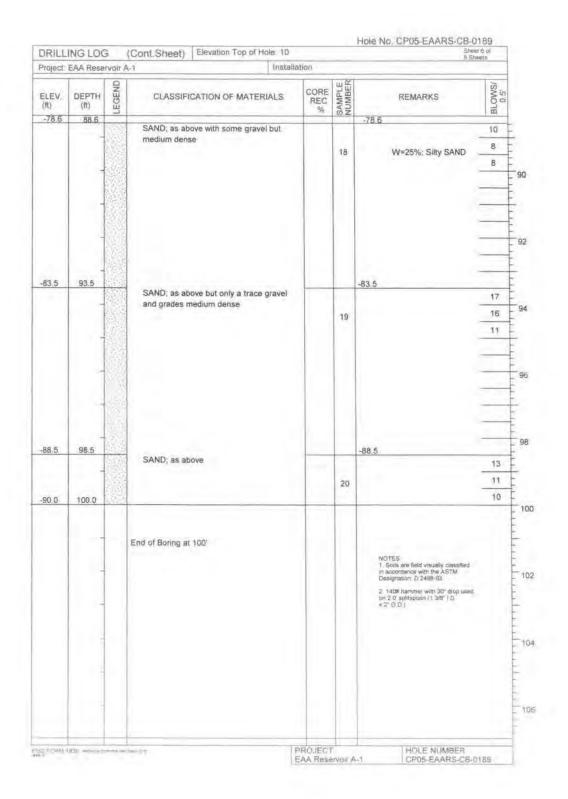


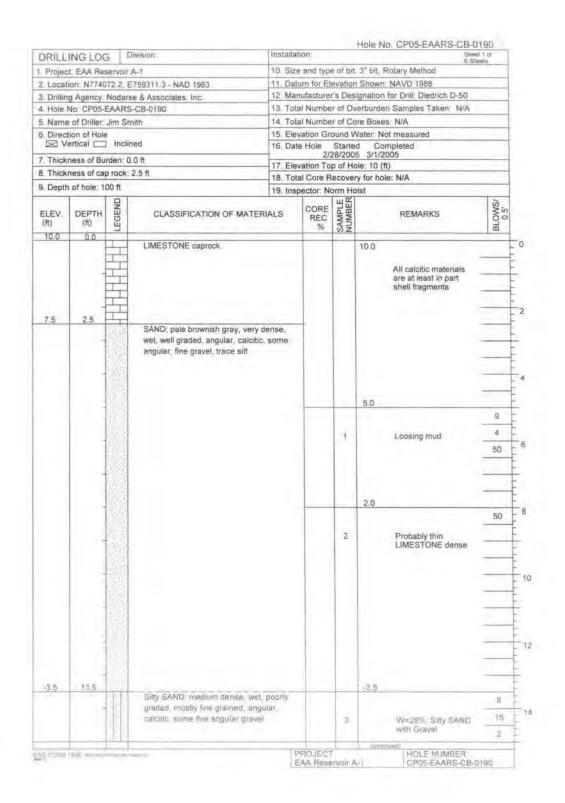


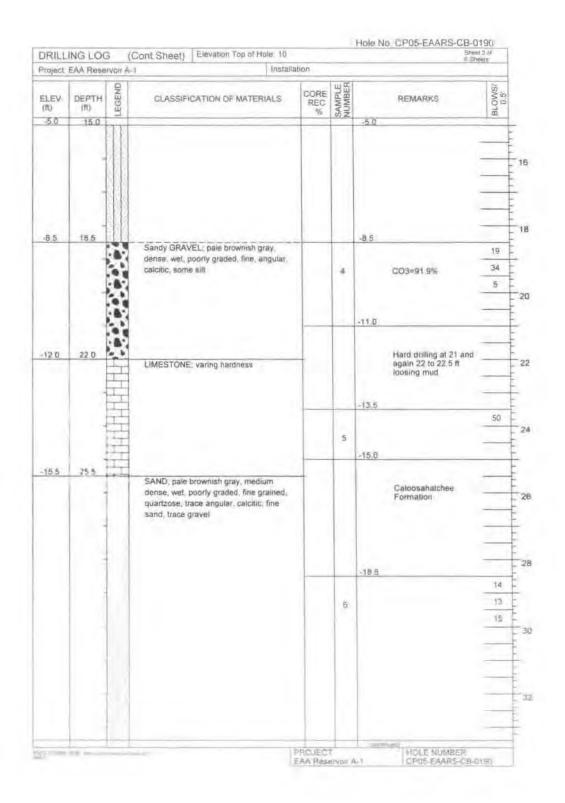


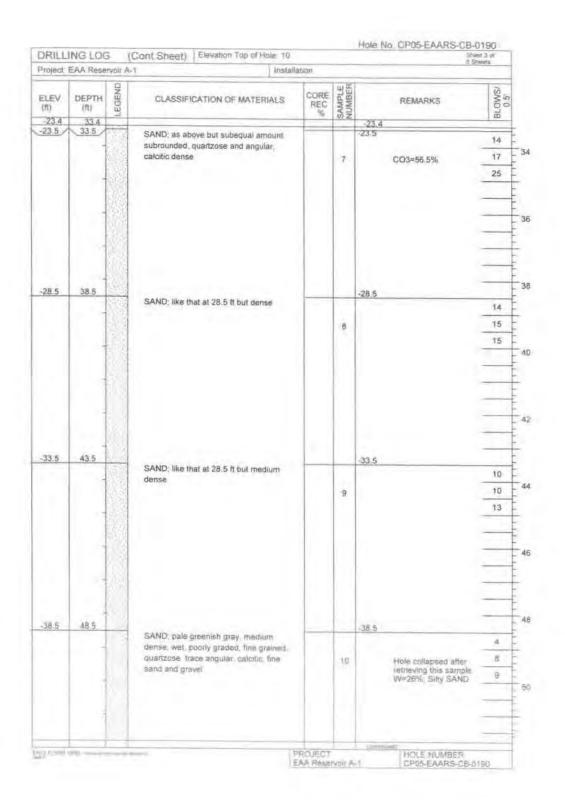


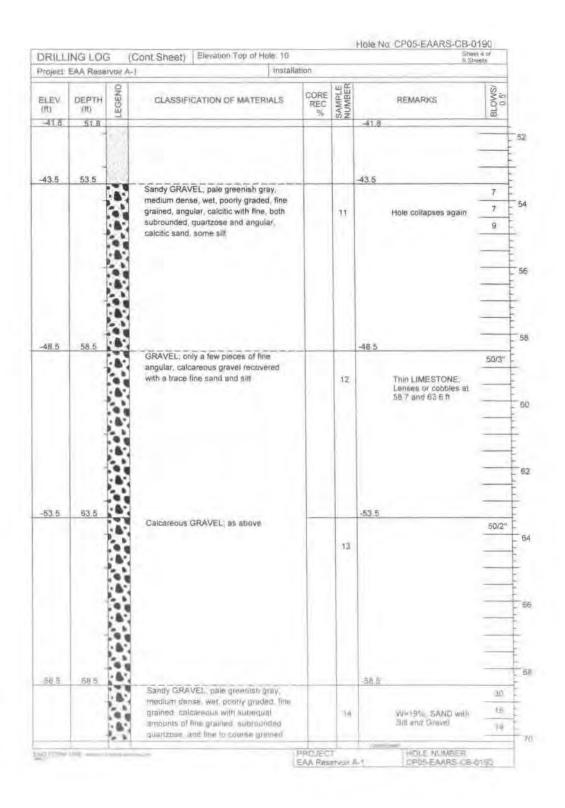


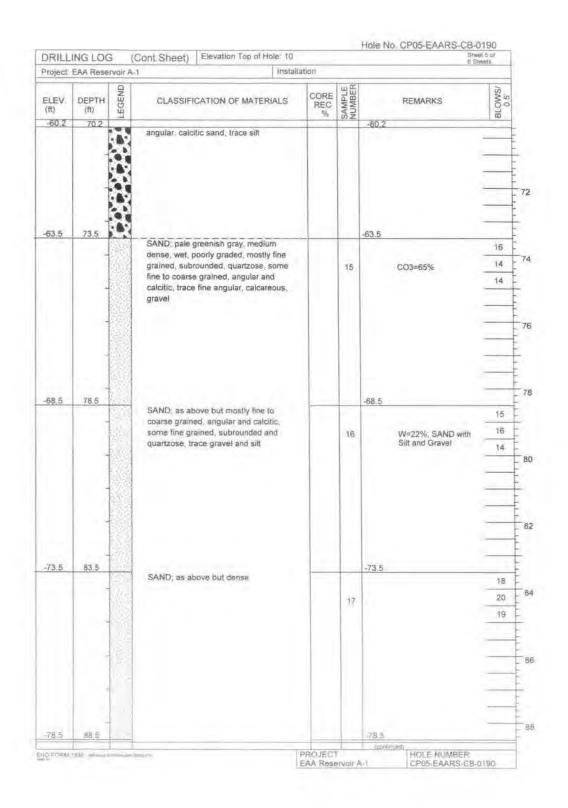


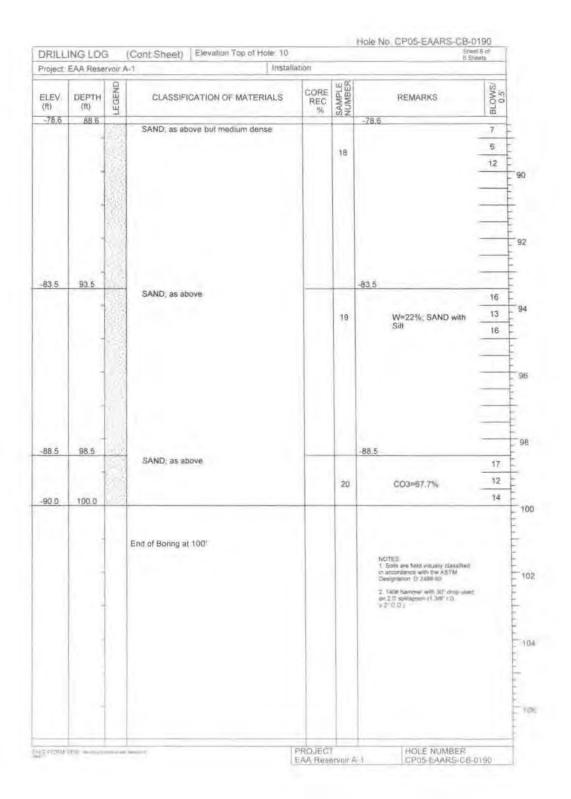


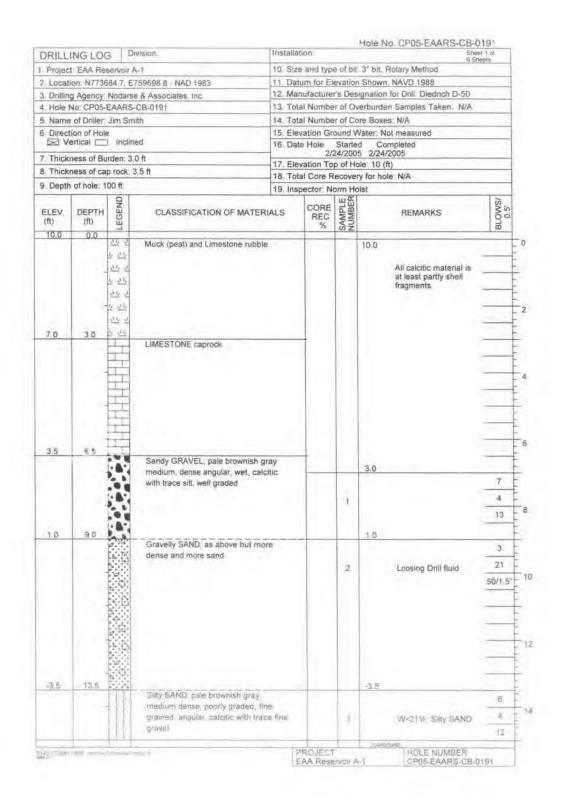


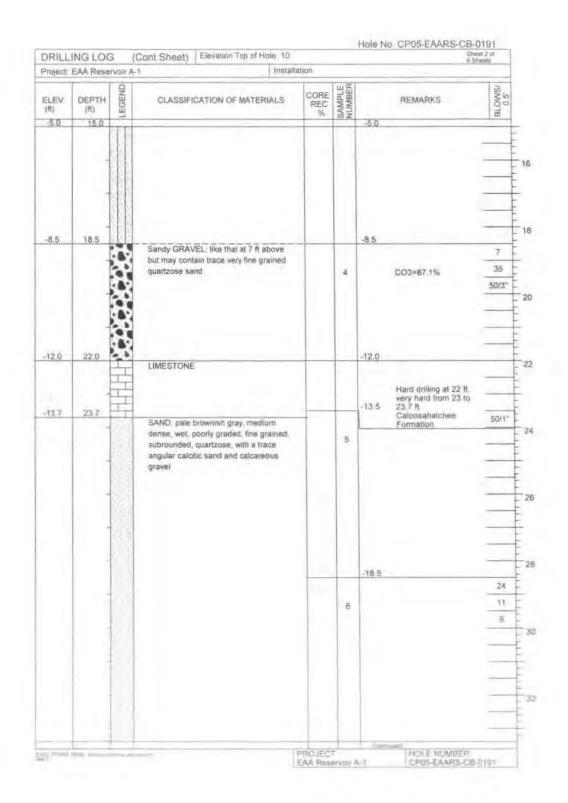


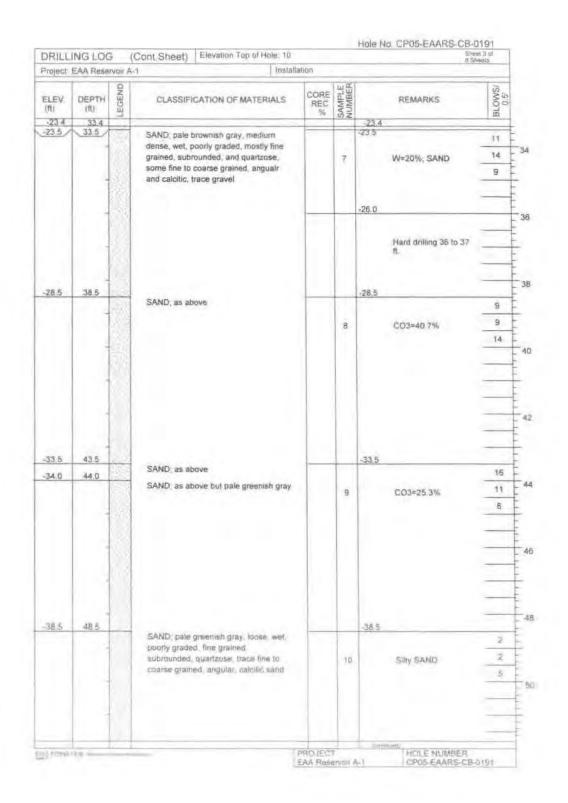


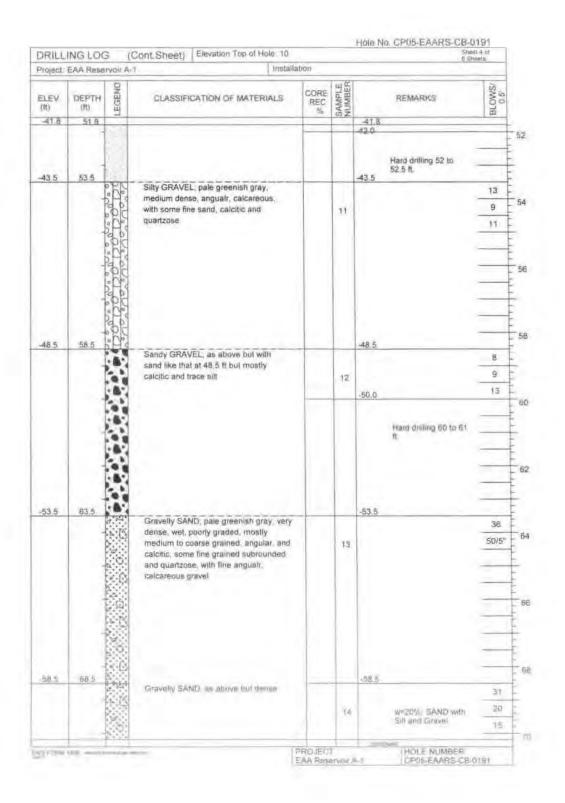


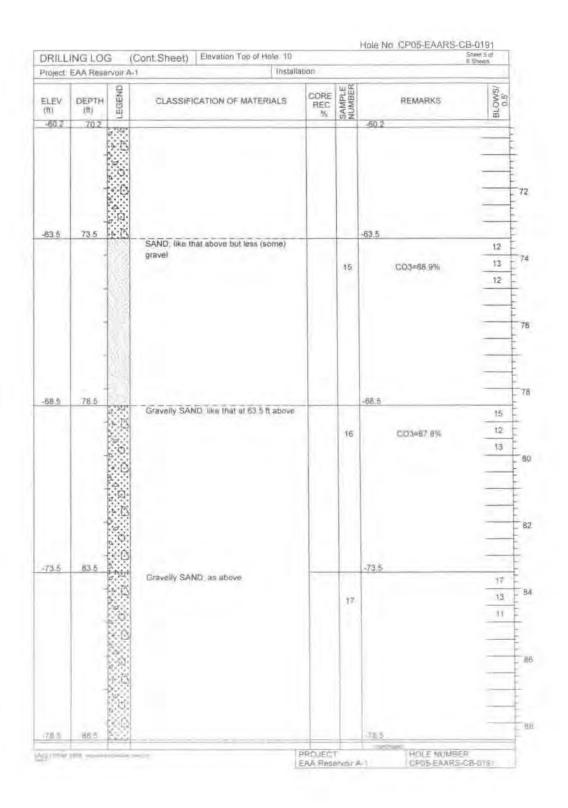


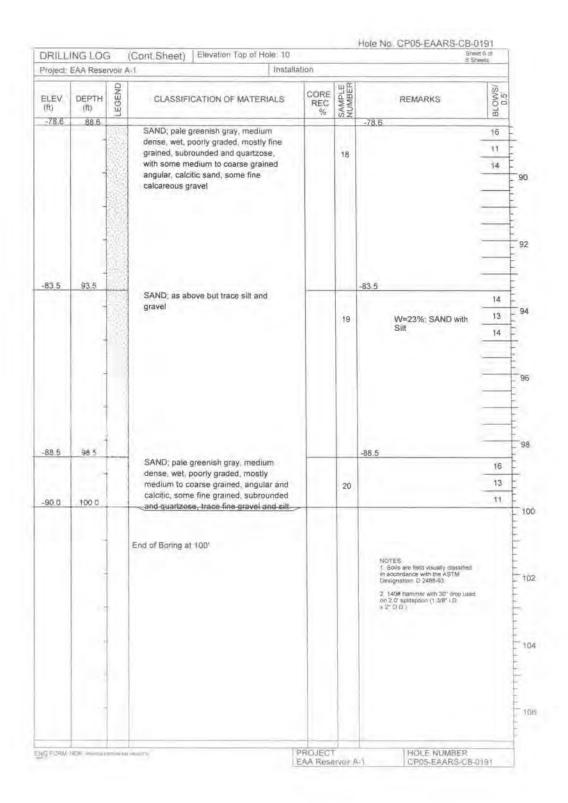


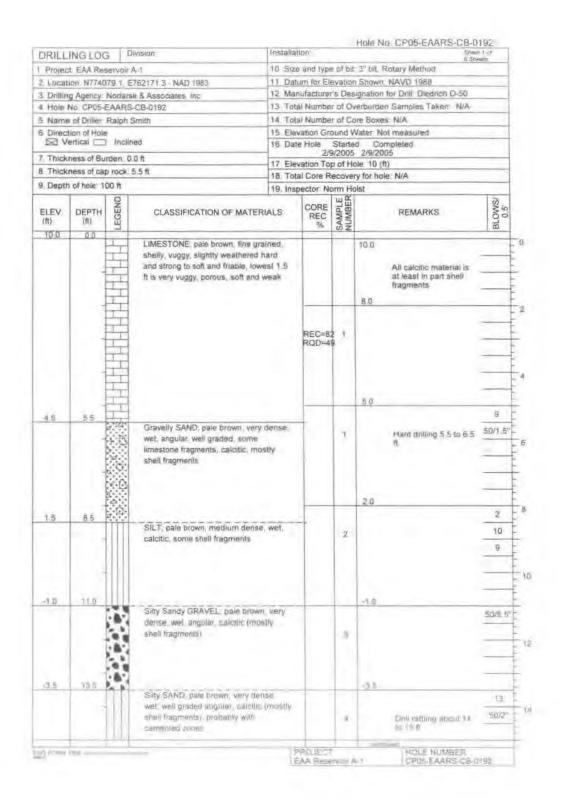


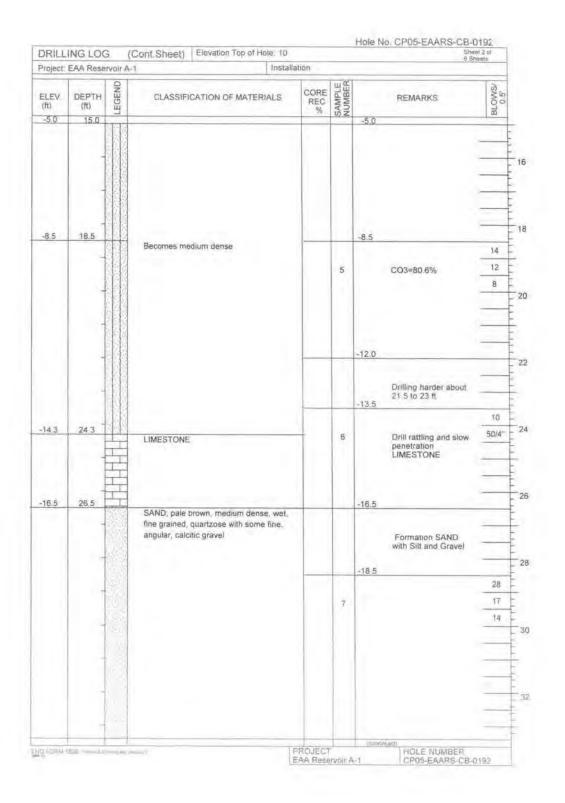


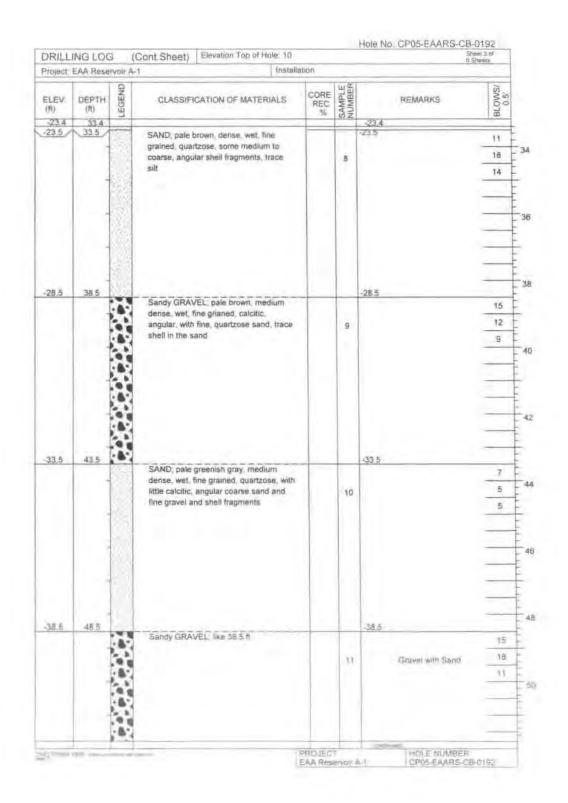


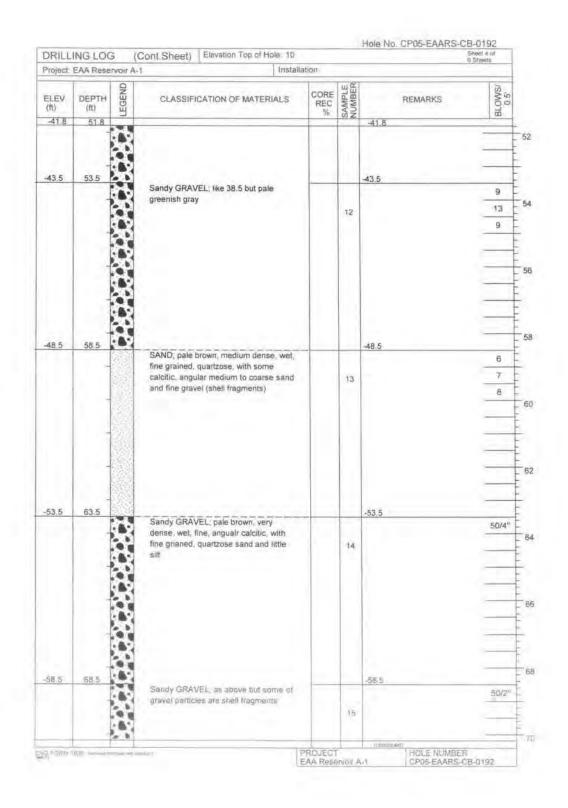


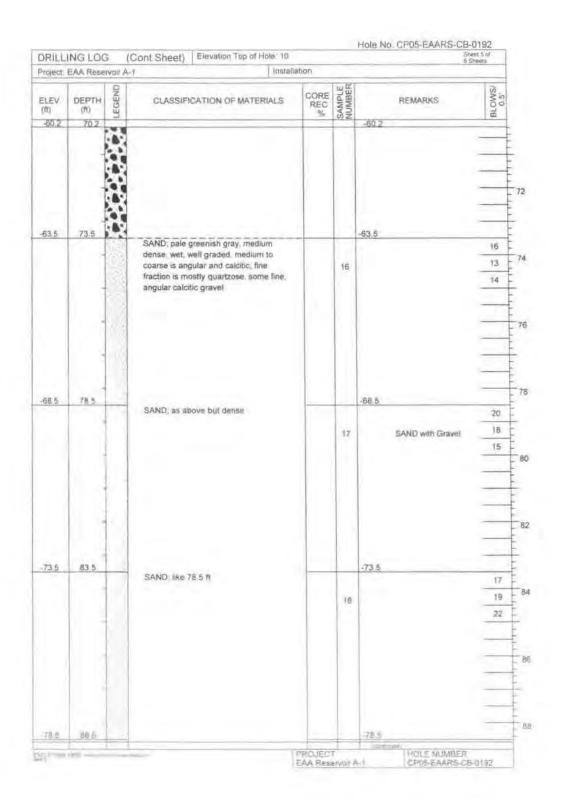


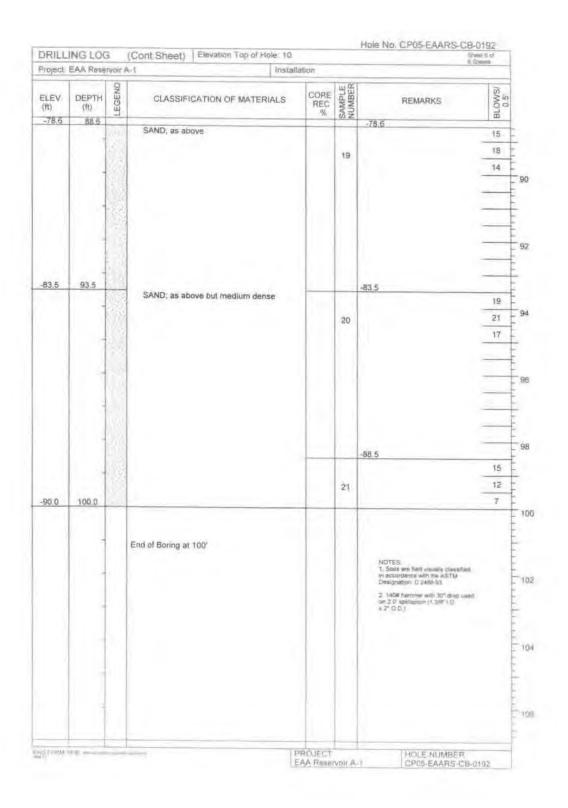




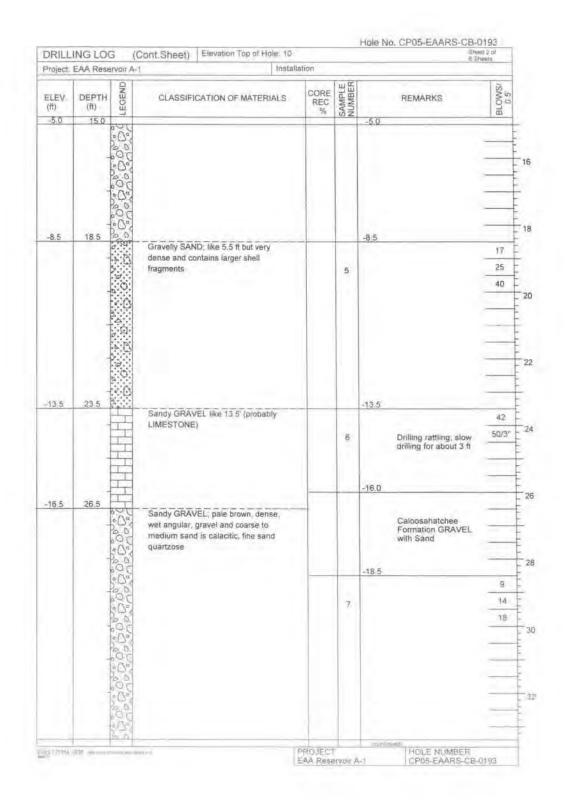


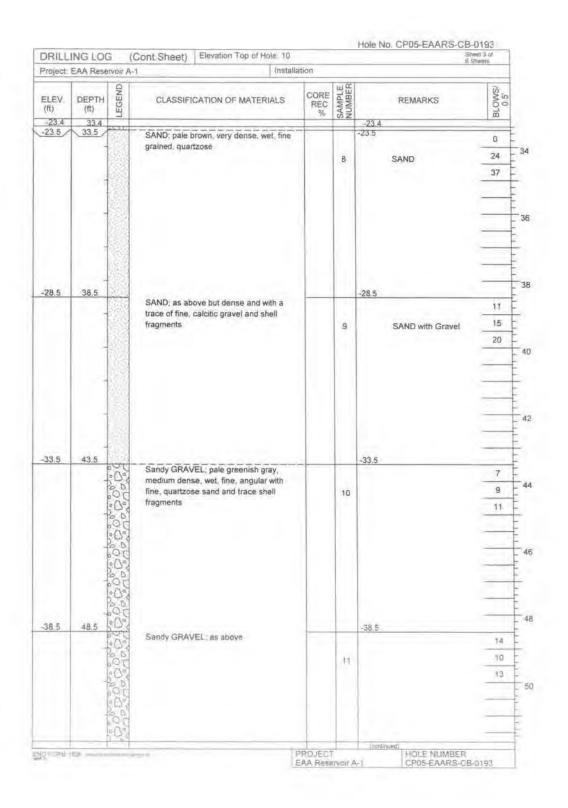


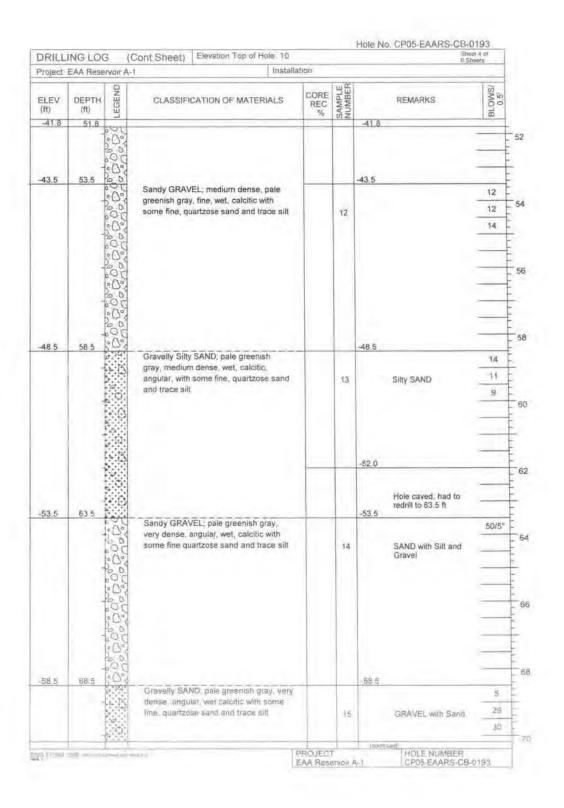


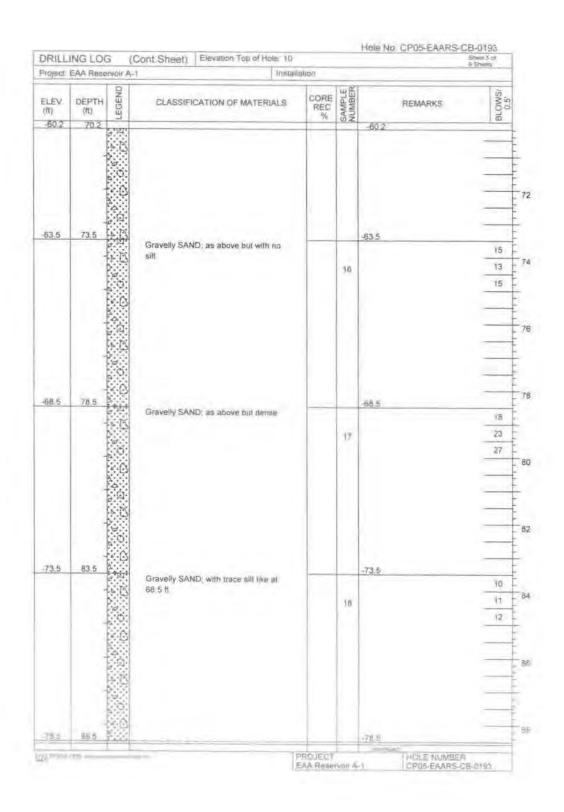


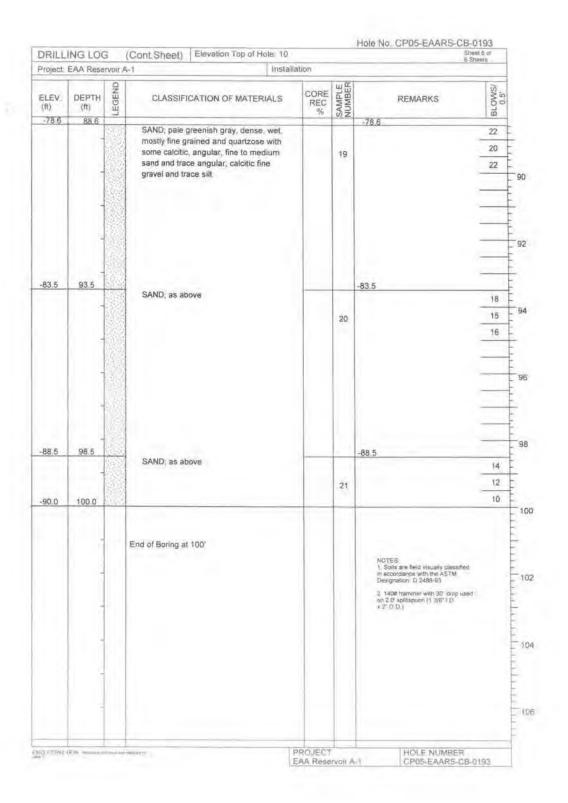
DRILLING LOG Division:					Hole No. CP05-EAARS-CB-0193 Installation: Sheet 1 of 8 Sheets 6			
1. Project, EAA Reservoir A-1					10. Size and type of bit. 3" bit. Rotary Method			
2. Location: N774466.6, E761983.8 - NAD 1983					11, Datum for Elevation Shown, NAVD 1988			
3. Drilling Agency: Nodarse & Associates, Inc.					12. Manufacturer's Designation for Drill: Diedrich D-50			
4. Hole No. CP05-EAARS-CB-0193					13. Total Number of Overburden Samples Taken N/A			
5. Name of Driller: Raiph Smith					14. Total Number of Core Boxes: N/A			
	tion of Hol	and		15. Elevation Ground Water. Not measured 16. Date Hole Started Completed				
	ertical		16. Da		Starte	d Completed 2/10/2005		
	ness of Bu		17. Ele		-	ole: 10 (ft)		
	ness of ca	5.0 ft.	18. To	al Core Re	ecove	ry for hole: N/A		
9. Depth	of hole: 1	00 ft		19. Ins	pector, No		olst	
ELEV.	DEPTH (ft)	EGEND	CLASSIFICATION OF MATERIALS		CORE REC %	SAMPLE	REMARKS	BLOWS/ 0.5'
10.0	0.0	-			.70	w Z		I III
9.5	0.5	28 3	Muck; organic silt, dark brown			10.0		
M. of	0.0	T	LIMESTONE; pale brown to ye	llow, fine			20-170-1-170-170	
			grained, shelly, vuggy, slightly weathered, strong and hard to soft, friable				All calcitic material is at least in part shell fragments	-
		T						
		1					8.0	
		1						
		T			-			
	100				REC=42 RQD=16			_
					1.00			
	11 3				1			
	甘							
								-
4.5	5.5						4.5	
		0	Gravelly SAND; light brown, medium					10
		4:0		ense, wet, angular, well graded, elcitic, shell fragments			CONTROL TO CAND	11
		· or ·	Carollo, Stiell Hagiliens			1	GRAVEL with SAND	10
		P. O.						10
		. 0						
		2						
1.5	8.5	D.					1.5	
1.0	0.0	الإنوا	Silty GRAVEL, white medium	dense.	-		1.3.	6
		5 70	wet line, angular, non plastic, calcitic					- N
	100 C 200 C		shell fragments			2	GRAVEL with Silt and Sand	5
								4
	57.5	000					La la	-
-1.0	110	STV	CRAVEL and himself	men lucal	-		-1,0	-
		. 0.	GRAVEL, pale brown, very dense, wet. line, angular, calcific, frace sand and silt, shell tragments.					B
	1 3					3	SAND with Gravel	50/5.5
						3	SAIND WITH GROVE	
		- 4						_
-3.5	13.5						-3.5	
		NOT S	Sandy GRAVEL pale frown very					12
		S B	dense fine angular, calcitic si				2072-007-007-0	34
		000	shell fragments			- A	Sravul with Silt and SAND	_
		100						:40
	1920				ROJECT		(continued)	

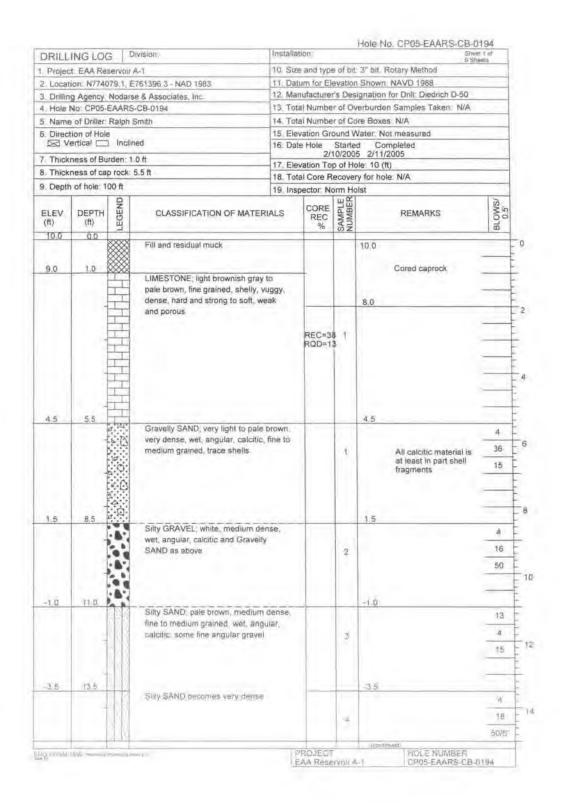


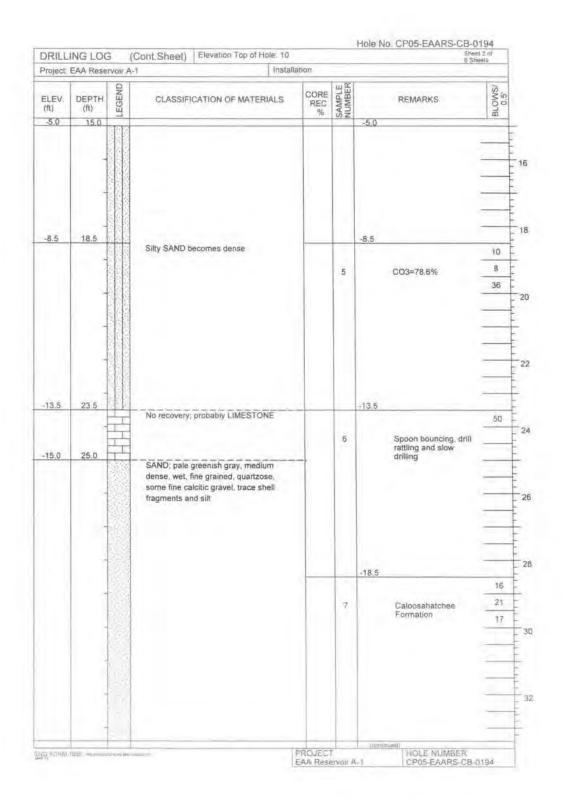


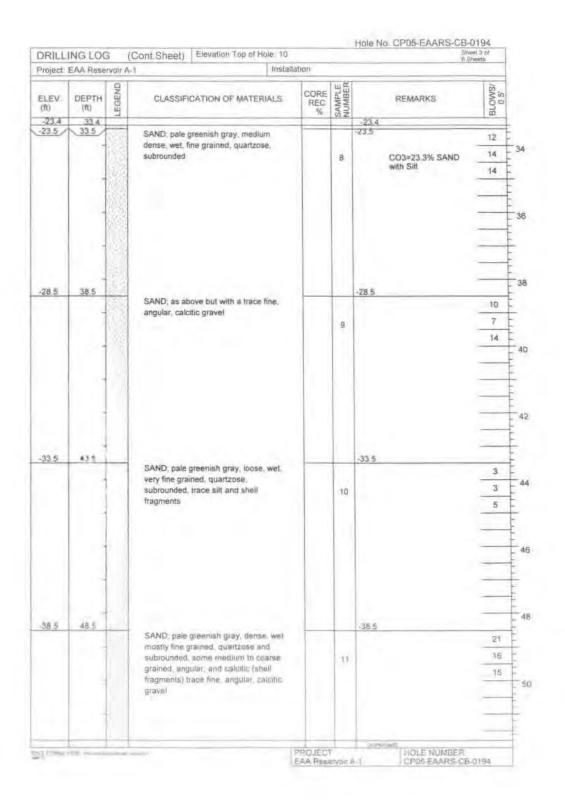


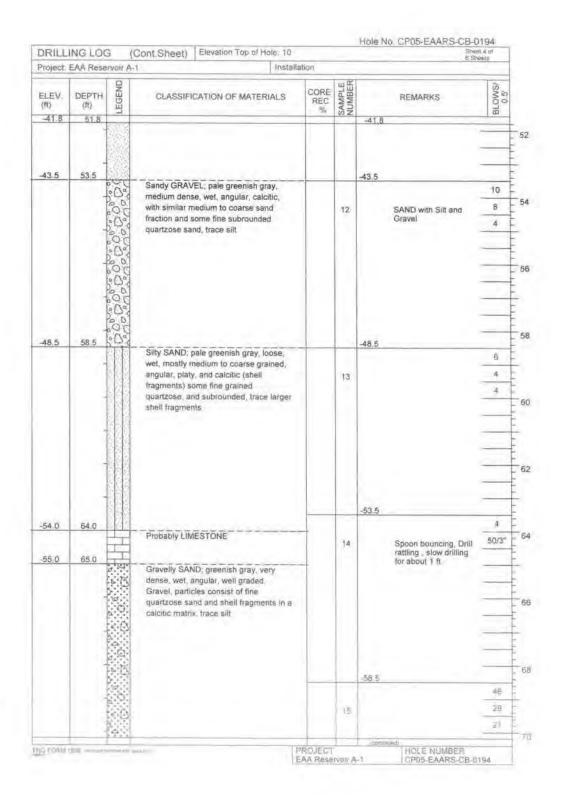


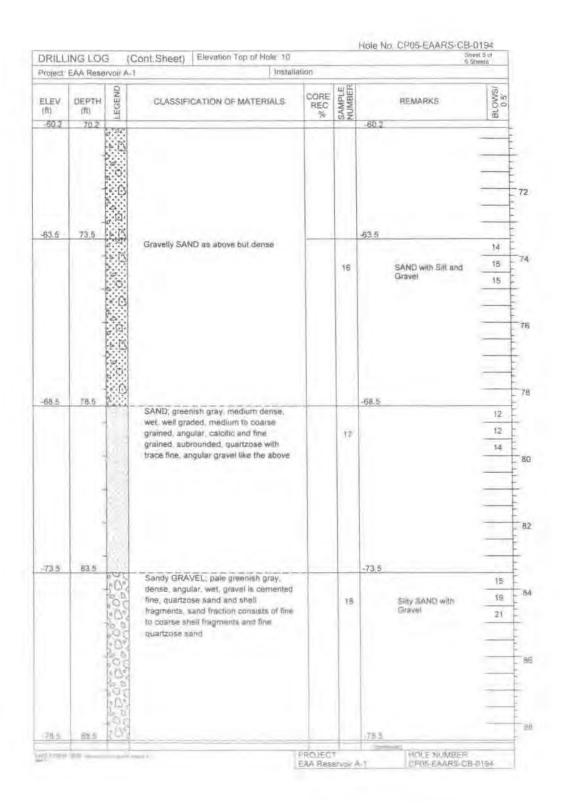


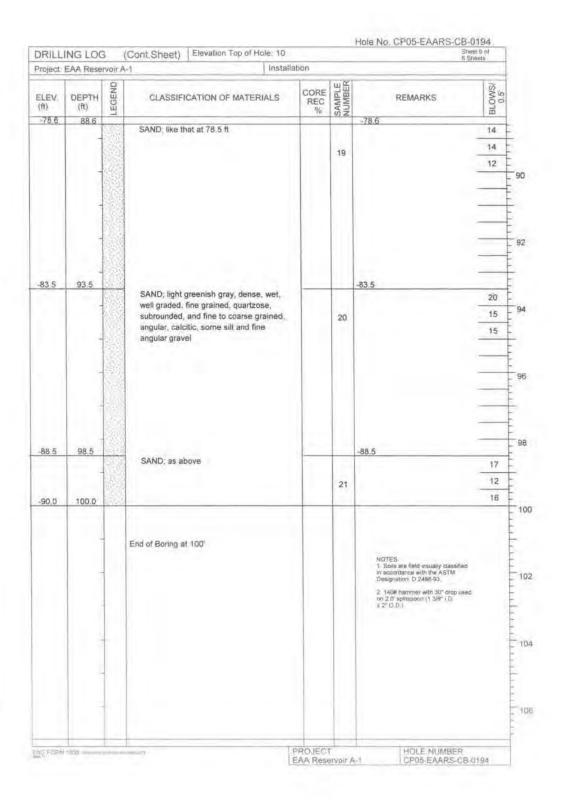


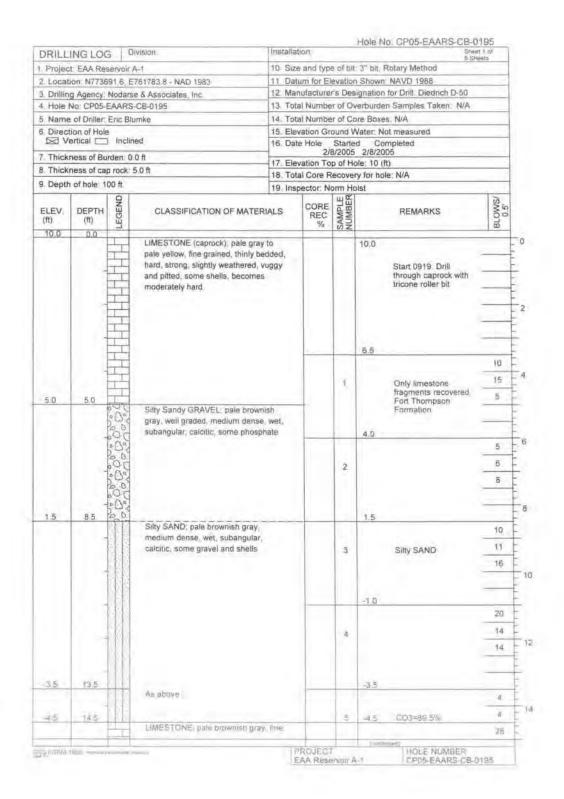


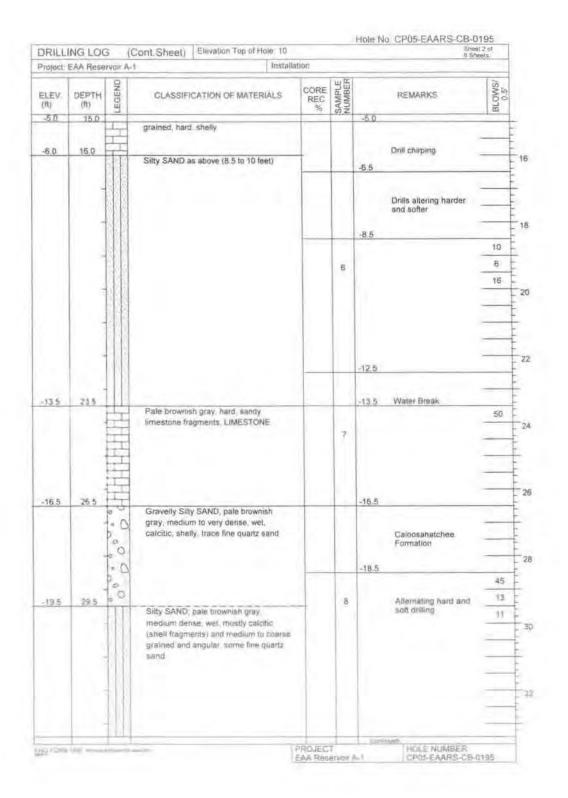


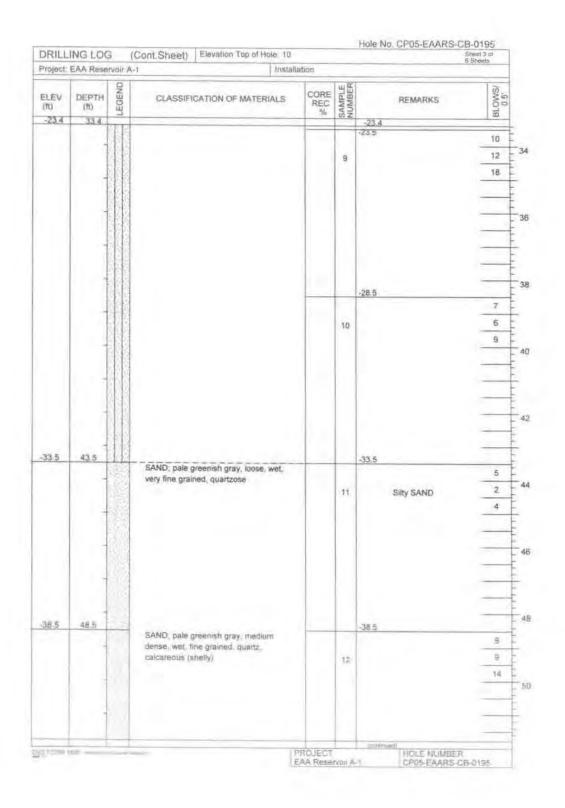


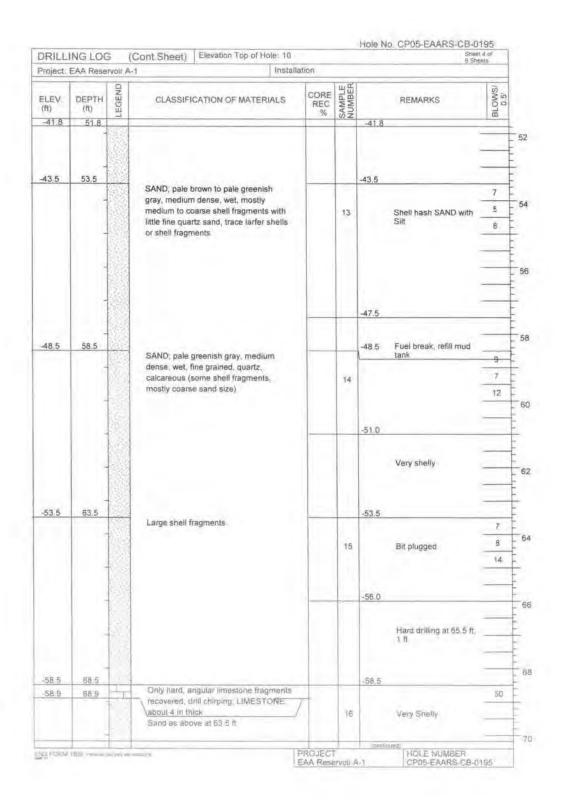


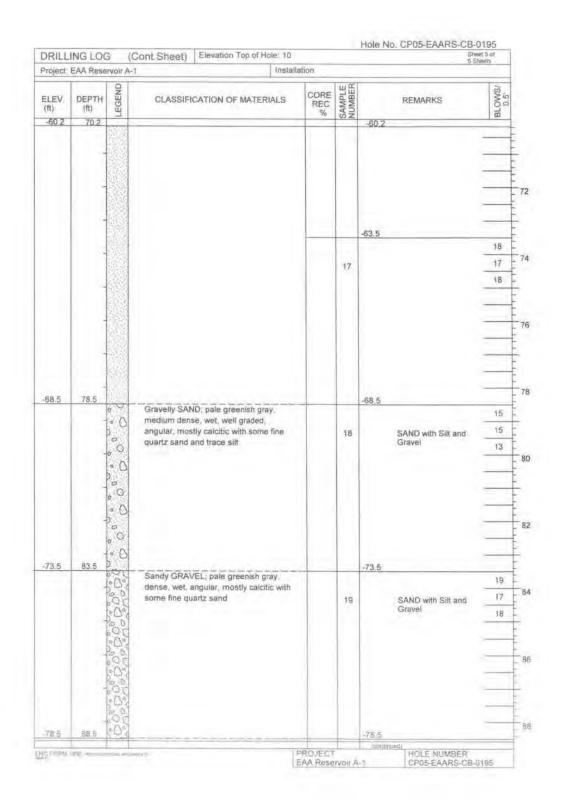


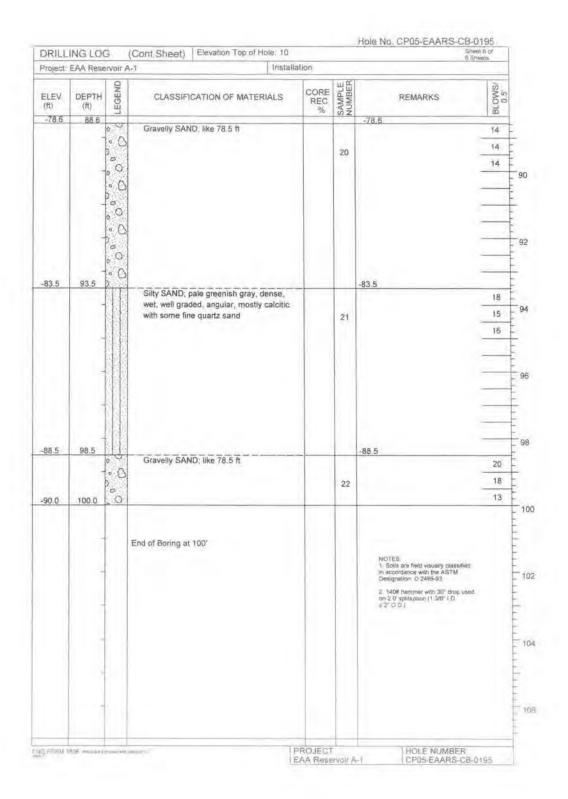


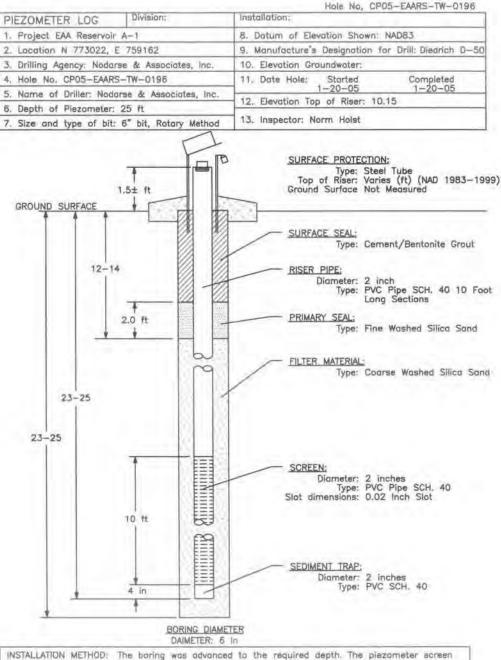






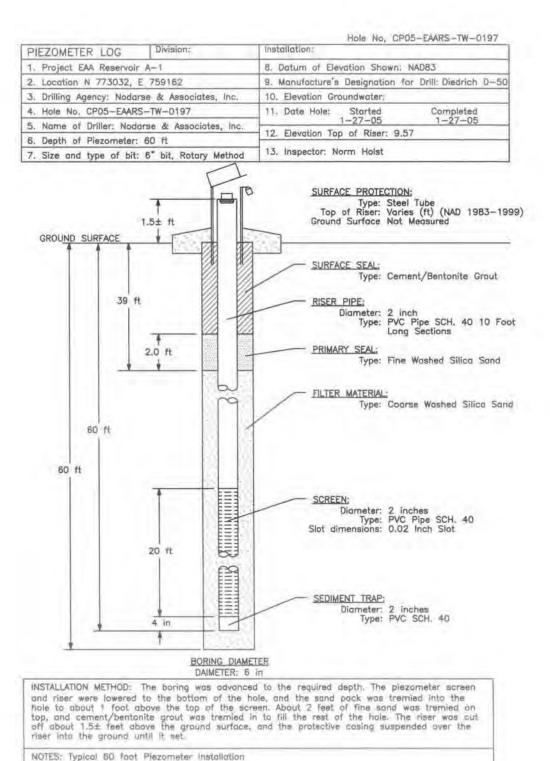


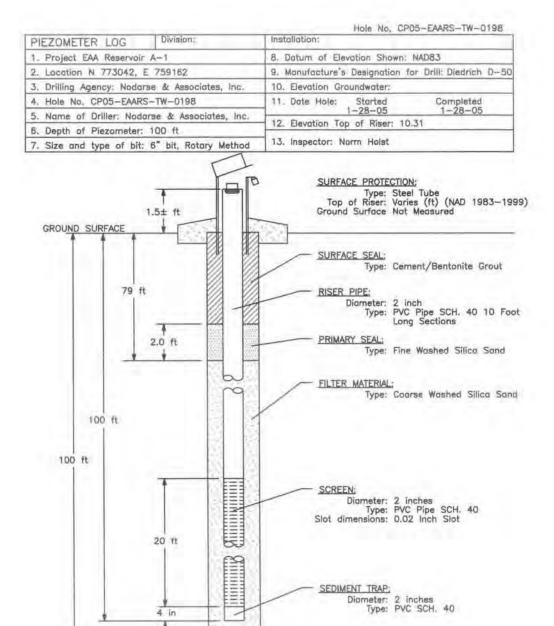




INSTALLATION METHOD: The boring was advanced to the required depth. The piezometer screen and riser were lowered to the bottom of the hale, and the sand pack was tremied into the hole to about 1 foot above the top of the screen. About 2 feet of fine sand was tremied on top, and cement/bentanite grout was tremied in to fill the rest of the hole. The riser was cut off about 1.5± feet above the ground surface, and the protective casing suspended over the riser into the ground until it set.

NOTES: Typical 25 foot Piezometer Installation

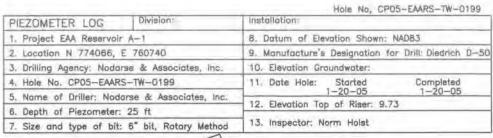


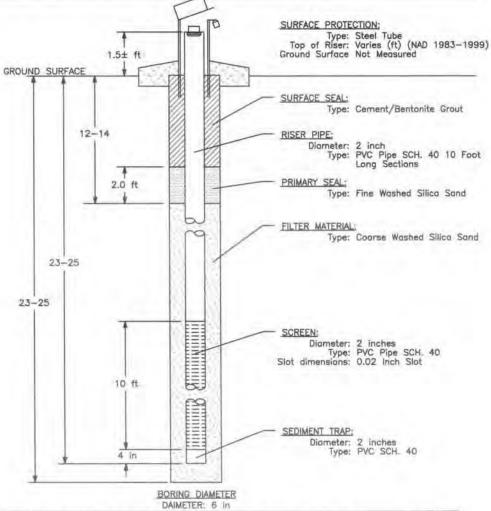


INSTALLATION METHOD: The boring was advanced to the required depth. The piezometer screen and riser were lowered to the bottom of the hole, and the sand pack was tremied into the hole to about 1 foot above the top of the screen. About 2 feet of fine sand was tremied on top, and cement/bentonite grout was tremied in to fill the rest of the hole. The riser was cut off about 1.5± feet above the ground surface, and the protective cosing suspended over the riser into the ground until it set.

BORING DIAMETER DAIMETER: 6 in

NOTES: Typical 100 foot Piezometer Installation

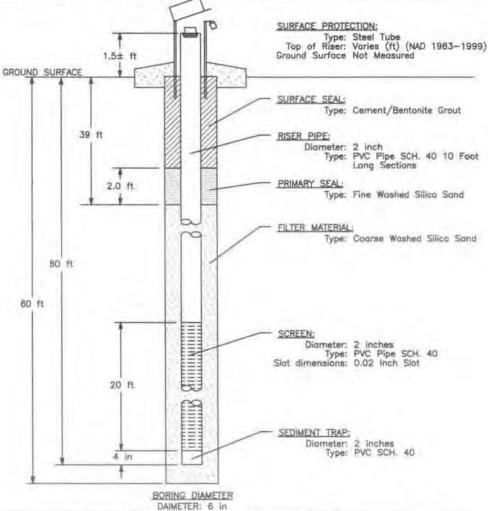




INSTALLATION METHOD: The boring was advanced to the required depth. The piezometer screen and riser were lawered to the bottom of the hole, and the sand pack was tremied into the hole to about 1 foot above the top of the screen. About 2 feet of fine sand was tremied on top, and cement/bentonite grout was tremied in to fill the rest of the hole. The riser was cut off about 1.5± feet above the ground surface, and the protective casing suspended over the riser into the ground until it set.

NOTES: Typical 25 foot Piezometer Installation

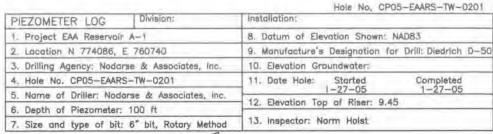


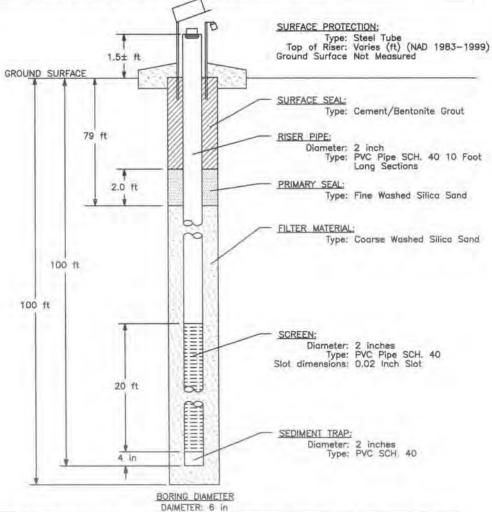


INSTALLATION METHOD: The boring was advanced to the required depth. The plezometer screen and riser were lowered to the bottom of the hole, and the sand pack was tremied into the hole to about 1 foot above the tap of the screen. About 2 feet of fine sand was tremied on top, and cement/bentonite grout was tramied in to fill the rest of the hole. The riser was out off about 1.5± feet above the ground surface, and the protective casing suspended over the riser into the ground until it set.

NOTES: Typical 60 foot Piezometer Installation

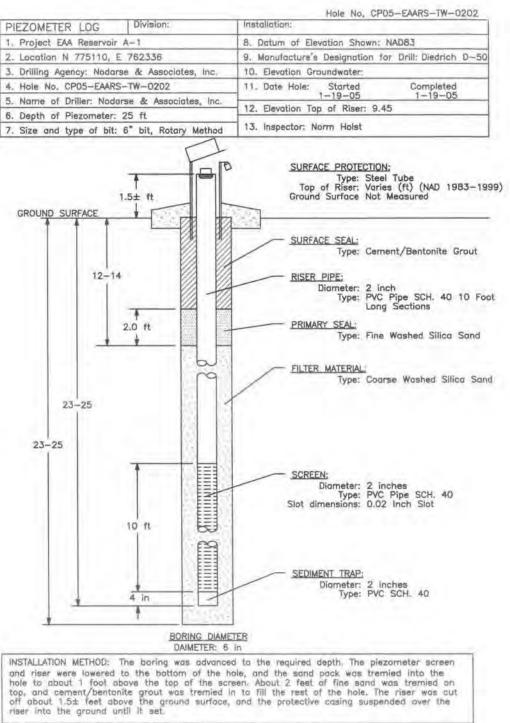
APPENDIX 1 TEST CELL BORINGS AND PIEZOMETER INSTALLATION LOGS: 201-220





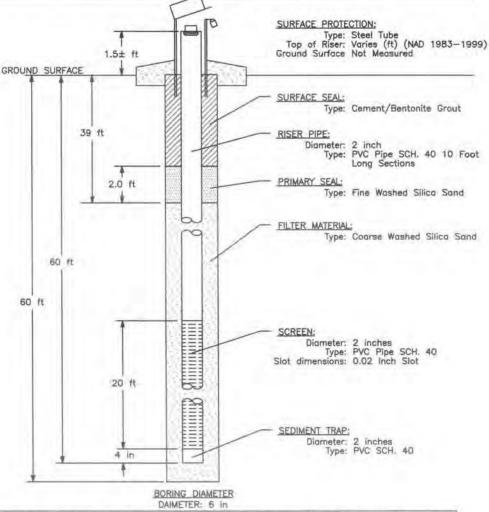
INSTALLATION METHOD: The boring was advanced to the required depth. The piezometer screen and riser were lowered to the bottom of the hole, and the sand pack was tremied into the hole to about 1 foot above the top of the screen. About 2 feet of fine sand was tremied on top, and cement/bentanite grout was tremied in to fill the rest of the hole. The riser was cut off about 1.5± feet above the ground surface, and the protective casing suspended over the riser into the ground until it set.

NOTES: Typical 100 loot Piezometer Installation



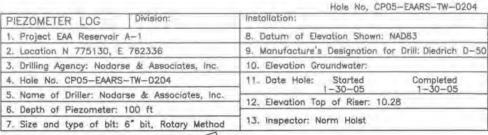
NOTES: Typical 25 foot Plezometer Installation

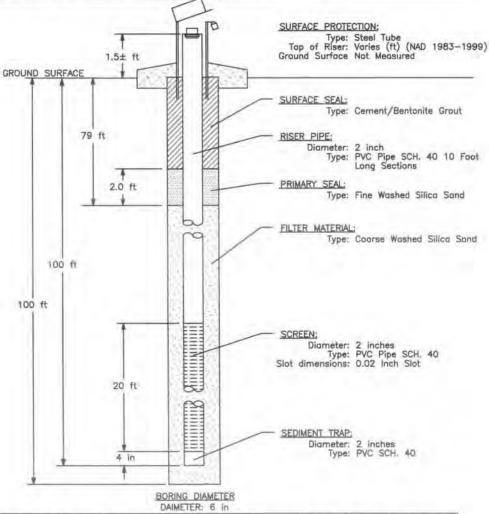




INSTALLATION METHOD: The boring was advanced to the required depth. The piezometer screen and riser were lowered to the bottom of the hole, and the sand pack was tremied into the hole to about 1 foot above the top of the screen. About 2 feet of fine sand was tremied on top, and cement/bentonite grout was tremied in to fill the rest of the hole. The riser was cut off about 1.5± feet above the ground surface, and the protective casing suspended over the riser into the ground until it set.

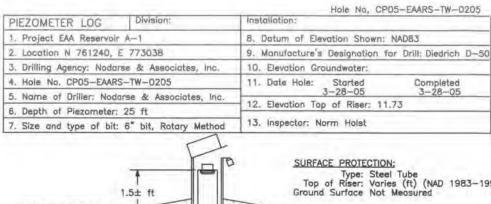
NOTES: Typical 60 foot Piezometer Installation

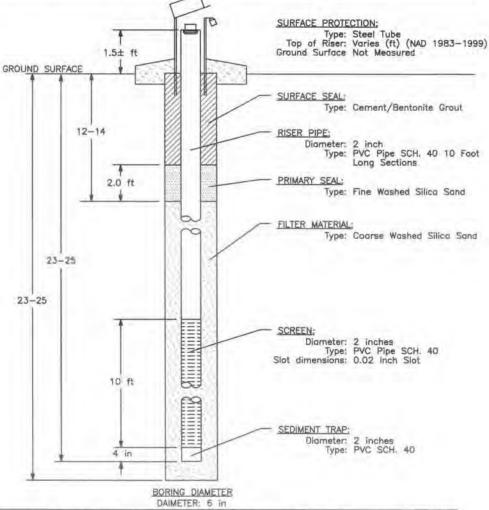




INSTALLATION METHOD: The boring was advanced to the required depth. The piezometer screen and riser were lowered to the bottom of the hole, and the sond pack was tremied into the hole to about 1 foot above the top of the screen. About 2 feet of fine sand was tremied on top, and cement/bentonite grout was tremied in to fill the rest of the hole. The riser was cut off about 1.5± feet above the ground surface, and the protective cosing suspended over the riser into the ground until it set.

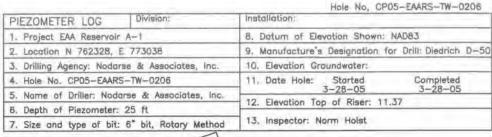
NOTES: Typical 100 foot Piezometer Installation

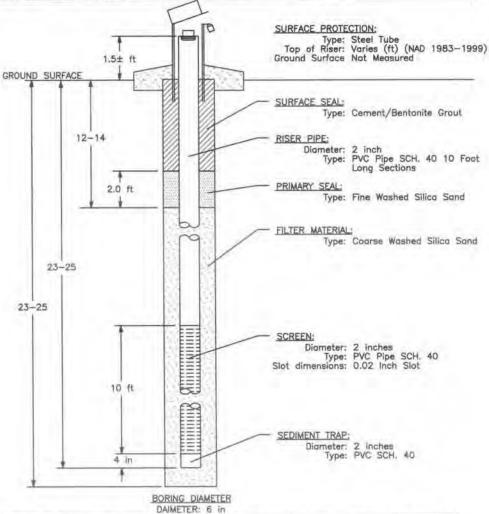




INSTALLATION METHOD: The boring was advanced to the required depth. The piezometer screen and riser were lowered to the bottom of the hole, and the sand pack was tremied into the hole to about 1 foot above the top of the screen. About 2 feet of fine sand was tremied on top, and cement/bentonite grout was tremied in to fill the rest of the hole. The riser was cut off about $1.5\pm$ feet above the ground surface, and the protective casing suspended over the riser into the ground until it set.

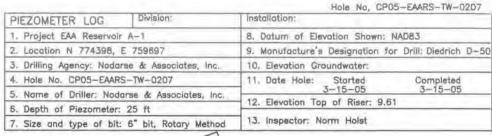
NOTES: Typical 25 foot Piezameter Installation

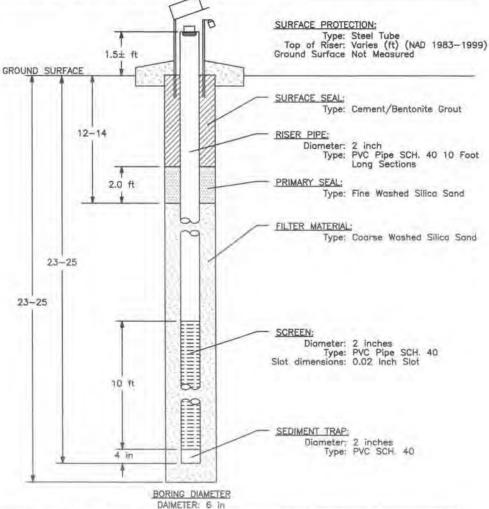




INSTALLATION METHOD: The boring was advanced to the required depth. The piezometer screen and riser were lowered to the bottom of the hole, and the sand pack was tremied into the hole to about 1 foot above the top of the screen. About 2 feet of fine sand was tremied on top, and cement/bentonite grout was tremied in to fill the rest of the hole. The riser was cut off about 1.5± feet above the ground surface, and the protective casing suspended over the riser into the ground until it set.

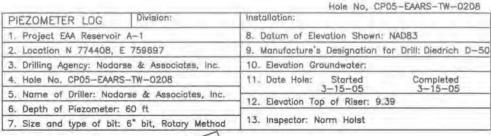
NOTES: Typical 25 foot Piezometer Installation

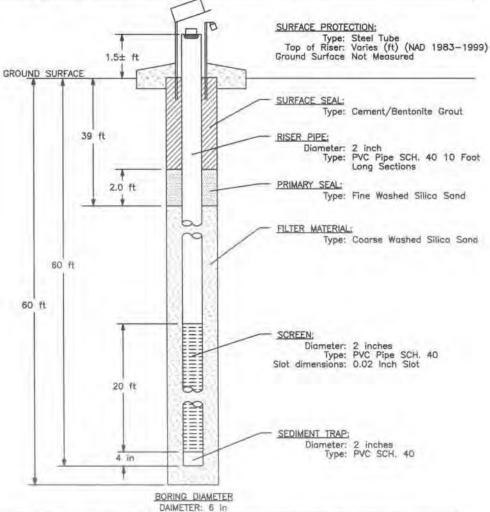




INSTALLATION METHOD: The boring was advanced to the required depth. The piezometer screen and riser were lawered to the bottom of the hole, and the sand pack was tremied into the hole to about 1 foot above the top of the screen. About 2 feet of fine sand was tremied on top, and cement/bentonite grout was tremied in to fill the rest of the hole. The riser was cut off about 1.5± feet above the ground surface, and the protective casing suspended over the riser late the ground until it set.

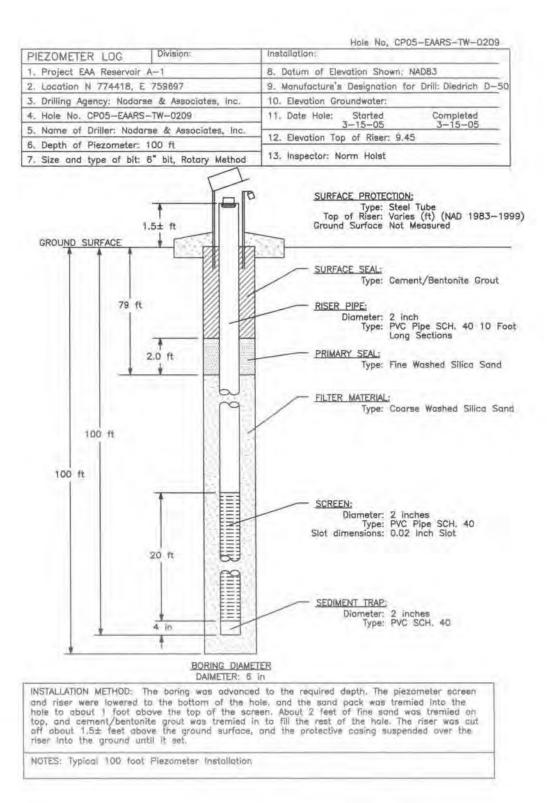
NOTES: Typical 25 foot Piezometer Installation

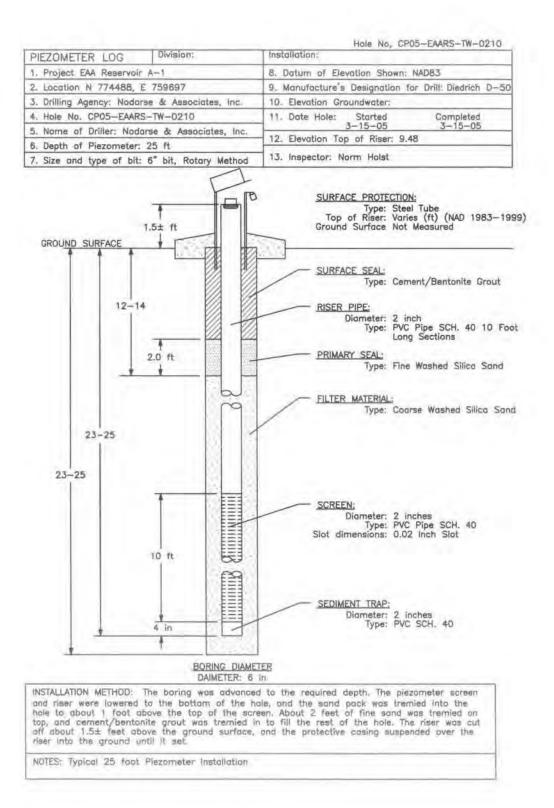


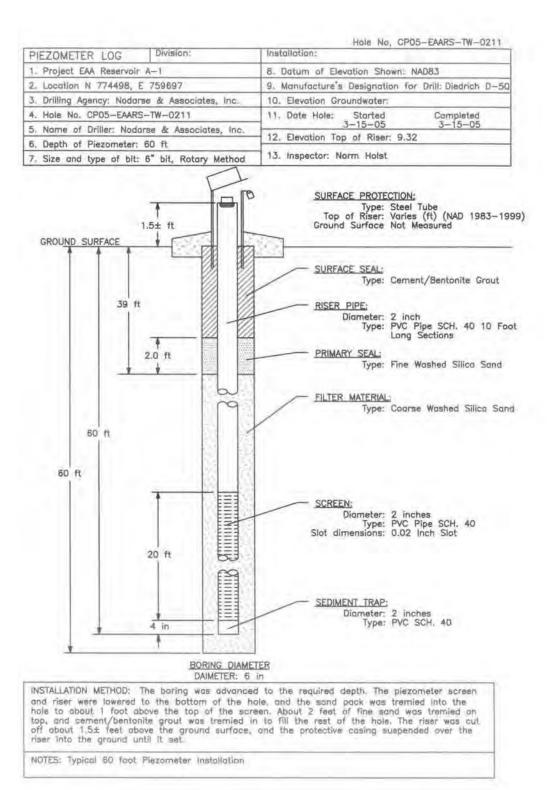


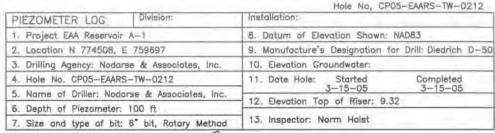
INSTALLATION METHOD: The boring was advanced to the required depth. The piezometer screen and riser were lowered to the bottom of the hole, and the sand pack was tremied into the hole to about 1 foot above the top of the screen. About 2 feet of fine sand was tremied on top, and cement/bentonite grout was tremied in to fill the rest of the hole. The riser was cut off about 1.5± feet above the ground surface, and the protective casing suspended over the riser into the ground until it set.

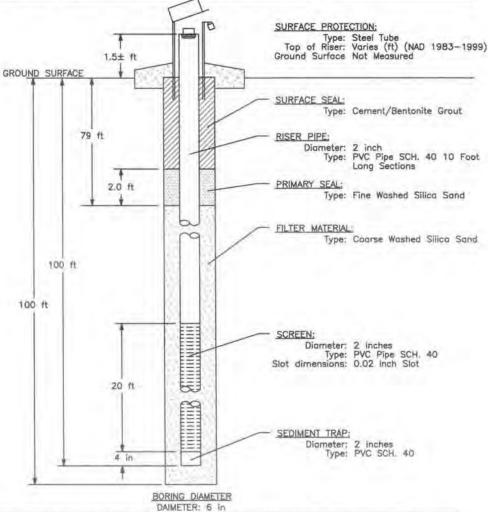
NOTES: Typical 60 foot Piezometer Installation





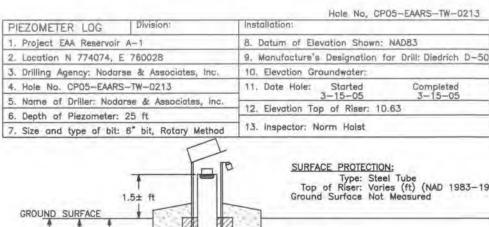






INSTALLATION METHOD: The boring was advanced to the required depth. The piezometer screen and riser were lowered to the bottom of the hole, and the sand pack was tremied into the hole to about 1 foot above the top of the screen. About 2 feet of fine sand was tremied on top, and cement/bentonite grout was tremied in to fill the rest of the hole. The riser was cut off about $1.5\pm$ feet above the ground surface, and the protective casing suspended over the riser into the ground until it set.

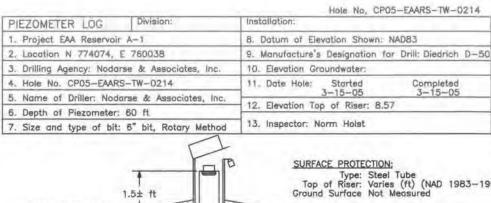
NOTES: Typical 100 foot Piezometer Installation

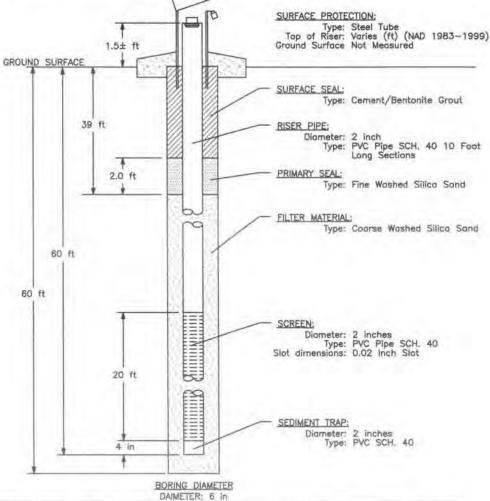


Type: Steel Tube Top of Riser: Varies (ft) (NAD 1983-1999) SURFACE SEAL: Type: Cement/Bentonite Grout 12-14 RISER PIPE: Diameter: 2 inch Type: PVC Pipe SCH, 40 10 Foot Long Sections PRIMARY SEAL: 2.0 ft Type: Fine Washed Silica Sand FILTER MATERIAL Type: Coarse Washed Silica Sand 23-25 SCREEN: Diameter: 2 inches Type: PVC Pipe SCH. 40 Slot dimensions: 0.02 Inch Slot 10 ft SEDIMENT TRAP: Diameter: 2 inches Type: PVC SCH. 40 4 in BORING DIAMETER DAIMETER: 6 in

INSTALLATION METHOD: The boring was advanced to the required depth. The piezameter screen and riser were lowered to the bottom of the hole, and the sand pack was tremied into the hole to about 1 foot above the top of the screen. About 2 feet of fine sand was tremied on top, and cement/bentonite grout was tremied in to fill the rest of the hole. The riser was cut off about 1.5± feet above the ground surface, and the protective casing suspended over the riser into the ground until it set.

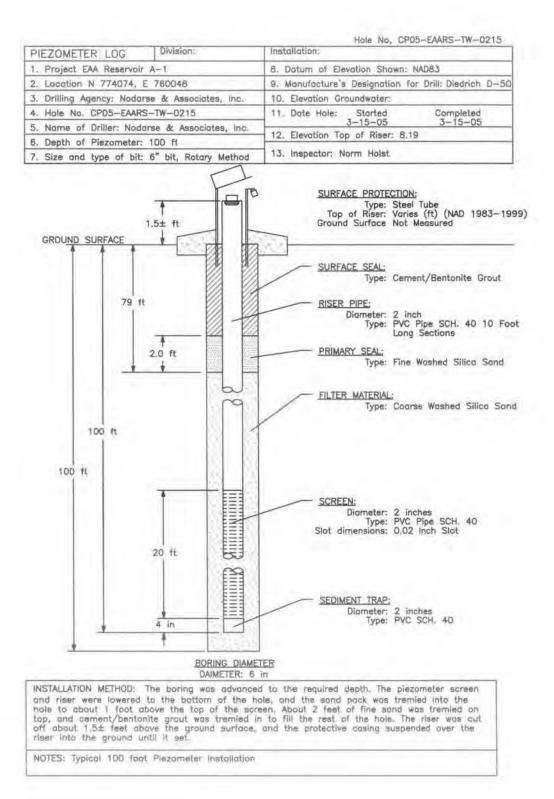
NOTES: Typical 25 foot Plezometer Installation

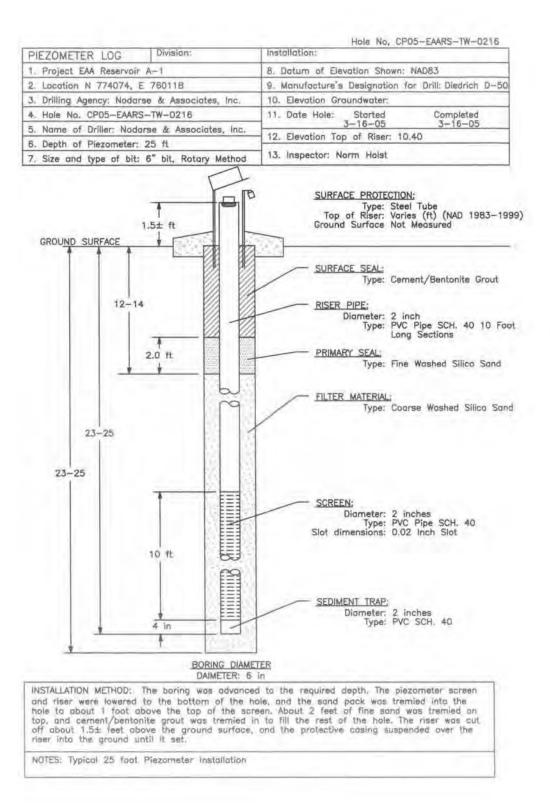


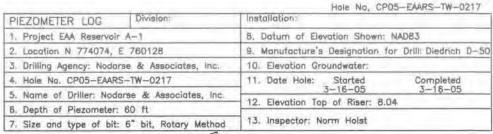


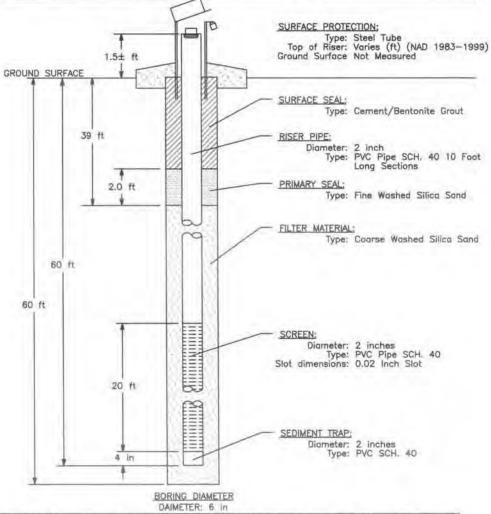
INSTALLATION METHOD: The boring was advanced to the required depth. The piezometer screen and riser were lowered to the bottom of the hole, and the sand pack was tremied into the hole to about 1 foot above the top of the screen. About 2 feet of fine sand was tremied on top, and cement/bentonite grout was tremied in to fill the rest of the hole. The riser was cut off about 1,5± feet above the ground surface, and the protective casing suspended over the riser into the ground until (t set.

NOTES: Typical 50 foot Piezometer Installation



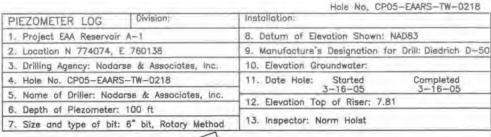


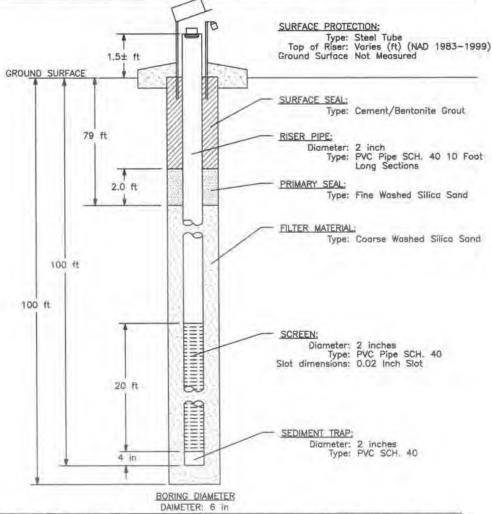




INSTALLATION METHOD: The boring was advanced to the required depth. The piezometer screen and riser were lowered to the bottom of the hole, and the sand pack was tremied into the hole to about 1 foot above the top of the screen. About 2 feet of fine sand was tremied on top, and cement/bentonite grout was tremied in to fill the rest of the hole. The riser was cut off about 1.5± feet above the ground surface, and the protective casing suspended over the riser into the ground until it set.

NOTES: Typical 60 foot Piezometer Installation

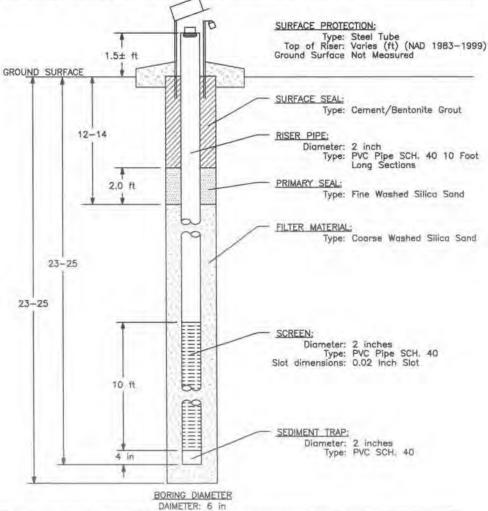




INSTALLATION METHOD: The boring was advanced to the required depth. The piezometer screen and riser were lowered to the bottom of the hole, and the sand pack was tremied into the hole to about 1 foot above the top of the screen. About 2 feet of fine sand was tremied on top, and cement/bentonite grout was tremied in to fill the rest of the hole. The riser was out off about 1.5± feet above the ground surface, and the protective casing suspended over the riser into the ground until it set.

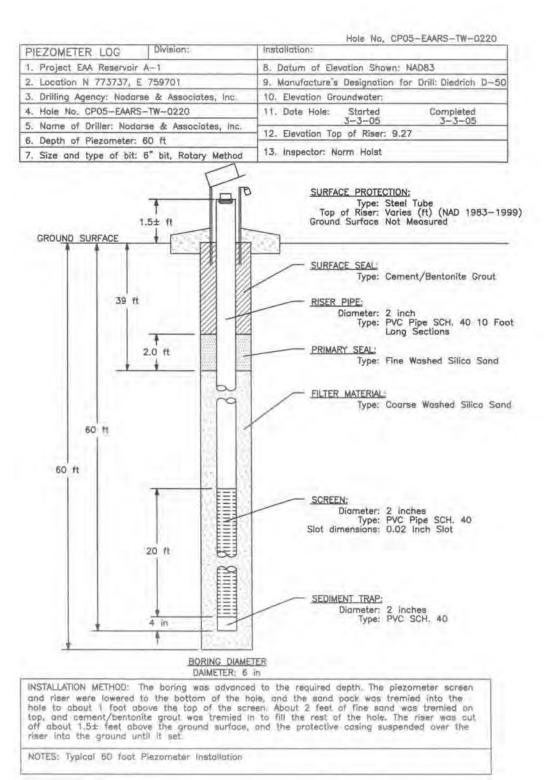
NOTES: Typical 100 foot Piezometer Installation



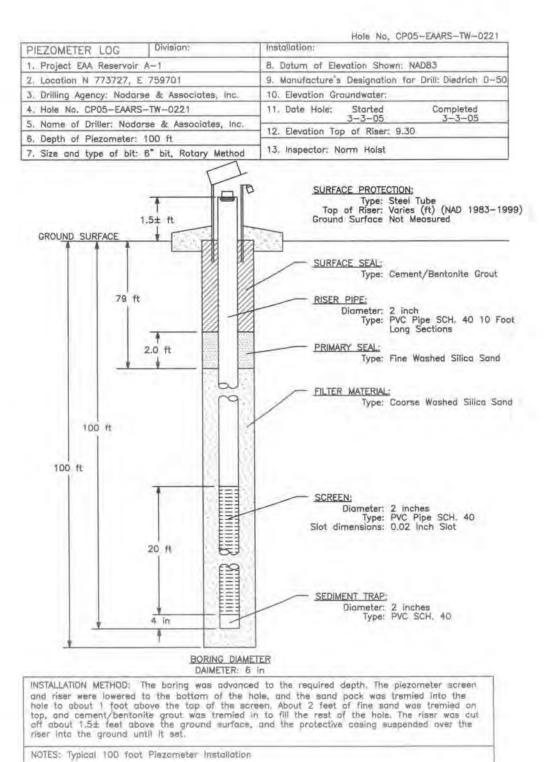


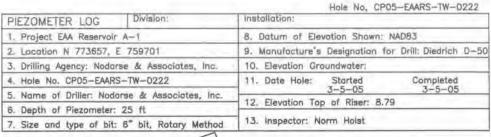
INSTALLATION METHOD: The boring was advanced to the required depth. The piezometer screen and riser were lowered to the bottom of the hole, and the sand pack was tremied into the hole to about 1 foot above the top of the screen. About 2 feet of fine sand was tremied on top, and cement/bentonite grout was tremied in to lill the rest of the hole. The riser was cut off about 1.5 feet above the ground surface, and the protective casing suspended over the riser into the ground until it set.

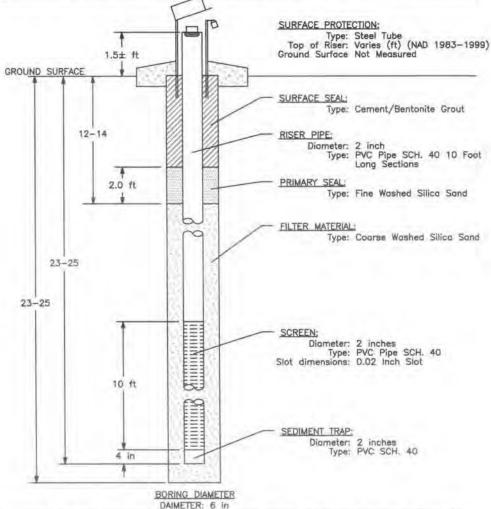
NOTES: Typical 25 foot Piezometer Installation



APPENDIX 1 TEST CELL BORINGS AND PIEZOMETER INSTALLATION LOGS: 221-240

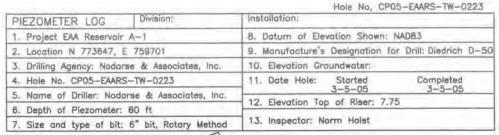


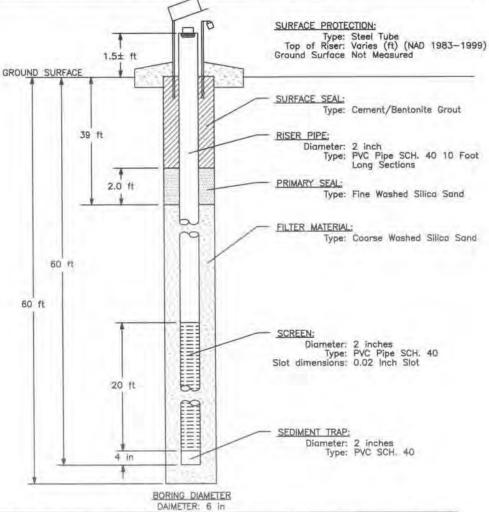




INSTALLATION METHOD: The boring was advanced to the required depth. The piezometer screen and riser were lowered to the bottom of the hole, and the sand pack was tremied into the hole to about 1 foot above the top of the screen. About 2 feet of fine sand was tremied on top, and cement/bentonite grout was tremied in to fill the rest of the hole. The riser was cut off about 1.5± feet above the ground surface, and the protective casing suspended over the riser into the ground until it set.

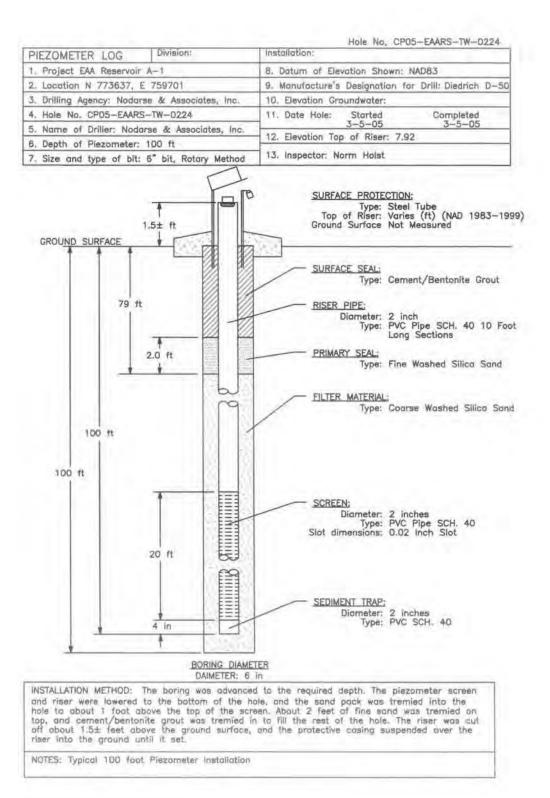
NOTES: Typical 25 foot Plezometer Installation

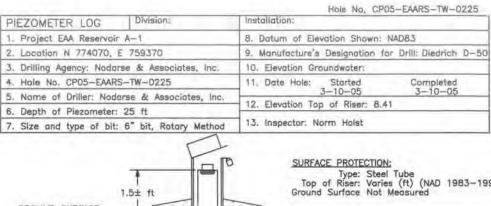


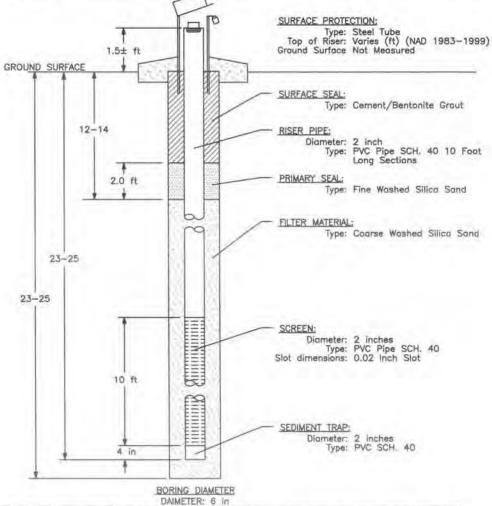


INSTALLATION METHOD: The boring was advanced to the required depth. The piezometer screen and riser were lowered to the bottom of the hole, and the sand pack was tremied into the hole to about 1 foot above the top of the screen. About 2 feet of fine sand was tremied on top, and cernent/bentonite grout was tremied in to fill the rest of the hole. The riser was cut off about 1.5± feet above the ground surface, and the protective casing suspended over the riser into the ground until it set.

NOTES: Typical 60 foot Piezometer Installation





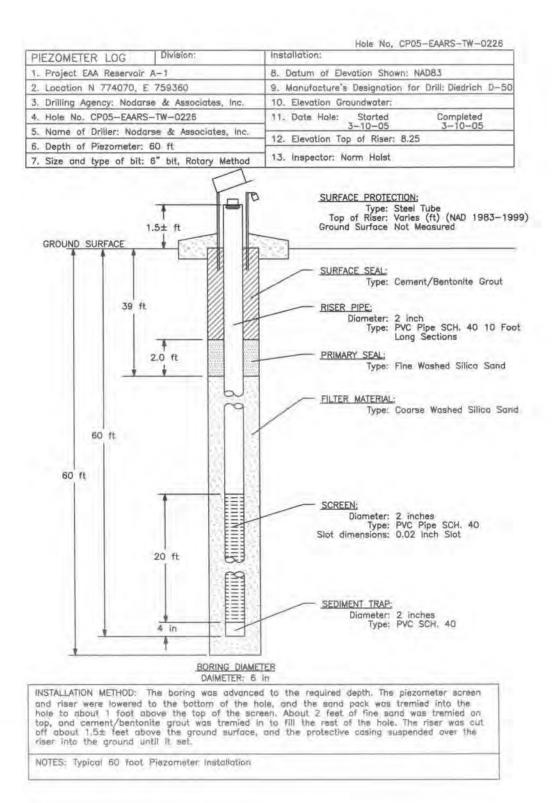


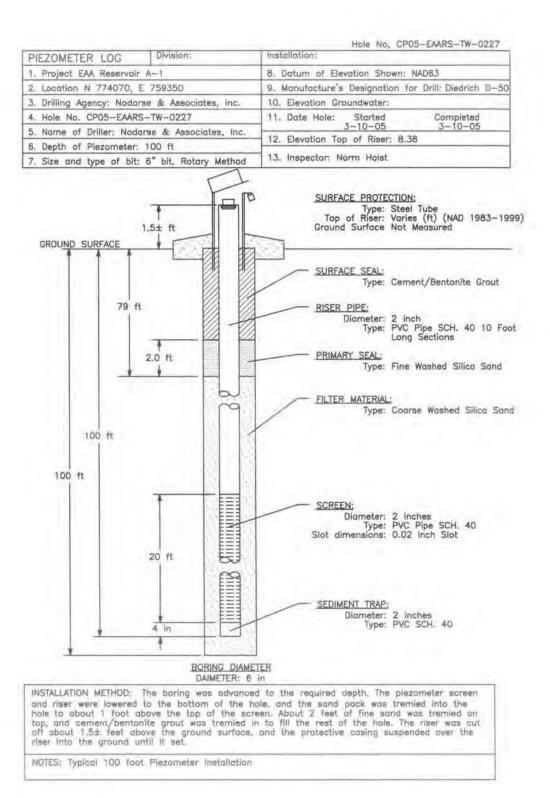
INSTALLATION METHOD: The boring was advanced to the required depth. The piezometer screen and riser were lowered to the bottom of the hole, and the sand pack was tremied into the hole to about 1 foot above the top of the screen. About 2 feet of fine sand was tremied on top, and cement/bentonite grout was tremied in to fill the rest of the hole. The riser was cut off about 1.5± feet above the ground surface, and the protective casing suspended over the riser into the ground until it set.

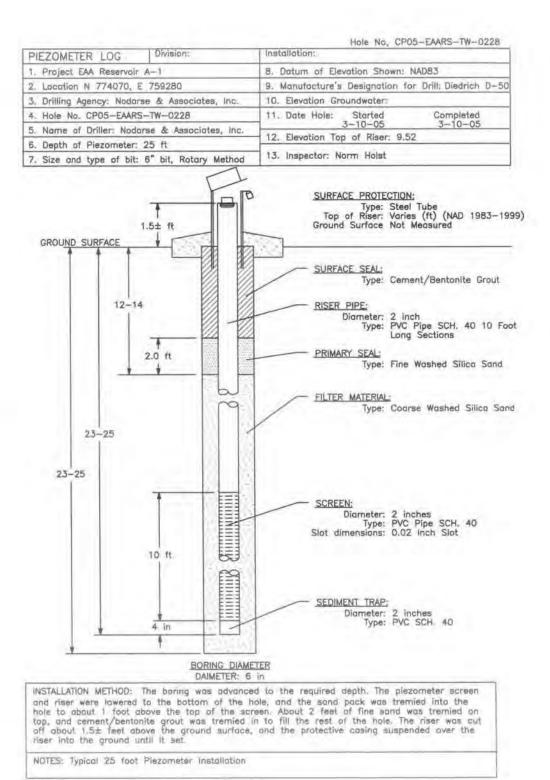
NOTES: Typical 25 foot Piezometer Installation

APPENDIX 2

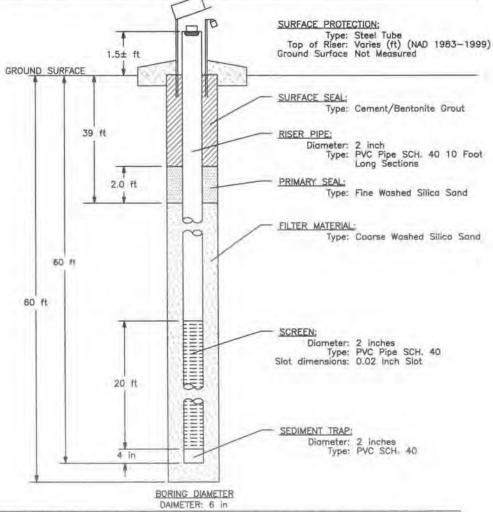
APPENDIX 2 SUPPLEMENTAL BORINGS AND PIEZOMETER INSTALLATION LOGS: 255-260







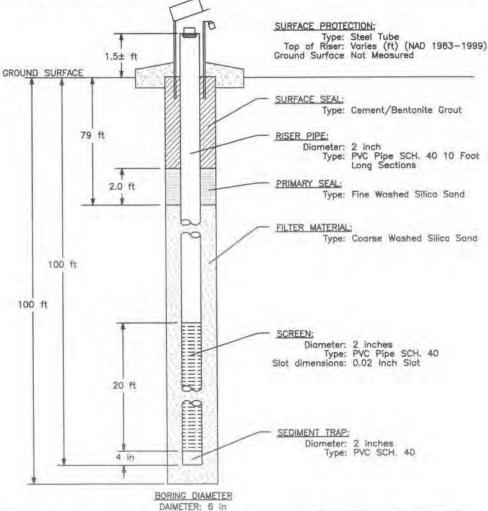




INSTALLATION METHOD: The boring was advanced to the required depth. The piezometer screen and riser were lowered to the bottom of the hole, and the sand pack was tremied into the hole to about 1 foot above the top of the screen. About 2 feet of fine sand was tremied on top, and cement/bentonite grout was tremied in to fill the rest of the hole. The riser was cut off about $1.5\pm$ feet above the ground surface, and the protective casing suspended over the riser into the ground until it set.

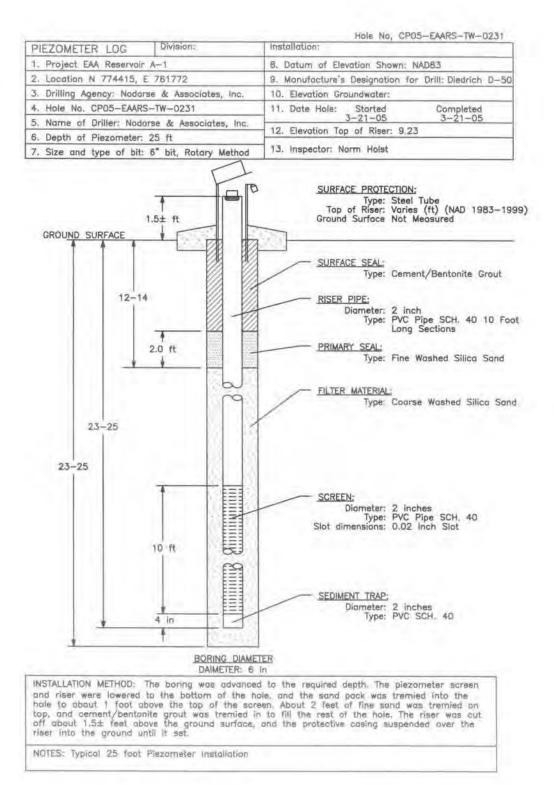
NOTES: Typical 60 foot Piezometer Installation

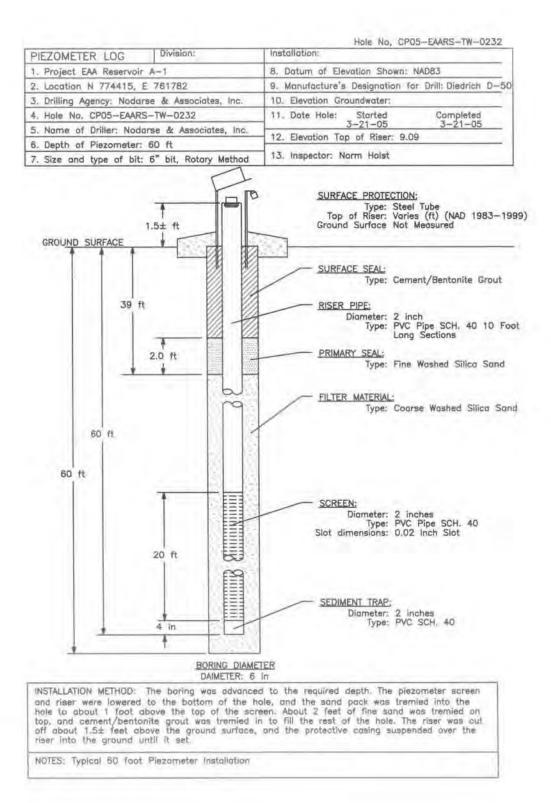


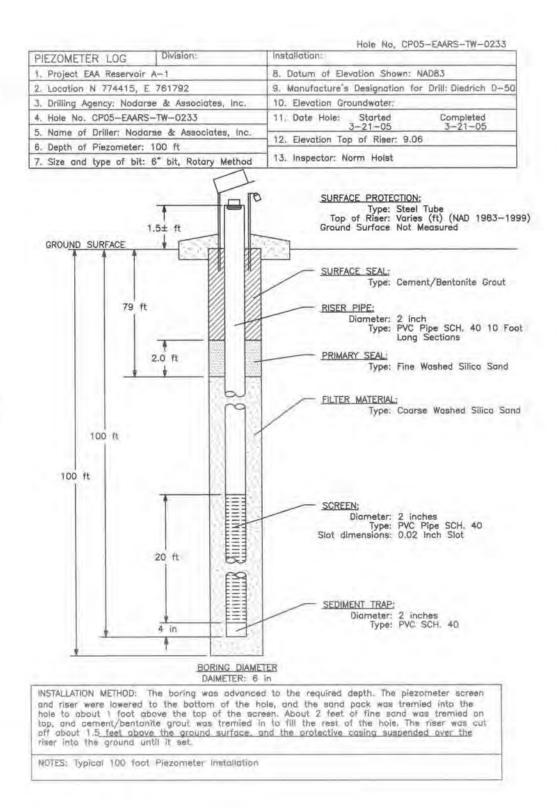


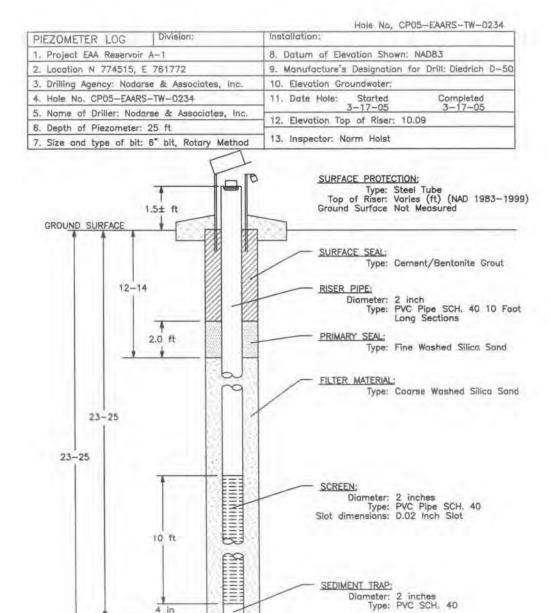
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NOTES: Typical 100 foot Piezometer Installation







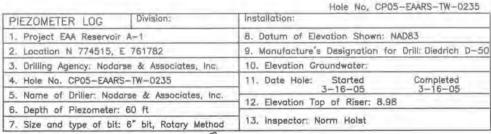


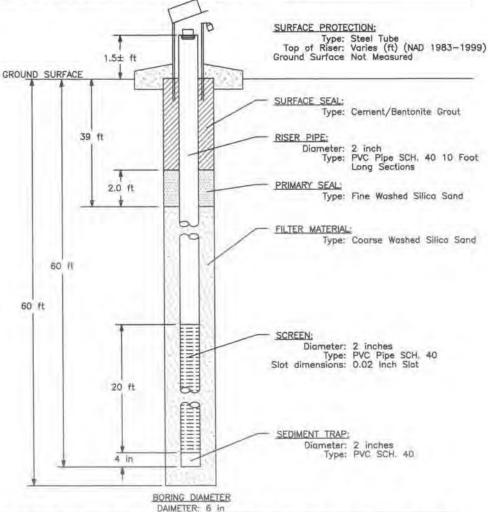
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BORING DIAMETER DAIMETER: 6 in

NOTES: Typical 25 foot Plazameter Installation

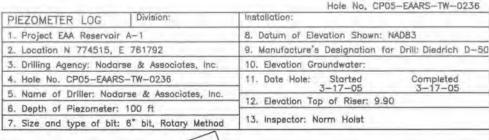
4 in

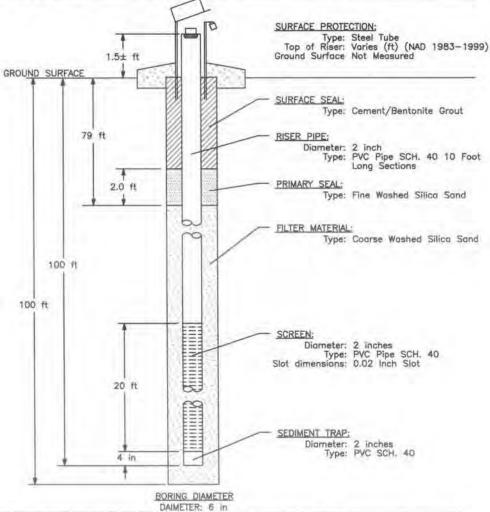




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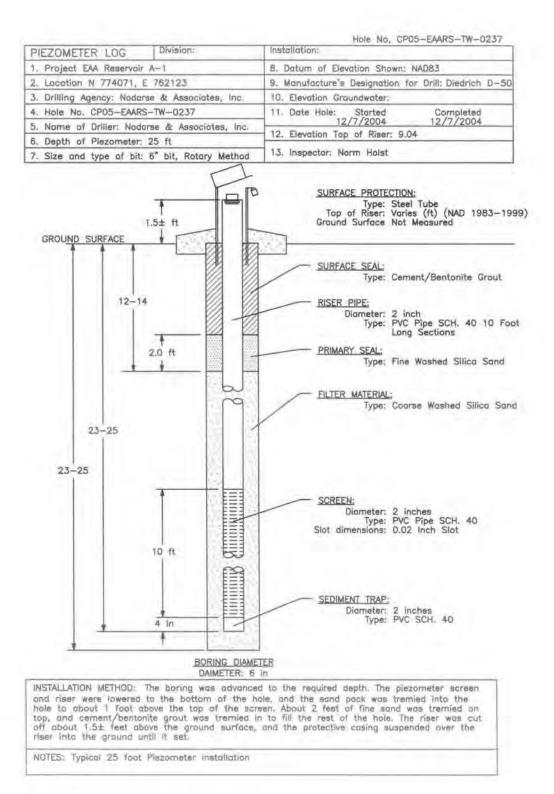
NOTES: Typical 60 foot Piezameter Installation

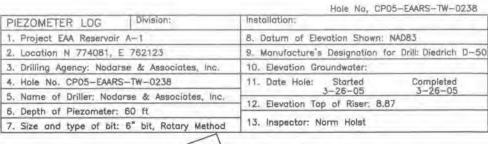


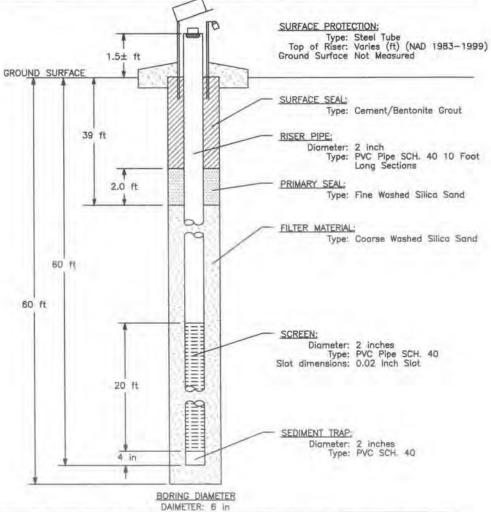


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NOTES: Typical 100 foot Piezometer Installation

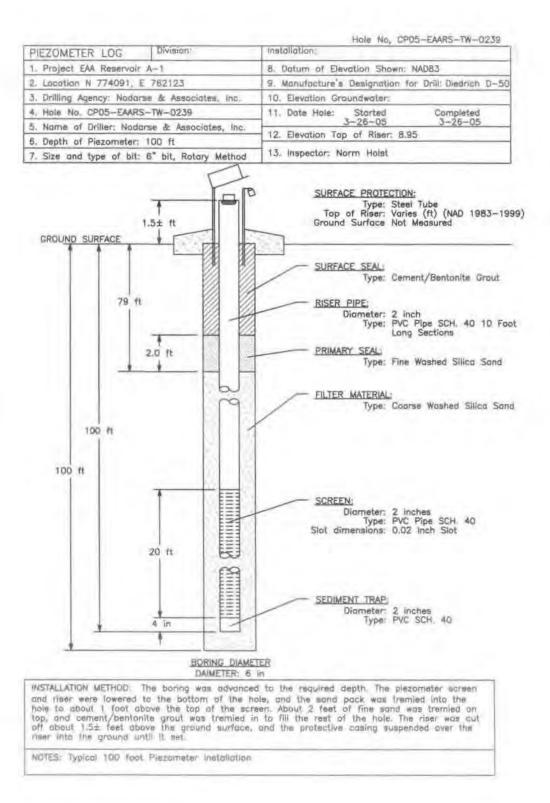


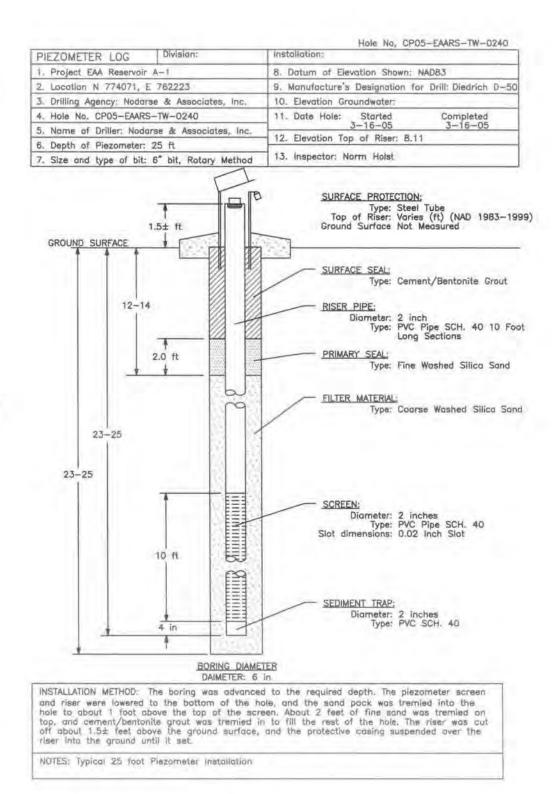




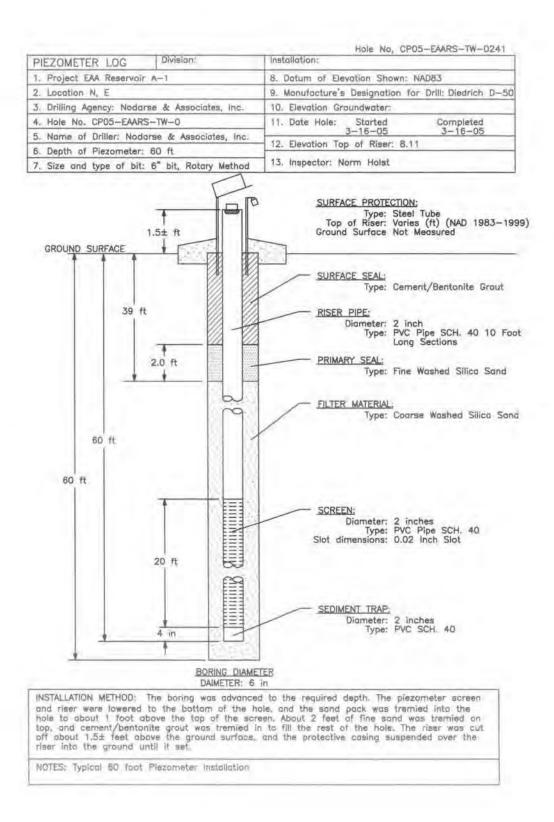
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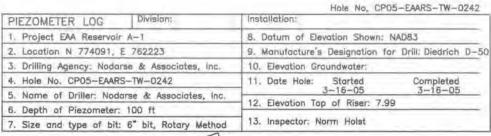
NOTES: Typical 60 foot Piezometer Installation

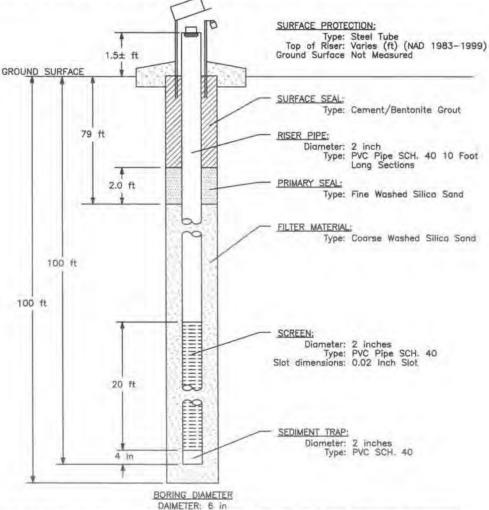




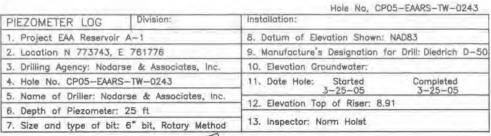
APPENDIX 1 TEST CELL BORINGS AND PIEZOMETER INSTALLATION LOGS: 241-254

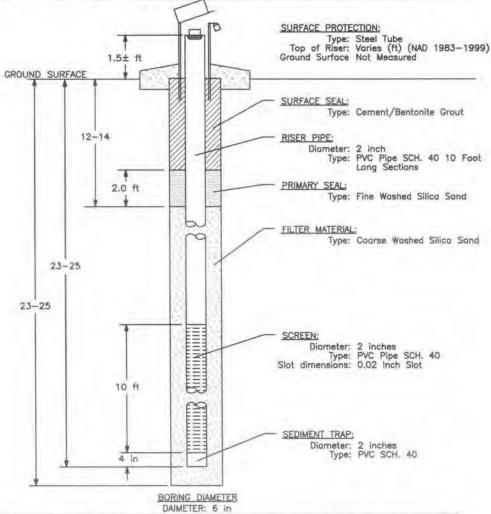




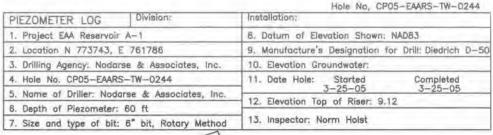


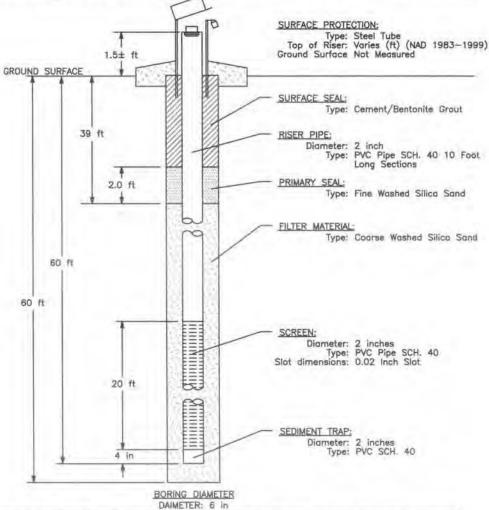
NOTES: Typical 100 foot Piezorneter Installation



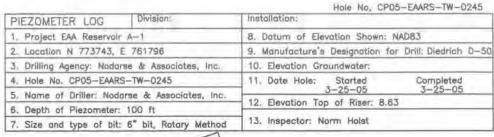


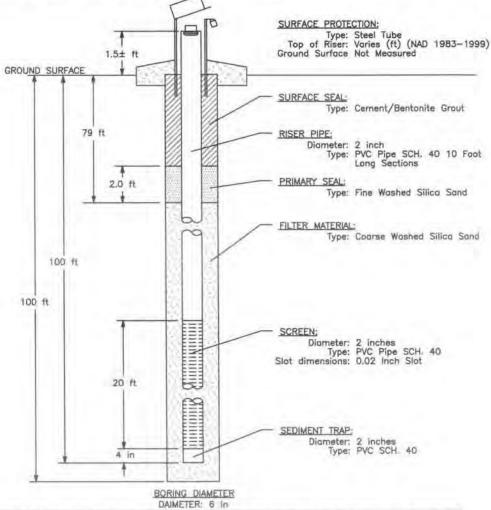
NOTES: Typical 25 foot Piezometer Installation



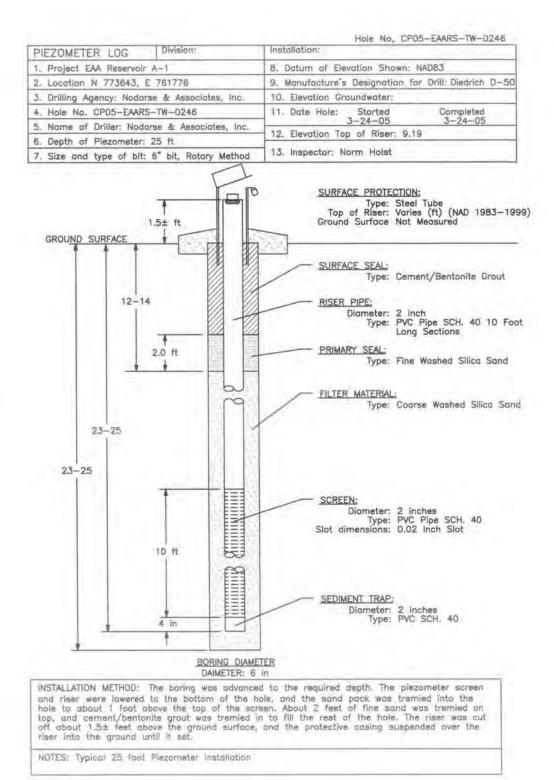


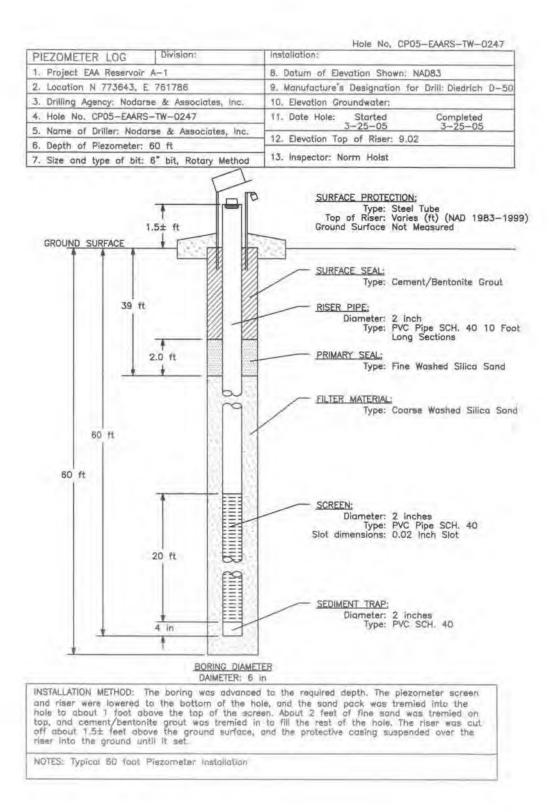
NOTES: Typical 60 foot Piezometer Installation

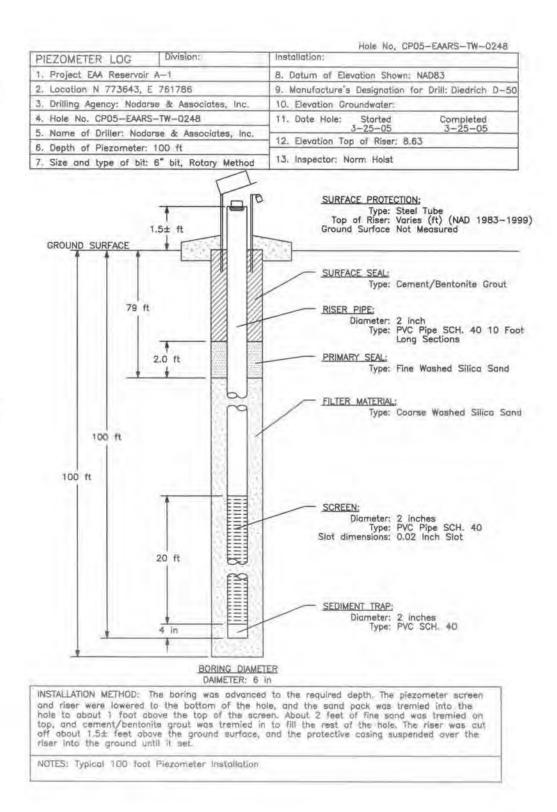


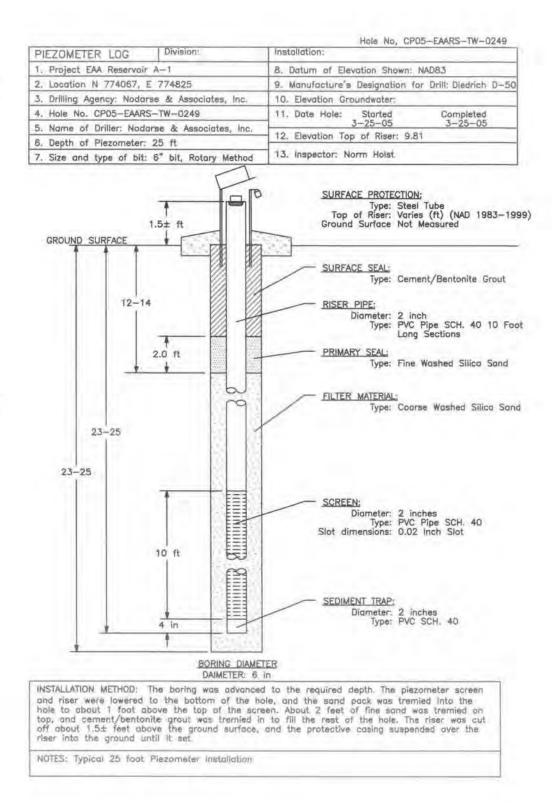


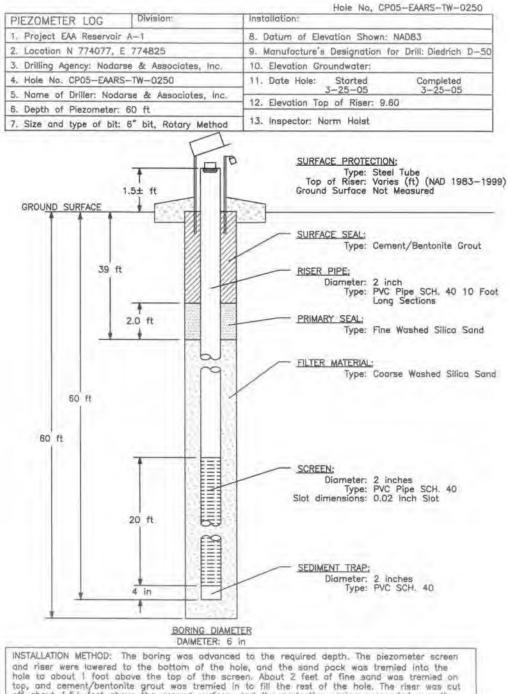
NOTES: Typical 100 foot Piezometer Installation





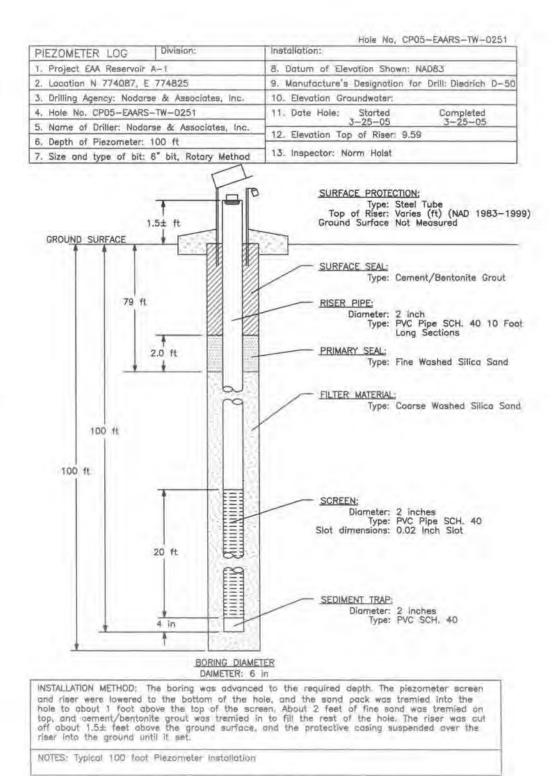


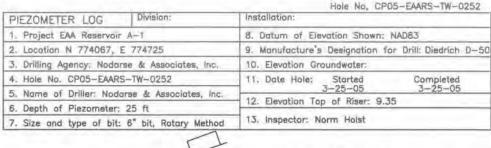


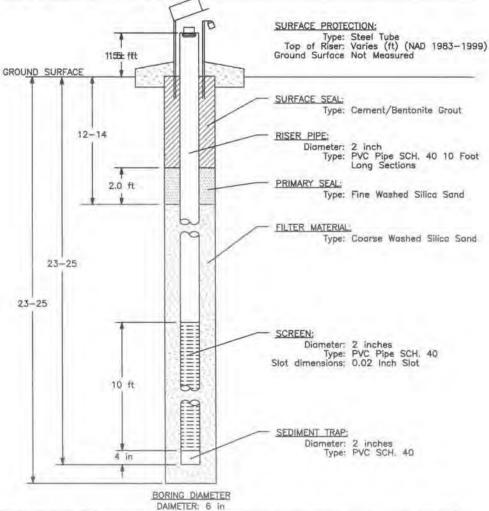


and riser were lowered to the bottom of the hole, and the sand pack was tremied into the hole to about 1 foot above the top of the screen. About 2 feet of fine sand was tremied on top, and cement/bentonite grout was tremied in to fill the rest of the hole. The riser was cut off about 1.5± feet above the ground surface, and the protective casing suspended over the riser into the ground until it set.

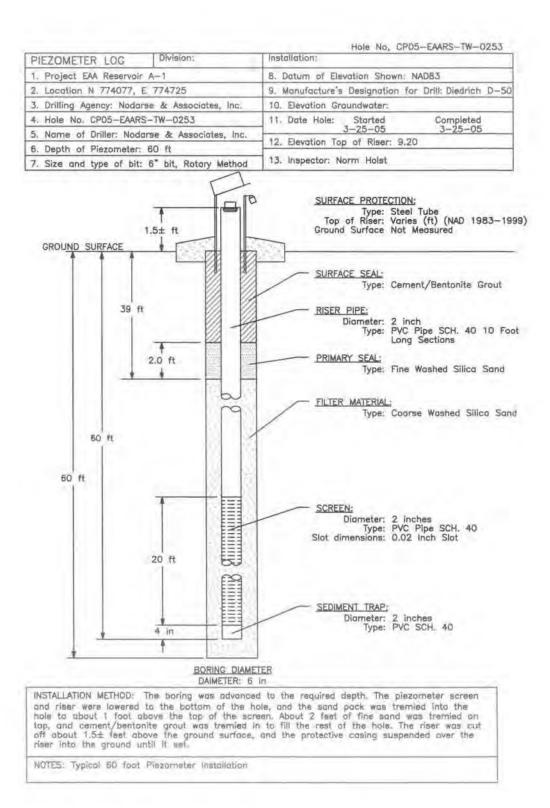
NOTES: Typical 60 foot Piezometer Installation

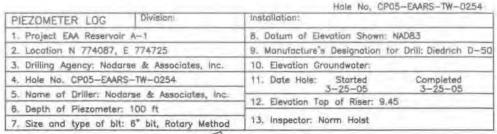


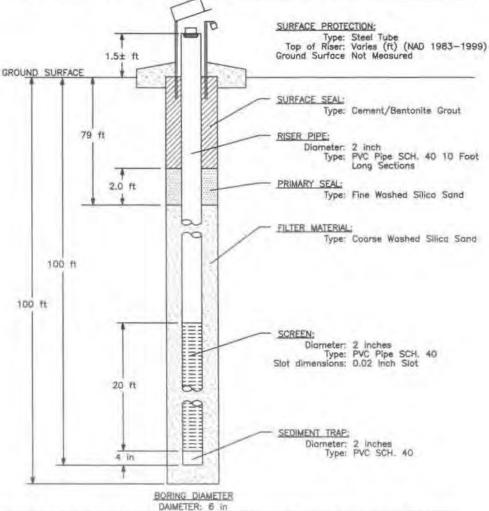




NOTES: Typical 25 foot Piezometer Installation







NOTES: Typical 100 foot Plezometer Installation

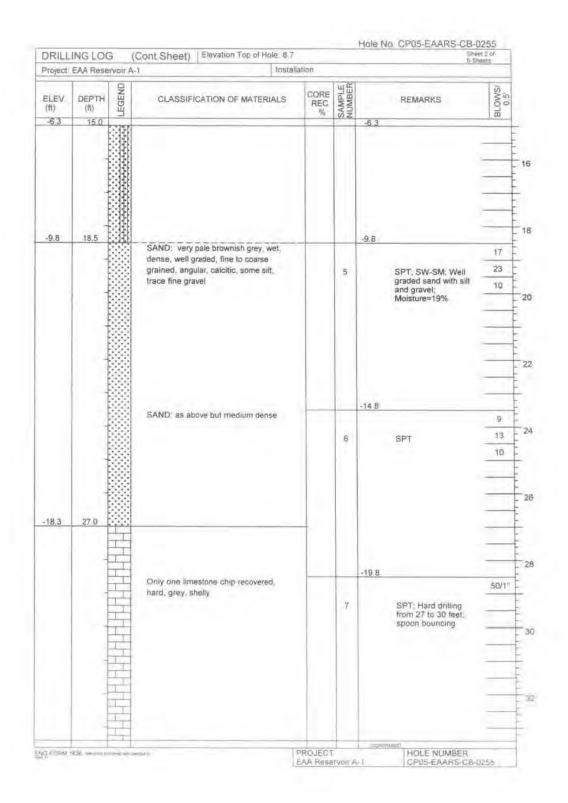
2224/2006 12:20:17 IIM

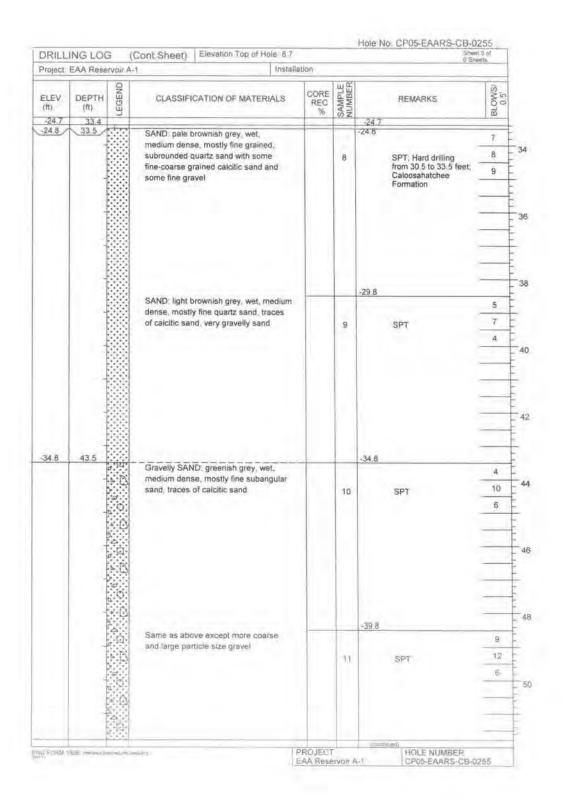
SOIL CLASSIFICATION CHART

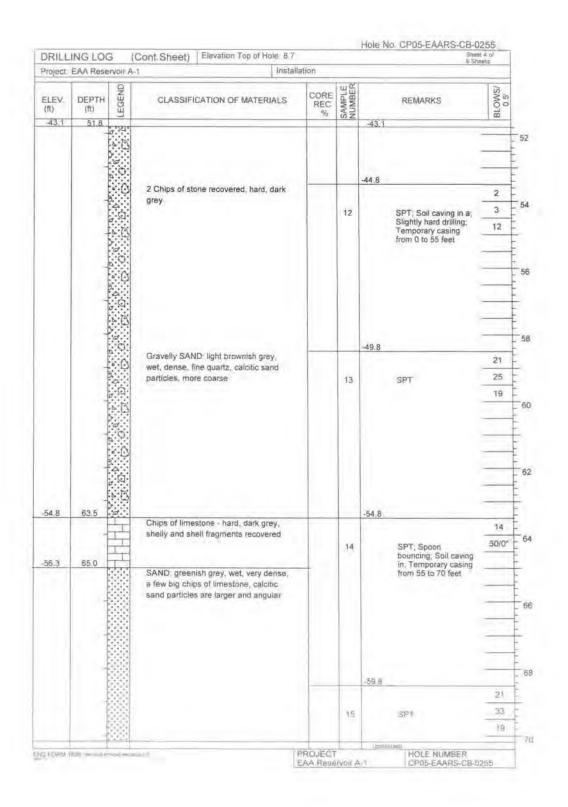
- 10	ONE	SYME	BOLS	TYPICAL		
IVI	AJOR DIVISI	UNA	10000 10000 10000 10000	LETTER	DESCRIPTIONS	
	GRAVEL AND	CLEAN GRAVELS	GRAVELS		WELL-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES	
COARSE GRAINED SOILS	GRAVELLY SOILS	(LITTLE OR NO FINES)		GP	POORLY-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES	
	MORE THAN 50% OF COARSE FRACTION	GRAVELS WITH FINES		GM	SILTY GRAVELS, GRAVEL - SAND - SILT MIXTURES	
	RETAINED ON NO. 4 SIEVE	(APPRECIABLE AMOUNT OF FINES)		GC	CLAYEY GRAVELS, GRAVEL - SAND CLAY MIXTURES	
MORE THAN 50% OF MATERIAL IS	SAND	CLEAN SANDS		sw	WELL-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES	
LARGER THAN NO. 200 SIEVE SIZE	SANDY	(LITTLE OR NO FINES)		SP	POORLY-GRADED SANDS, GRAVELLY SAND, LITTLE OR NO FINES	
	MORE THAN 50% OF COARSE	SANDS WITH FINES		SM	SILTY SANDS, SAND - SILT MIXTURES	
	PASSING ON NO 4 SIEVE	(APPRECIABLE AMOUNT OF FINES)		sc	CLAYEY SANDS, SAND - CLAY MIXTURES	
		LIQUID LIMIT LESS THAN 50		ML	INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY OR CLAYEY FINE SANDS OR CLAYEY SILTS WITH SLIGHT PLASTICITY	
FINE GRAINED SOILS	SILTS AND CLAYS			CL	INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS, LEAN CLAYS	
0.0.0				OL	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY	
MORE THAN 50% OF MATERIAL IS SMALLER THAN NO. 200 SIEVE				МН	INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SAND OR SILTY SOILS	
SIZE	SILTS AND CLAYS	LIQUID LIMIT GREATER THAN 50		СН	INORGANIC CLAYS OF HIGH PLASTICITY	
				ОН	ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY ORGANIC SILTS	
HIGHLY DRGANIC SOILS			10 37 07 7 1 17 17 17 10 17 17 17	PT	PEAT, HUMUS, SWAMP SOILS WITH HIGH ORGANIC CONTENTS	

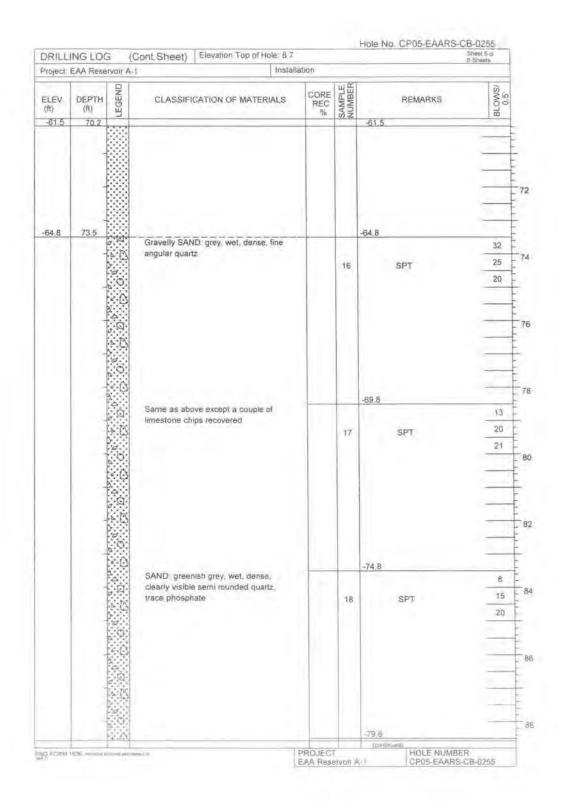
NOTE DUAL SYMBOLS ARE USED TO INDICATE BORDERLINE SOIL CLASSIFICATIONS

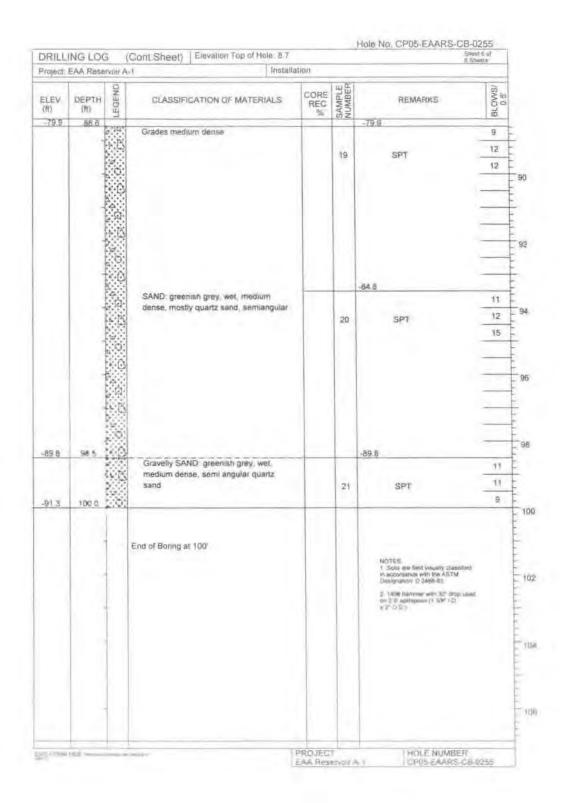
DRILLING LOG Division					Hole No. CP05-EAARS-CB-0255 Installation 6 Sheets					
1 Project EAA Reservoir A-1					10 Size and type of bit 3" bit. Rotary Method					
	-		758378.8 - NACI 1983	11 Datum for Elevation Shown, NAVD 1988						
			e & Associates, Inc.	12 Manufacturer's Designation for Drill Diedrich D-50						
	No CP05			13 Total Number of Overburden Samples Taken. N/A						
	of Driller		Williams	14. Total Number of Core Boxes. 1						
	tion of Hole ertical		ned	15 Elevation Ground Water Not measured						
				15 Date Hole Started Completed 6/30/2005 5/30/2005						
	ness of Bu			17 Elevation Top of Hole: 8.7 (ft)						
	ness of ca		N/A	18. Total Core Recovery for hole: N/A						
9. Depth of hole: 100 ft				19 Ins	pector, N.	Holst	and A.M	M. Noronha	1-	
ELEV (ft)	DEPTH (ft)	LEGEND	CLASSIFICATION OF MATE	RIALS	CORE REC	SAMPLE		REMARKS	BLOWS/ 0.5'	
8.7	0.0	276-2	DEAT							
	1	4 90	PEAT		1		B.7		7	
7.7	10	311. 3				1	7.7	SPT. At least part of	50	
		1	LIMESTONE Ian to light brown,			1.11		the calcilic material noted below is shell		
		17	fine grained with some shells, the	inty	120			tragments		
	-	T	bedded, hard, strong, vuggy		(RQD	7		HQ coring	-	
		T			16%)					
		T								
		T				1				
	1 3	T								
		1								
		1								
27	6.0	1					2.7			
		33		SAND: very pale brown, wet, medium			2.2			
		Talk and the second sec	dense, well graded, fine to medi- grained, angular calculic trace in						2	
	1		granted, angular, carcine bace and					100	7	
						5	SPT	SPT	4.	
		333							3.5	
									7.5	
na.	90						F 0-		31	
-0.3	90	10	Sifty SAND white, wet loose, w	ell			-0.3			
-0.3	90		graded, fine to coarse grained,				-03		5	
-0.3	90					3	-0.3	SPT SM. Bilty sand	5 4	
-0.3	90		graded, fine to coarse grained,			3	-0.3	SPT_SMLBilty sand with grave! Moisture=33%	5	
-0.3	90		graded, fine to coarse grained,			3	-0.3	with gravel	5 4	
-0.3	90		graded, fine to coarse grained,			3	-0.3	with gravel	5 4	
-0.3	90		graded, fine to coarse grained,			3	-0.3	with gravel	5 4	
-0.3	90		graded, fine to coarse grained,			3	-03	with gravel	5 4	
-0.3	90		graded, fine to coarse grained,			3	-0.3	with gravel	5 4	
-0.3	90		graded, fine to coarse grained,			3	-0.3	with gravel	5 4	
-0.3	90		graded, fine to coarse grained,			3		with gravel	5 4	
-0.3	90		graded, fine to coarse grained, a calcitic	angular,		3	-03 -48	with gravel	5 4	
-0.3	90		graded, fine to coarse grained, a calcitic	angular,				with grave!; Moisture=33%	5 4 3	
-0.3	90		graded, fine to coarse grained, a calcitic	angular,		3		with gravel	5 4 3	

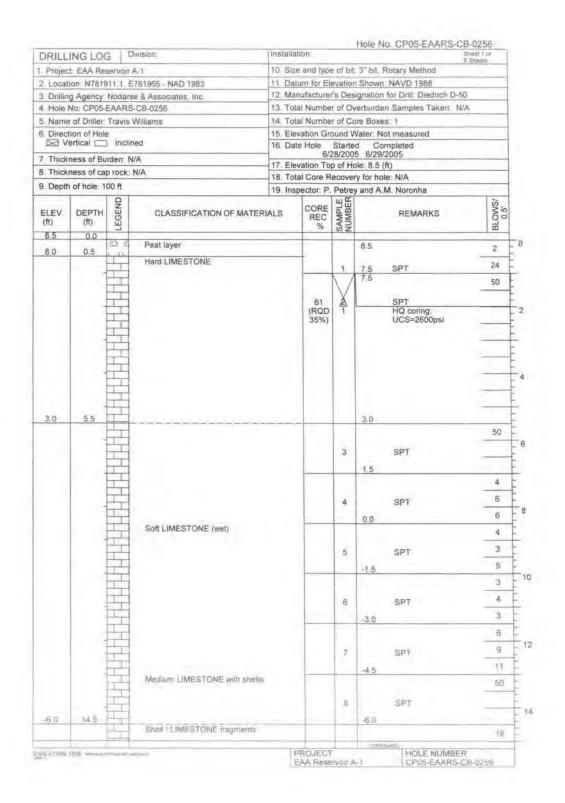


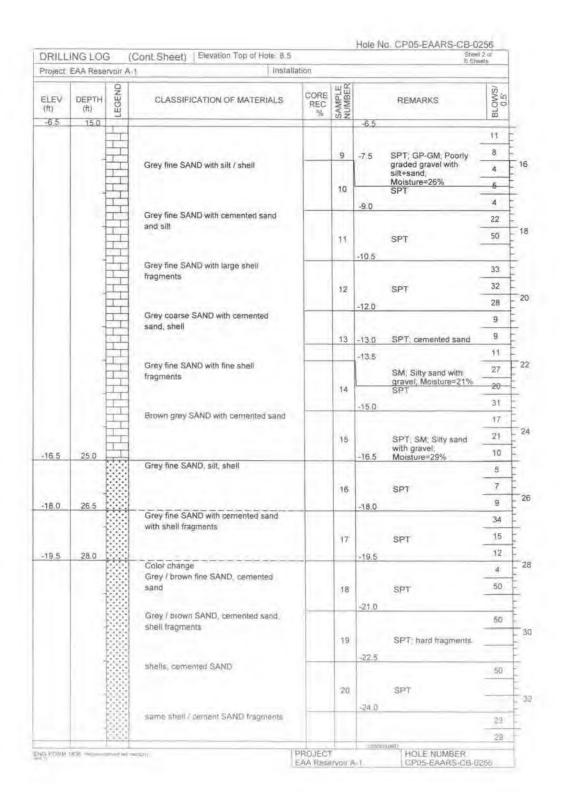




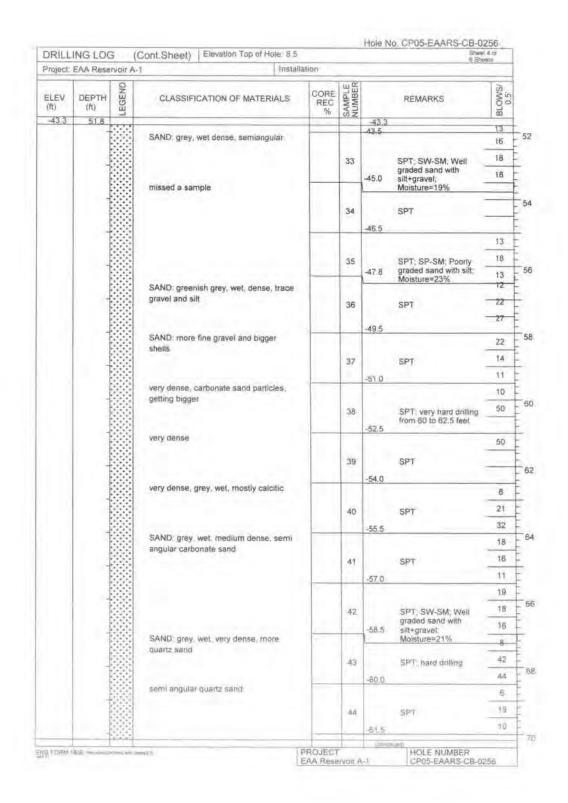




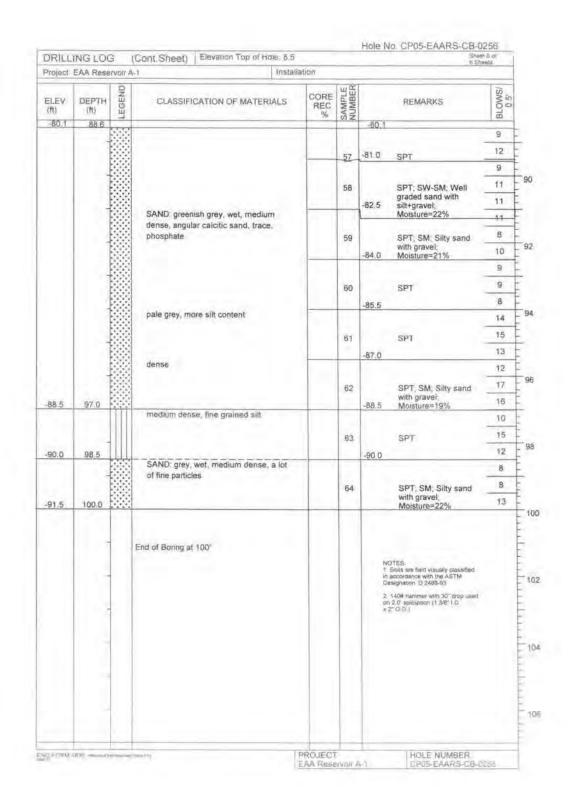


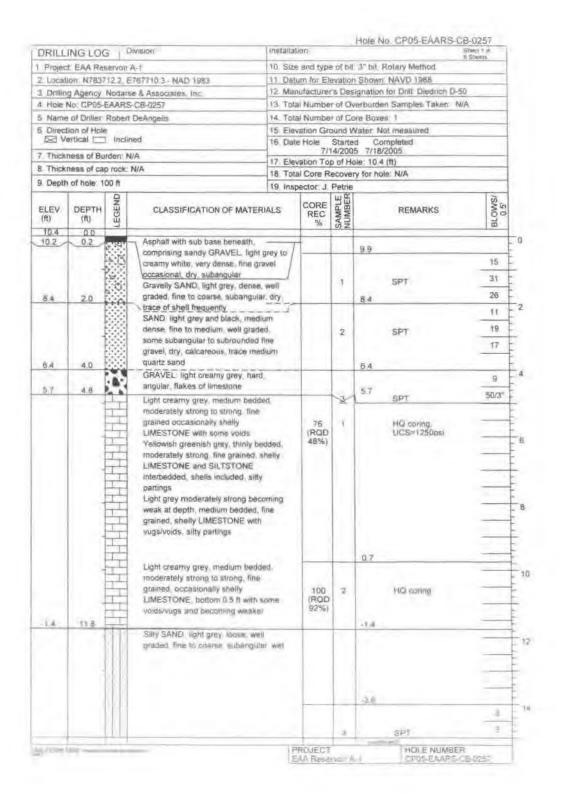


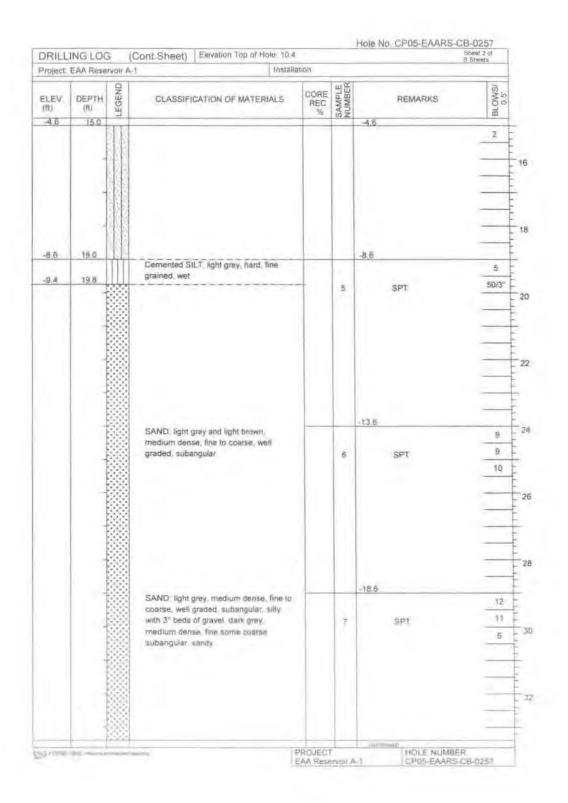
oject.	EAA Reser	Voir A-1	ont Sheet) Elevation Top of Hole 8.5	ation		0.34	sets
LEV 1)	DEPTH (ft)	LEGEND	CLASSIFICATION OF MATERIALS	CORE REC %	SAMPLE	REMARKS	BLOWS/ 0.5'
24.9	33.4	111				-24.9	30
	1		shell fragments to fine grey SAND with	-	1	-25.5 SPT	20
		small shell fragmens / cemented sands		34		18	
	1				22	SPT	27 46
	1		grey fine SAND - shell fragment and			-27.0	29
	1		cemented sand		100	about a	40
	1				23	SPT	33
	1		Large rock (cemented SAND) with shell			-28.5	12
			/ sand (grey, brown)			could-	16
	1				24	SPT	12
			grey / brown fine sand with small shells	-		-30.0	3
	1						3
	1				25	SPT; SM; Silty sand with gravel;	4
	1		grey / brown silty fine sand with	-		-31.5 Moisture=22%	3
			cemented sand				6
					26	SPT	5
3.0	41.5		dark brown siightly sandy CLAY	+		-33.0	2
	1				-	SPT	3
		same		27		4	
					-34.5	3	
				28	28	SPT	3
					-36.0	2	
		dark brown slightly CLAY, trace sand	dark brown slightly CLAY, trace sand			-50.0	4
					29	SPT; SP-SM; Poorly	4
7.5	46.0				2.0	graded sand with silt37.5 Moisture=31%	5
			dark brown silty fine SAND with shell			STID MINISTER OF THE	10
					30	SPT	29
9.0	47.5				1	-39.0	31
			greyish brown fine SAND with shell				11
			31	SPT	10		
				15		12	
			SAND: greenish grey, wet, fine to coarse angular calcetic sand, with some				
		306	coarse angular calcetic sand, with some fine grained rounded quartz, trace sill			lan T	
						42.0	
					1		10-
					32	SPT	15

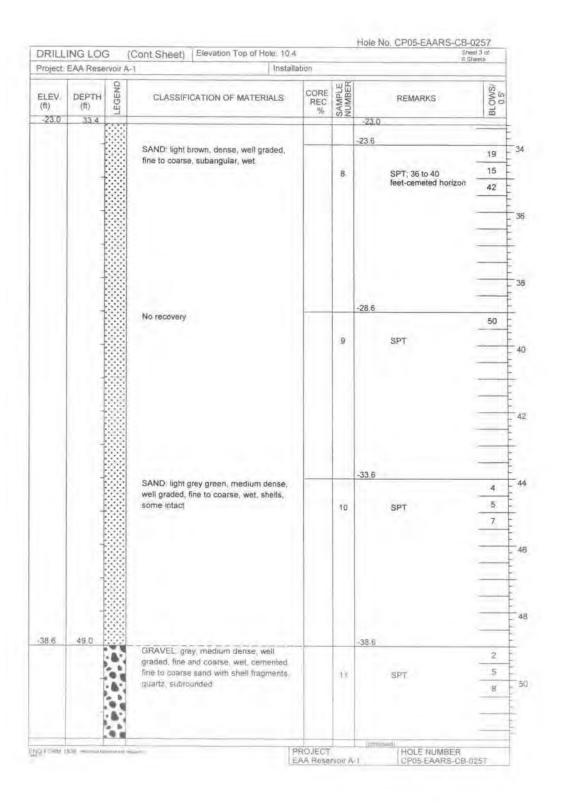


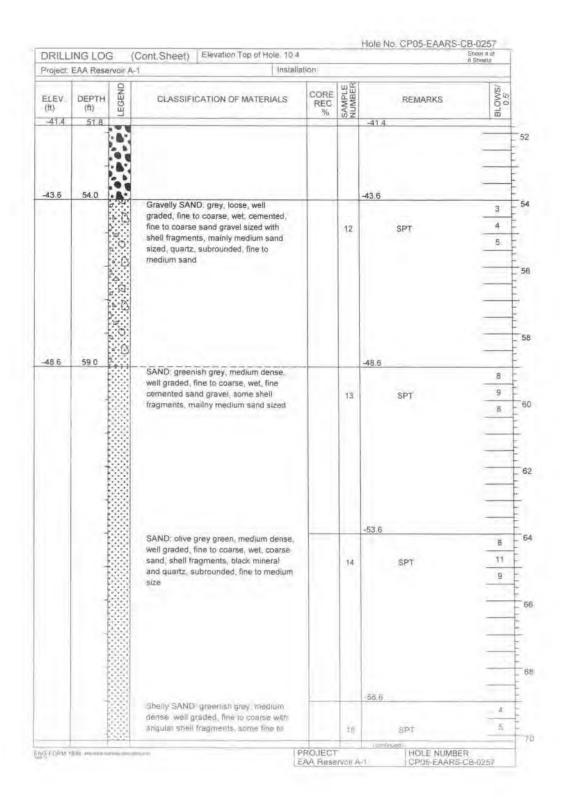
	ING LOC EAA Rese	-	ont Sheet) Elevation Top of Hole 8.5	tion	_	ý Sh	eett	
Tuject	LAN nose	-	Histolie	ritori .	1 100		_	
LEV (ft)	DEPTH (ft)	LEGEND	CLASSIFICATION OF MATERIALS	CORE REC %	SAMPLE	REMARKS	BLOWS/	
-61.7	70.2	23.52	darker shade of grey, semiangular		-	-61.7	5	
			carbonate and quartz sand			All the state of the sale	5	
	1				45	SPT; SP-SM; Poorly graded sand with silt;	4	
		1333	1838	Loose sand			-63.0 graded sand with silt; Moisture=28%	3
	9					3.2	4	
					46	SPT	- 6	
	1		medium dense, semi round quartz and	-		-64.5	4	
			carbonte sand, traces of phosphate		121	33	16	
	1				47	SPT	-	
			semi round quartz and carbonate sand	-	-	-66.0	13	
	1	***	Service desired and service services		-		13	
					48	SPT	21	
	1	92.91	SAND: pale greenish grey, wet, medium			-57.5	17	
			dense, semiangular carbonate sand				9	
	1				49	SPT; SP-SM, Poorly graded sand with	14	
						-69.0 silt+gravel;	10	
	1					Moisture=28%	-6	
		333		50	SPT	12		
	1		Approximate and a second			-70.5	12	
	1		bigger quartz sand crystalls, semi rounded				9	
	1		(ourse)	51	51	SPT	16	
					-72.0	15		
			Dense SAND, traces of phosphate				15	
					52	SPT; SP-SM; Poorly	20	
	1	33.5				graded sand with silt: -73.5 Moisture=20%	17	
			Dense, wet semi round quartz and carbonate sand				15	
			Carbonate sand		53	SPT	18	
						-75.0	20	
							12	
		353			54	SPT	16	
			SAND wet, medium dense, carbonate		1	-76.5	16	
	1					- Navide	11	
		sand particle size is bigger	sand particle size is bigger		55	SPT; SW-SM; Well	13	
	1				20	graded sand with	15	
						-78.0 silt+gravel; Molsture≃26%	13	
	1			56	no.	PAT	17	
					56	SPT	19	
	1		medium dense, light brown angular	-	-	-79.5	8	
_	carbonate SAND				(po::limwed)	0		

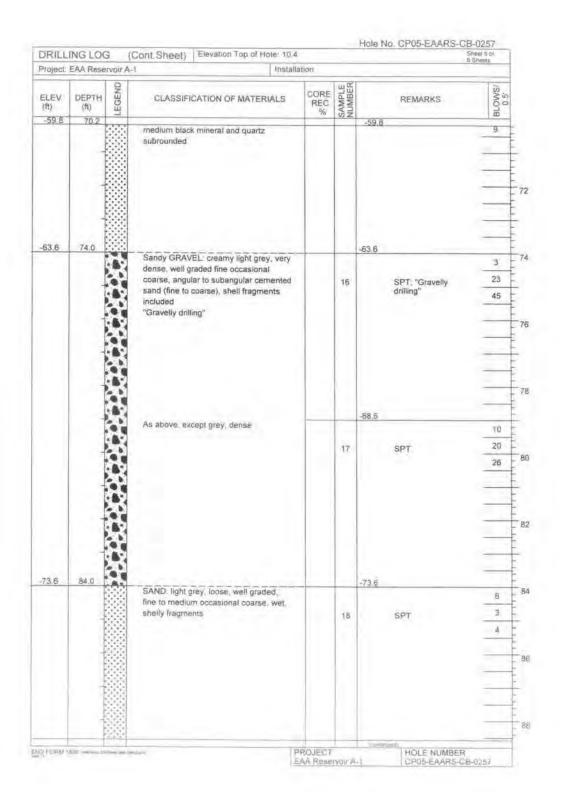


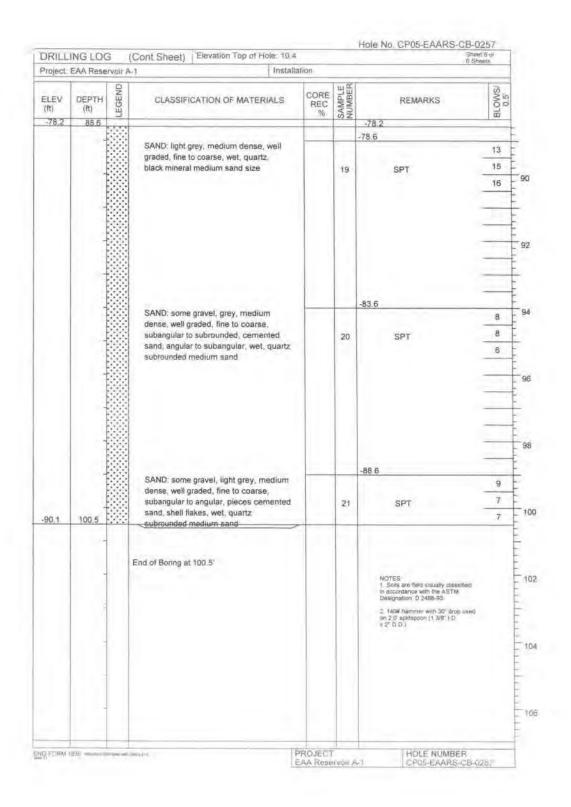




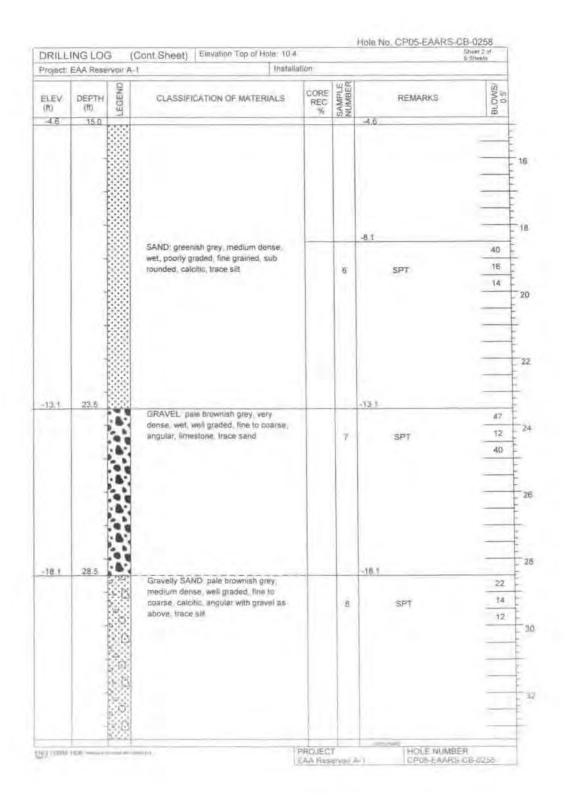


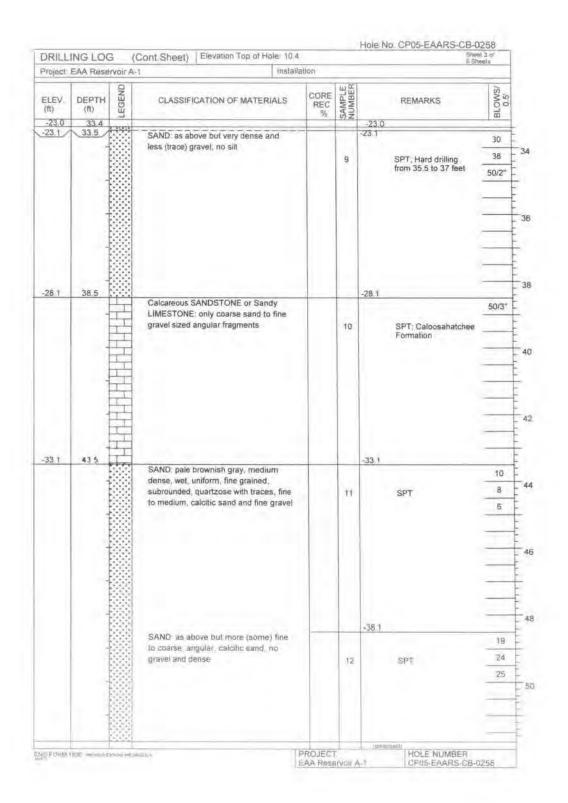


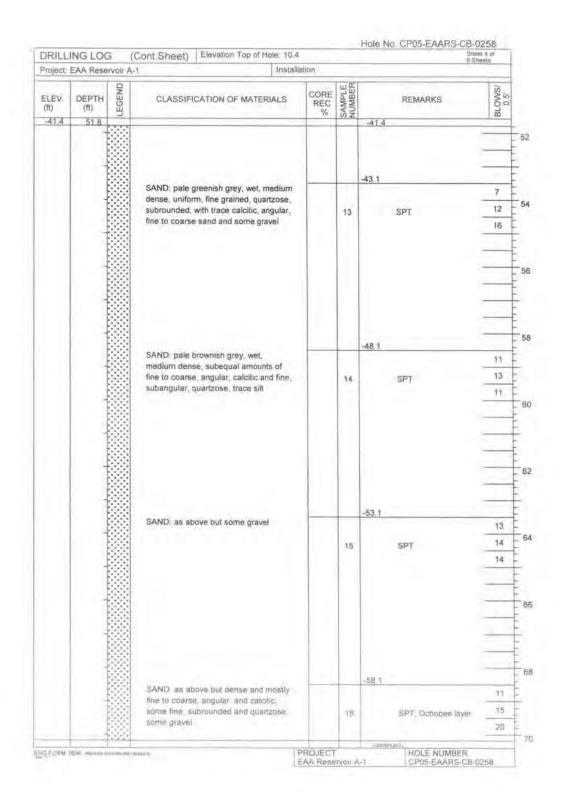


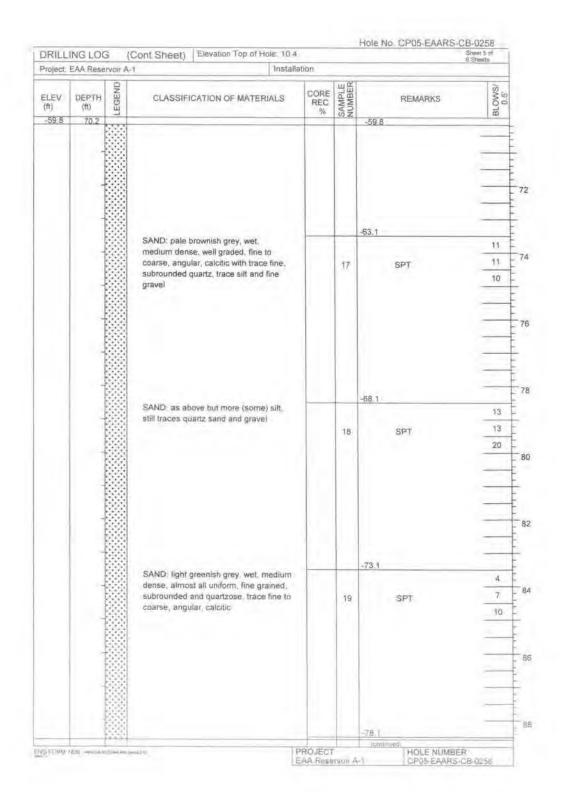


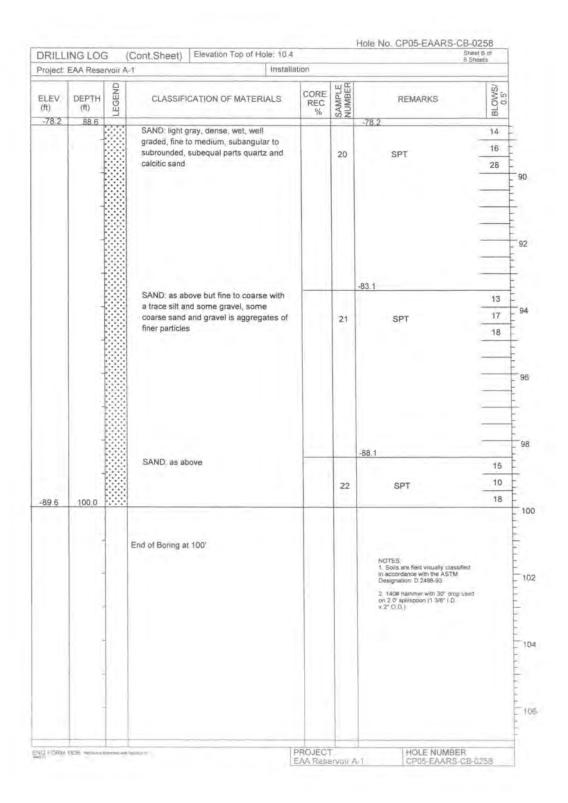
DRILLING LOG Division					Hole No. CP05-EAARS-CB-0258						
Projec	± EAA Re	Servoir	A-1	10. Size and type of bit. 3" bit. Rotary Method							
2 Locat	ion: N780	135.8, E	770597 1 - NAD 1983	11. Datum for Elevation Shown NAVD 1988							
			e & Associates, Inc.	12 Manufacturer's Designation for Drill: Diedrich D-50							
4 Hole I	No: CP05	EAARS	-CB-0258	13. Total Number of Overburden Samples Taken: N/A							
5. Name	of Driller	Enk Bl	iemke	14 Total Number of Core Boxes: 1							
6. Direction of Hole					15. Elevation Ground Water, Not measured						
⊠ Vertical ☐ Inclined					16 Date Hote Started Completed 8/5/2005 8/9/2005						
7. Thickness of Burden: N/A					17 Elevation Top of Hole: 10.4 (ft)						
5. Thick	ness of ca	N/A	18. Total Core Recovery for hole. N/A								
9. Depth	of hole: 1	19. Inspector: N. Holst									
ELEV (ft)	DEPTH (ft)	EGEND	CLASSIFICATION OF MATER	RIALS	CORE REC %	SAMPLE		REMARKS	BLOWS/		
10.4	0.0	30 %	PEAT brownish black, moist loo	ise fine		U) Z	10.4				
		444	grained, non-plastic, organic, roo	ts		1		SPT, All calcilic	3		
		4 46					8.9	material noted below is at least in part	3		
		45 1					0.5	shells.	2		
	1	20 110				2		CDT	2		
		4 70				4	2.4	SPT	3		
		343					14		2		
		5 00			1	3		SPT	-		
		100							27		
5.9	4.5	406					5.9		50/4"		
		1	LIMESTONE: tan to light gray an								
	1	T	prown, thinly bedded, fine to coarse grained, fossiliferous, viggs, strong ar hard to soft, weak and porous			Y	New York Towns	-			
		T			IRQD		HQ coring Ranges from wackestone to grainstone		-		
			Haid to done would arise portone	to to dott. Weak and paroni							
		1			1 1	1					
		1									
	100	H									
		T							-		
									-		
1.9	8.5	1									
1.00	0.0	g.331	Gravelly SAND: light brownish g	ray							
		P. 17.	very dense, wet, well graded. In						-		
	100	of .	coarse grained, angular calcilic.	some		-	0.9				
		0	and						33		
						4		SFT: Hard drilling	50/3"		
								from 10 to 11.5 feet			
									_		
	1										
		688							-		
		CO				1			-		
-2.1	13.5	1					31				
				SAND light grey martium densi						14	
			moderately graded. This is mode grained argular matths, some			5		792	12		
		333				100	-		94		
		200		_	-	-	700				

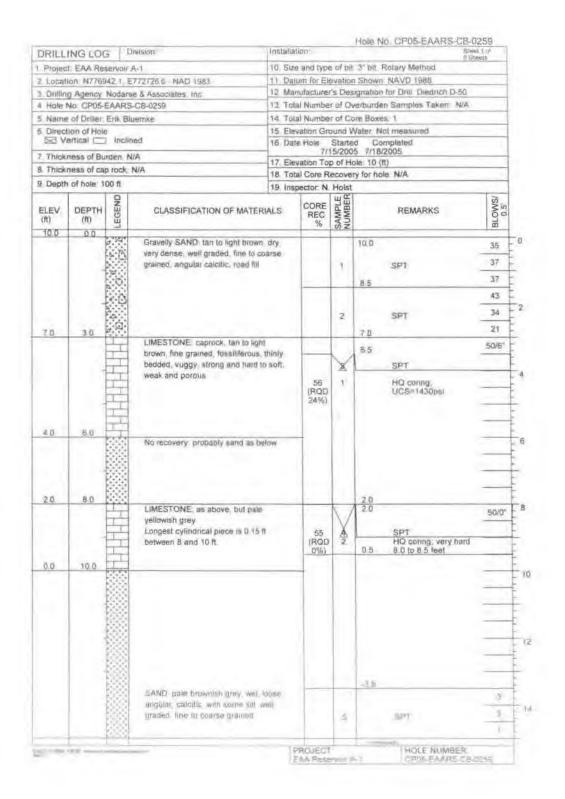


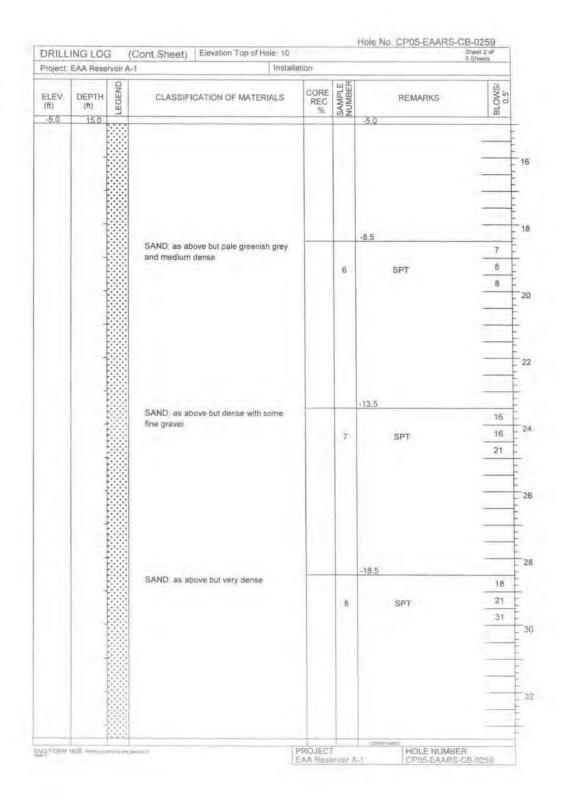


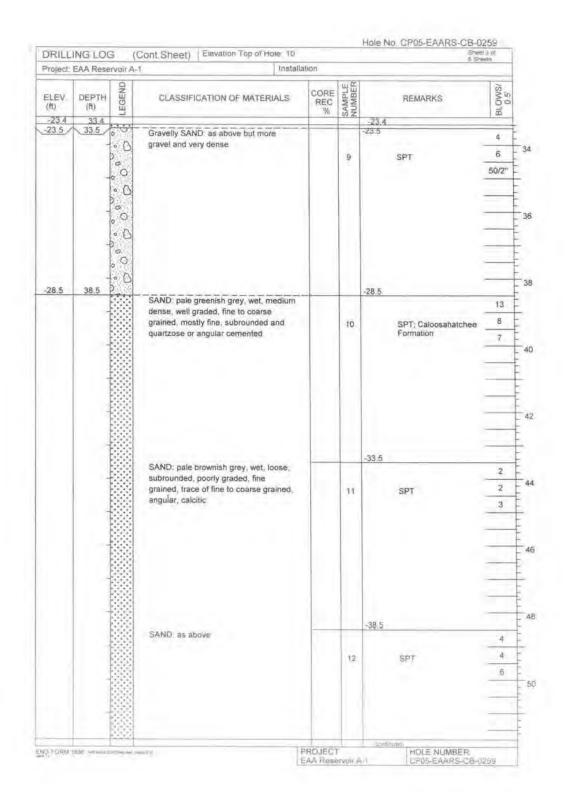


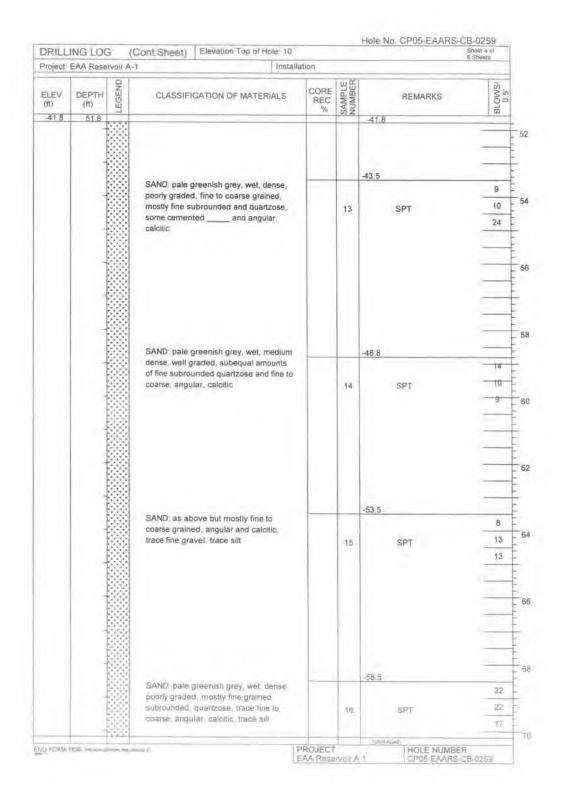


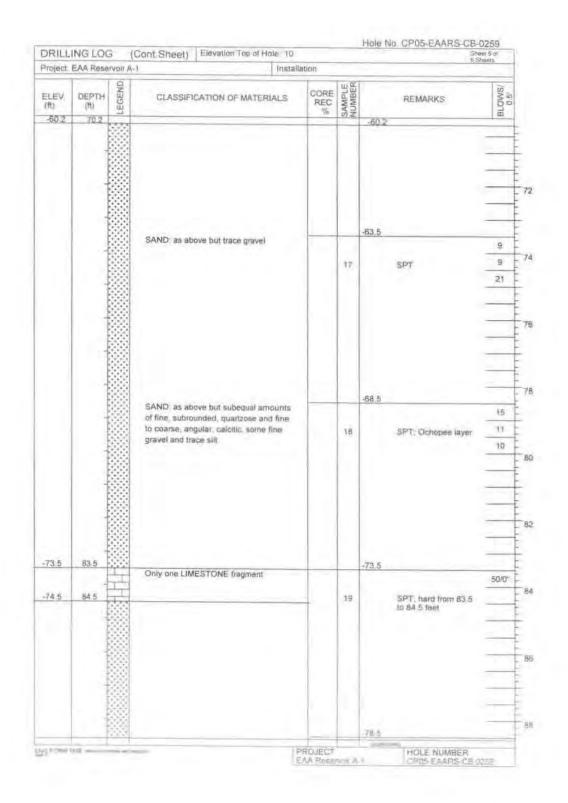


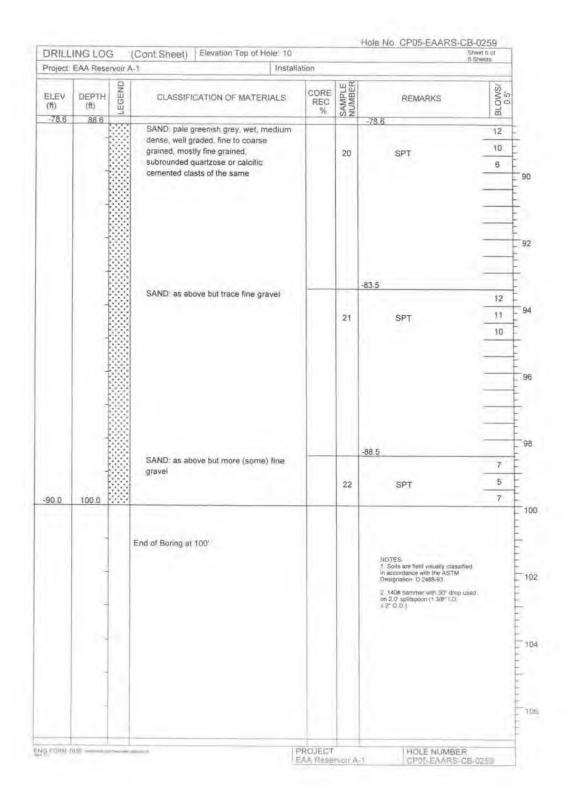




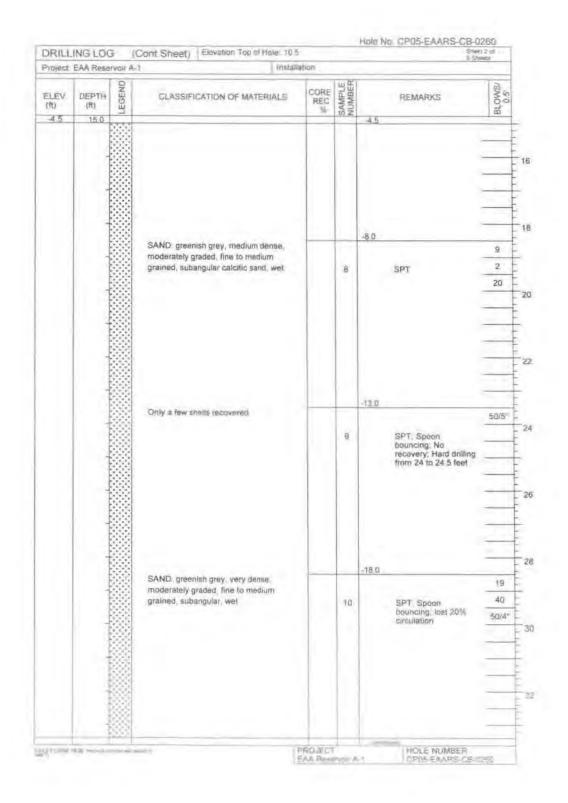


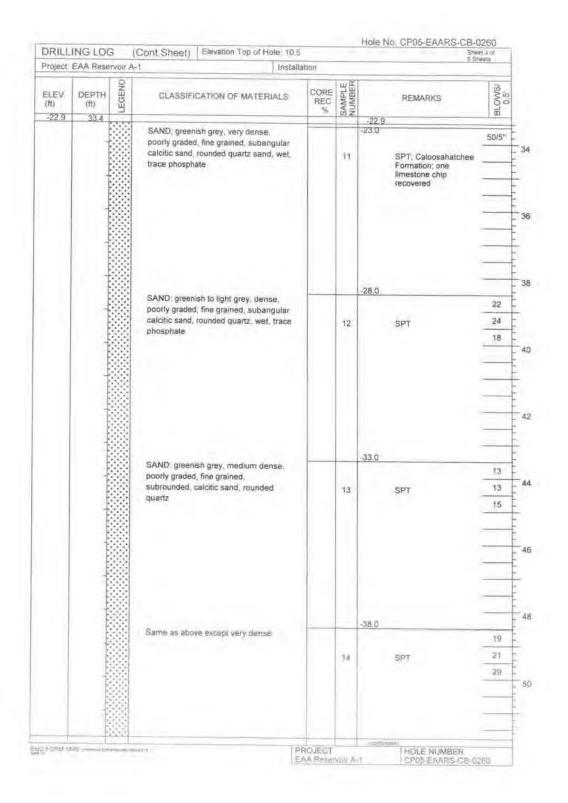


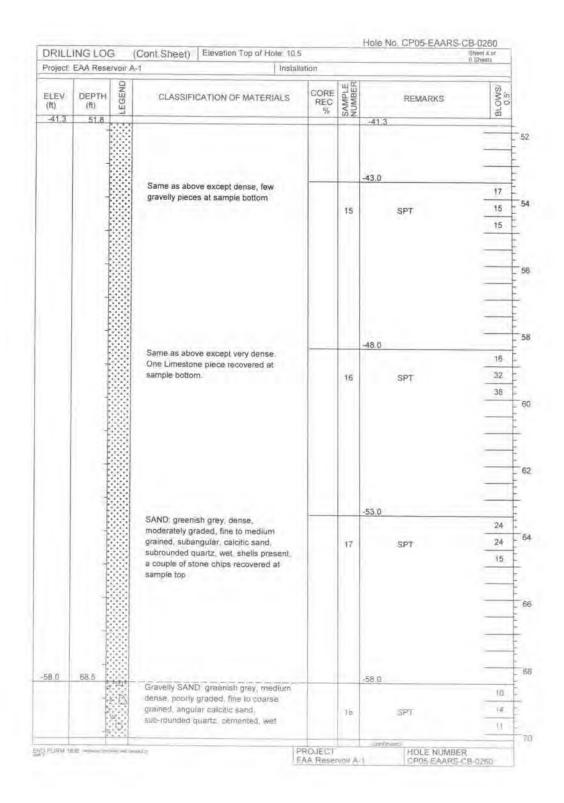


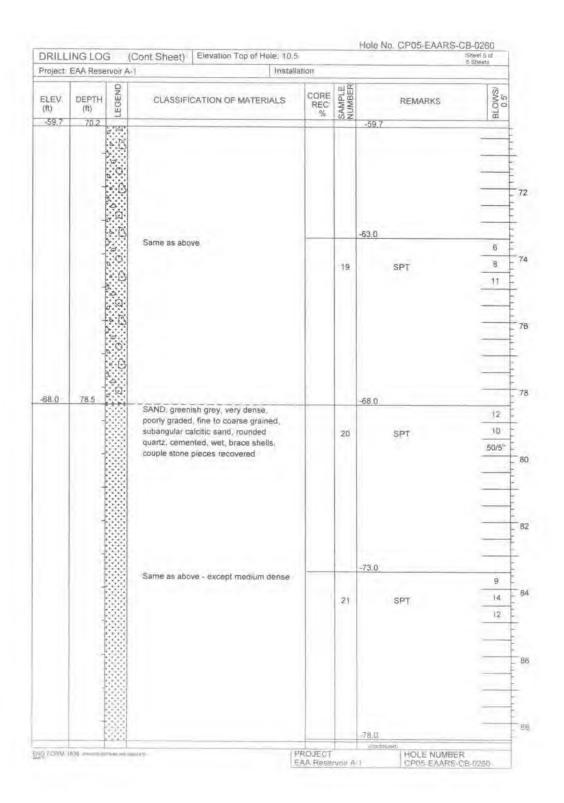


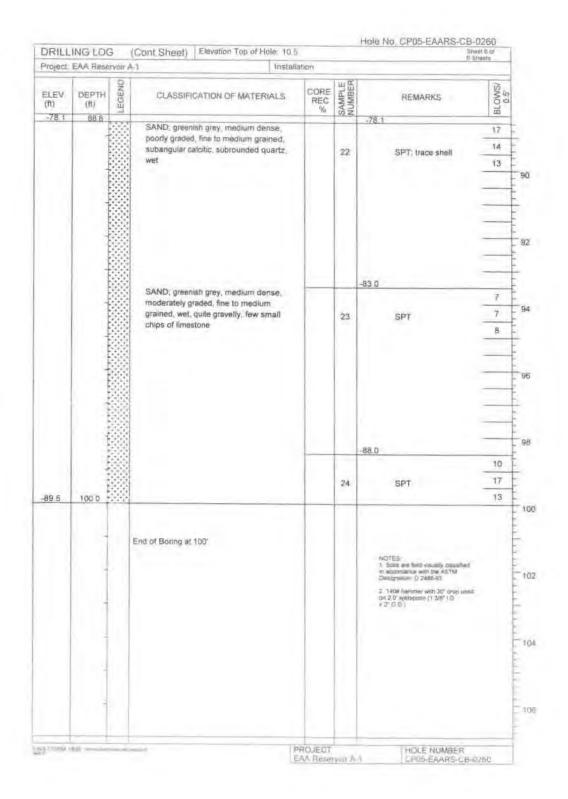
DRILL	NG LO	G 0	IVISION	Installatio	re=		1996 190	CP05-EAARS-CB-0	i i et	
	EAA Re		A-1	10 Size and type of bit 3" bit. Rotary Method						
Locati	on N7/3	583 6. E	775194 5 - NAD 1983	11 Datum for Elevation Snown, NAVD 1988						
			e & Associates, Inc.	12 Manufacturer's Designation for Drill Diedrich D-50						
	16 CP05			13. Total Number of Overburden Samples Taken: N/A						
-	of Orlier		Williams	14. Total Number of Core Boxes: 1						
	ion of Holestical		ed	-				nt measured	_	
				16 Date		Starte 28/200	5 8/8/20	mpleted 005		
	ness of Bu			17. Eleva	tion Top	of H	ole: 10.5	(ft)		
		-	N/A	18 Total	_	_		e N/A		
a Depth	of hole: 1			19. Inspe	ctor. A.		ronha		Te	
ELEV, (ft)	DEPTH (ft)	LEGEND	CLASSIFICATION OF MATERIA	ALS	CORE REC %	SAMPLE		REMARKS	BLOWS/ 0.5'	
10.5	0.0	31, 3	DEAT block toppe could needed	Ent		-	10.5			
		4 3/3	PEAT: black, loose, poorly graded, grained, dry, mostly organic	ine		1	10.5		111	
		144	S Class and Lineard Soldierine			1		SPT	3	
		9 25							3	
		43, 11	Same as above except medium de	enso.			9.0		3	
	-	la at la	Wet						_	
		040				2		SPT	6	
		4.25					7.5		7	
		84 0	Same as above with some stone of	hips					2	
		4 154			- 1/ 1	-	ep.		2	
	-	00 0			3	SPT		14		
	U 1850								-14	
	1	10 4	* Contract of				5.5			
		4.5	Same as above						4	
		26				4	SPT, Casing 0 to 5 feet due to rocks	SPT Casing 0 to 6	10	
		0.8						feet due to rocks	9	
		25						obstructing borehole advancement	-	
		10 1			_	-	3,5		arrive.	
		6.63							50/4"	
25	80	45 4				5	25	SPT		
			A few small pieces of limestone							
		1	recovered LIMESTONE traces of shells, void	de do	A D	4		UD covins		
	0		not appear very common	05 00	(RQD	1		HQ coring; UCS=9768ps; Core;	-	
					22%)			7 to 12 feet, 22" of lotal recovery one	_	
	1							piece each of 4 5".		
		T						4.75", 9" and 5.75"		
		T		- 1						
		1							_	
15	12.0	15550	CARD and how	- T			-1.5			
		1833	SAND light brown very dense in graded fine to coarse grained and comented west with traces of shell						29	
		88				(0)		SPT	50/6"	
		1000	fragments			100	10			
		188	SAND light grey, defiled, poorly gr	raded.		-	-3.0		9	
		E33	fine to medium graineri. Autourgoli						-	
		188				1	3PT pincer of		14	
		1888						Immutoria racovered	.19	





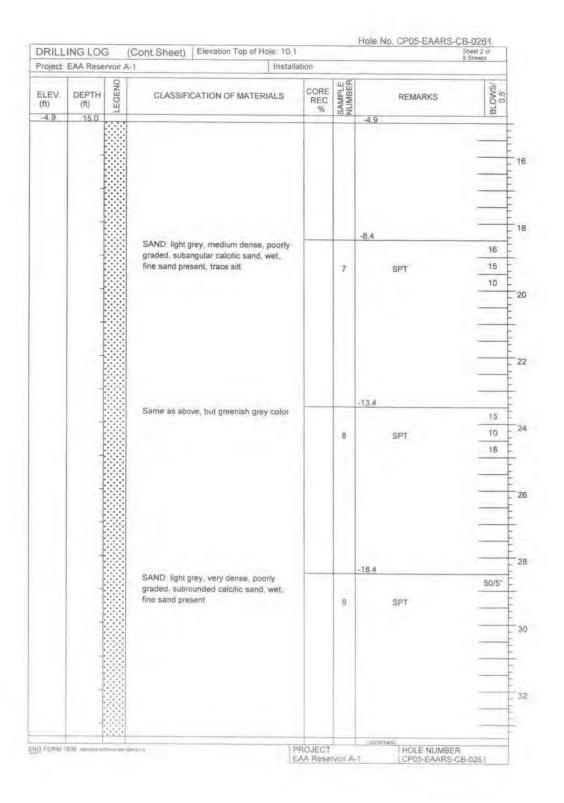


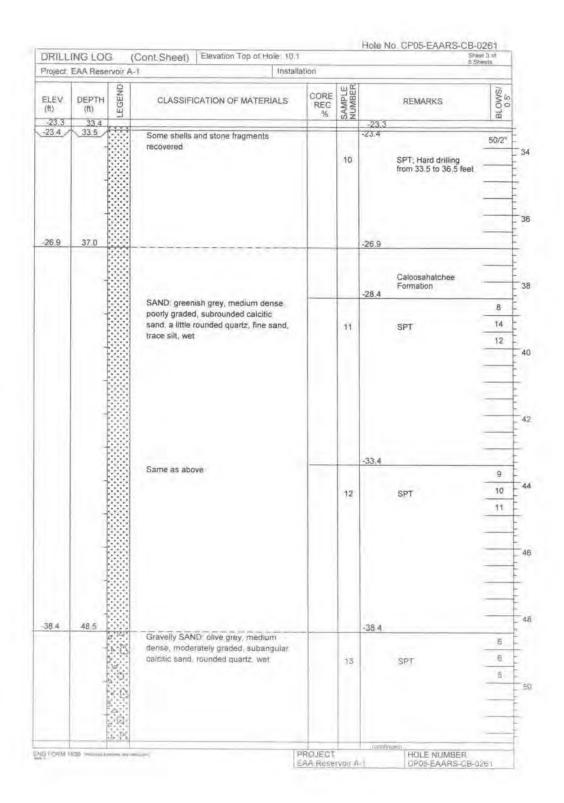


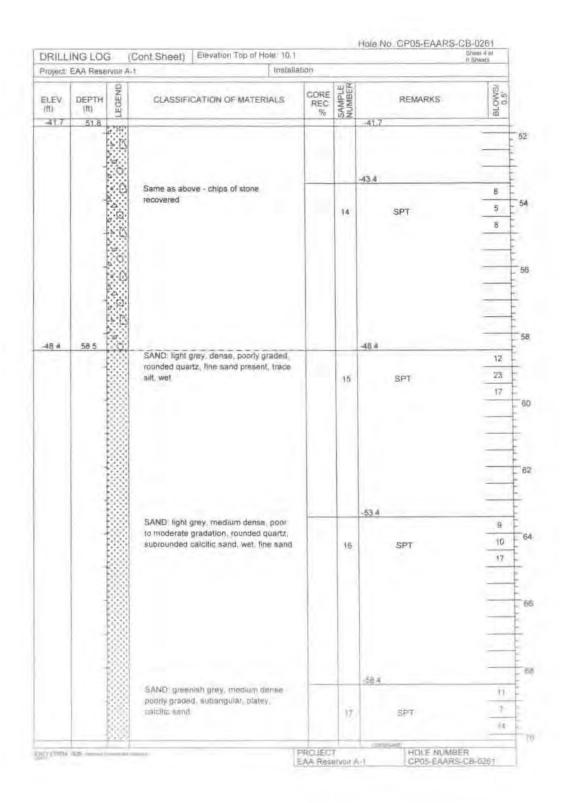


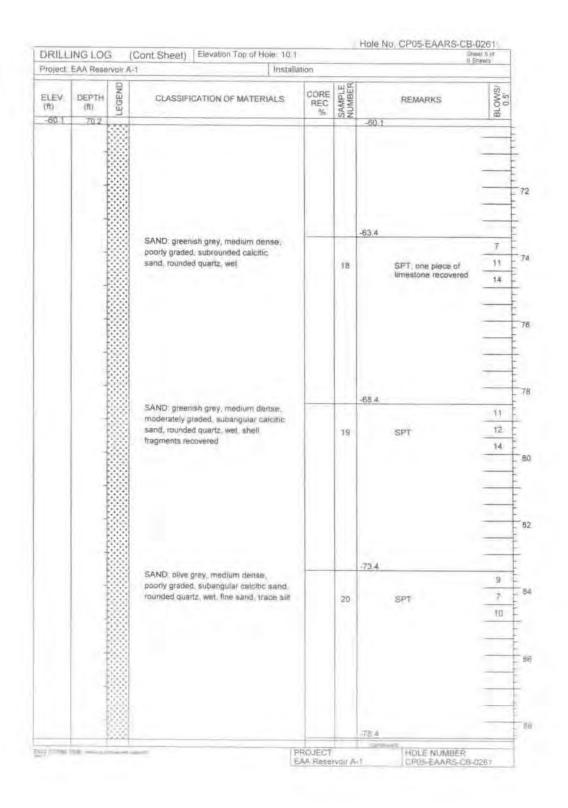
APPENDIX 2 SUPPLEMENTAL BORINGS AND PIEZOMETER INSTALLATION LOGS: 261-280

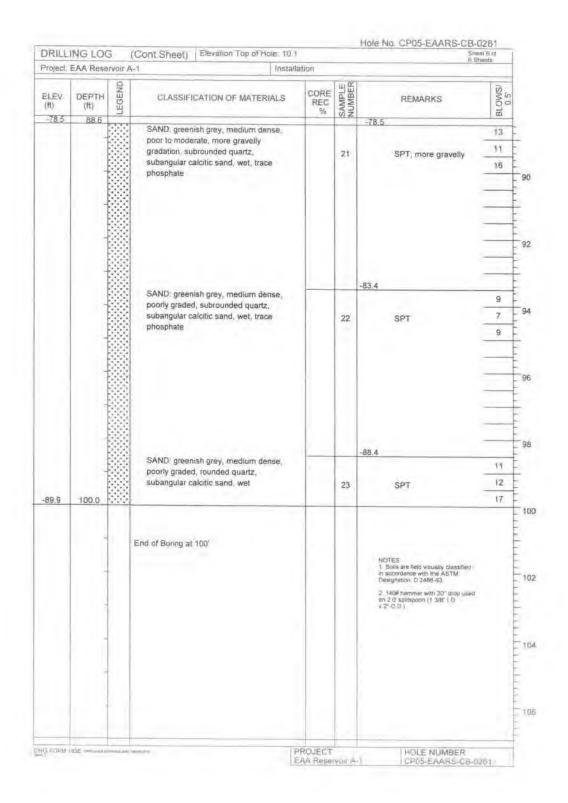
DRILL	ING LO	ivision	Hole No. CP05-EAARS-CB-0261 Installation: Provided To Shared						
Projec	t EAA Re	servoir /	4-1	10 Size and type of bill, 3° bil, Rotary Method					
-			11 Datum for Elevation Shown NAVD 1988						
		E. M. Caledid Asserted Table	12 Manufacturer's Designation for Drill: Diedrich D-50						
A Hole No. CP05-EAARS-CB-0261				13 Total Number of Overburden Samples Taken. N/A					
5. Name of Driller, Travis Williams				14. Total Nurr					
	ion of Hol ertical		16. Date Hole	Start	ted Co	ompleted			
7. Thicks	ness of Bu	rden N	/A	7/18/2005 7/18/2005 17. Elevation Top of Hole: 10.1 (ft)					
8 Thick	ness of ca	p rock t	NIA .	18. Total Core	_				
9. Depth	of hole: 1	00 ft		19 Inspector.					
ELEV (ft)	DEPTH (ft)	EGEND	CLASSIFICATION OF MATERIA	LS COR	MPLE		REMARKS	BLOWS/ 0.5*	
10.1	0.0	3		9/	032			100	
		(11)	ROADFILL Sandy GRAVEL dense	F		10.1		22	
			creamy gray, well graded, line and coarse, subangular to angular, dry		1		SPT	33	
2.0	300	0	seems, owninging to miguin, ory		1		21-1	12	
8.6	15	13 60 40	PEAT: black, very loose, fine graine	ud	+	8.6		-	
		6 10	clayey	-				1	
		44 0	Sample (ar # 3) Peat		2		SPT		
		2 23				7.1	7.1		
6.6	3.5	124 5						9	
0.0	204	g. 34.	3a) Gravelly SAND greenish grey,		1		a mer		
		1.13	dense, angular calcitic sand, wet		3		SPT	5.	
		TO T	Same as about - but may do		+	5.6			
		0.	Same as above - but very dense, limestone chips					12	
4.6	5.6		And the soules		4	4.6	SPT	50/4"	
			LIMESTONE light grey to white, ve						
		1	hard, shell fragments, quite pornus		1		HO coring. UCS=4340psi.		
		丰		(RC	(II)				
	1 1			14	6)		Care 5 5 to 10.5 feet	-	
		-				1			
		1			1				
		1				1			
	- 3	1							
		1							
-D.4	10.5		CAMP annuals		+	-0.4			
		3.3	SAND greenish grey, medium den moderately graded, angular calcitic					8	
		233	sand well trace gravel		5		SPT, 2 Irmestone	8	
		(888)					chips recovered	8	
	1							-	
		100							
		233							
		333				-2.4			
		100	SAND light grey medium dense.					12	
		633	moderately graded; angular salcitic sand, wel		-		SPT Hard drilling	В	
		2632	Seating man		-13		from 12 to 13.5 feet	-	
		40.50					TO ATTLE TO SERVICE THE PARTY OF THE PARTY.	- 80	



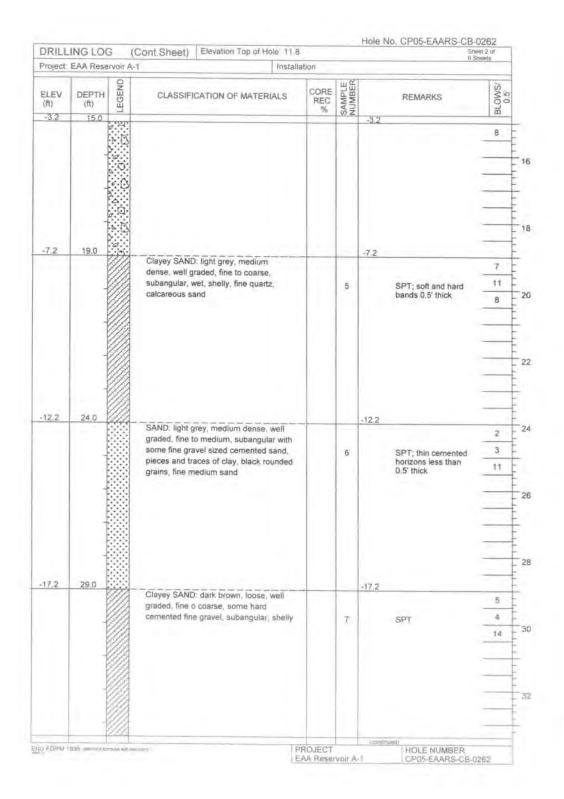


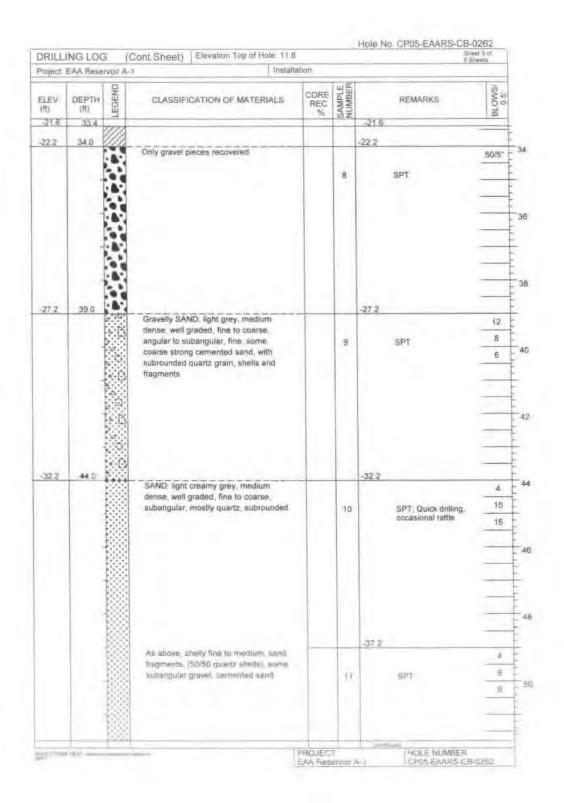


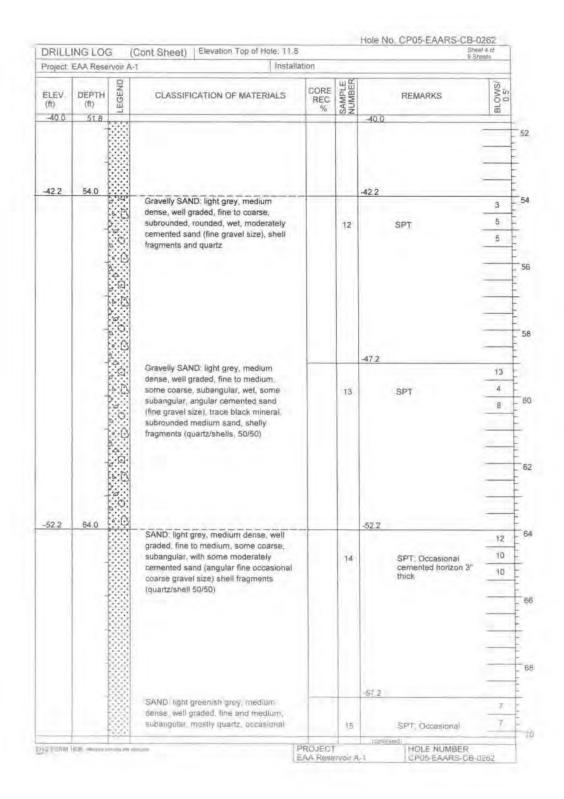


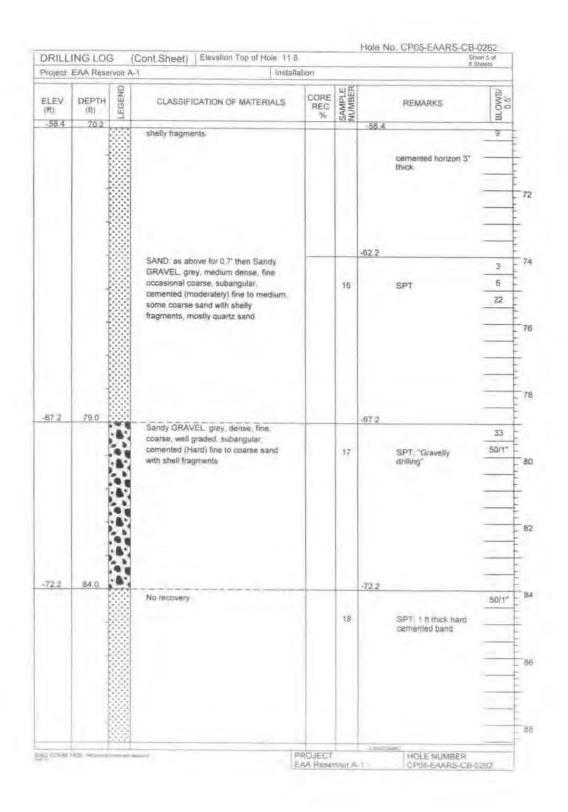


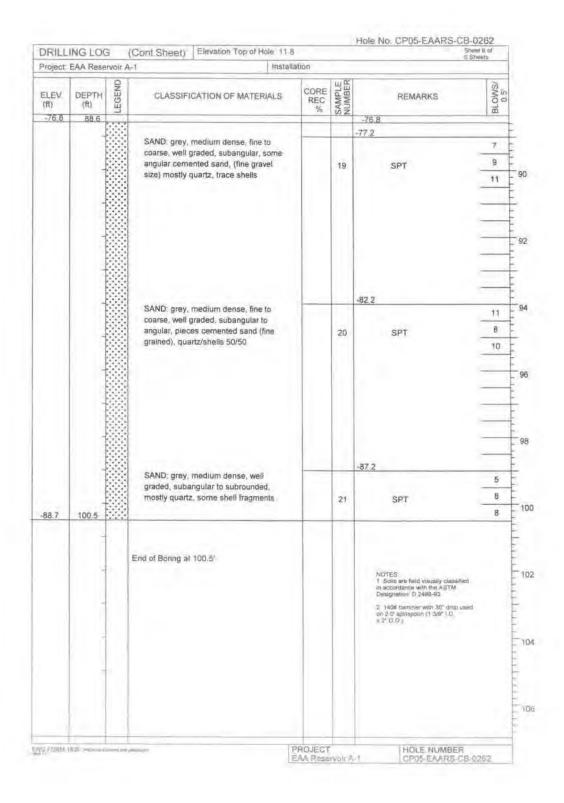
DRILLING LOG Division					Hole No. CP05-EAARS-CB-0262 Installation					
	t EAA Re	_	(-1)	10. Size and type of bit. 3" bit. Rotary Method						
	_		1692.6 - NAD 1983	11. Datum for Elevation Shown: NAVD 1988						
Drilling Agency, Nodarse & Associates, Inc.					nufacturer	's Des	ignation for Drill: CME-55			
4. Hole No. CP05-EAARS-CB-0262					al Numbe	r of Ov	verburden Samples Taken N/	A.		
5. Name	of Driller	Robert	DeAngelis	14 Tot	al Numbe	r of Co	ore Boxes: 1			
	tion of Hole					-	Vater Not measured			
₩ V	ertical [Incline	80	16. Dat		Starte				
7. Thick	ness of Bu	rden N	A	7/18/2005 7/20/2005 17. Elevation Top of Hole: 11.8 (ft)						
B. Thick	ness of ca	p rock: N	WA.	18. Total Core Recovery for hole: N/A						
9 Depth	of hole: 1	00.5 ft		19. lns	pector. J.					
ELEV.	DEPTH (ft)	LEGEND	CLASSIFICATION OF MATER	RIALS	CORE REC %	SAMPLE	REMARKS	BLOWS/		
11,8	0.0		COAUPI BALL	Norma						
			GRAVEL, light creamy grey, med dense, fine, some coarse, angula				11.8	15		
		- 1	subarigular, dry, silty and sandy	10		1	SPT Road hase	13		
	1					1	41.14 (1945 1144	4		
	100	- 1						-		
9.8	2.0	20 10	01 - 1 F1	Dr. Car			9.8			
		4 11/2	2' to 4.5' under our weight - no re Peat on side of SPT	covery				0		
		80.0				2	SPT			
		6 33				-				
		144 3								
		6.36								
		27 =								
							6.8			
	1	6 21 6 21 5 4	3" soft brown PEAT, then organi	Ċ			0.0	0		
			CLAY, very soft, brown black, v	et						
						3	SPT	0		
5.3	6.5	4.54					5.3	3		
		1	Light grey, thinly bedded, moder	rately						
		-	strong to strong, fine grained LIMESTONE with rodent/worm b	olae	100		100			
		1	Light grey and creamy brown, m	molted, (RQI	(RQD	1	HQ coring, UCS=3690psi	_		
	3	T	medium bedded, moderately stre		30%1			_		
			strong, fine grained, shelly LIME		DNE					
			becoming porous for bottom 0.3							
		T						-		
		T				1		_		
		T								
~~	1						0.3			
0.3	11.5	0.34	Gravelly SAND light gray, medi	um	1		8.3			
		10.75	dense, well graded, fine to coars			1				
		2.33	angular to subangular, wet, gray	rei						
		100	flakes (fine size) limestone she	11						
			fragments							
		100						_		
		500				1	42			
		88						10		
		HEE.				A	SPT	18		
	_	40.00			-	-	(aminos)			



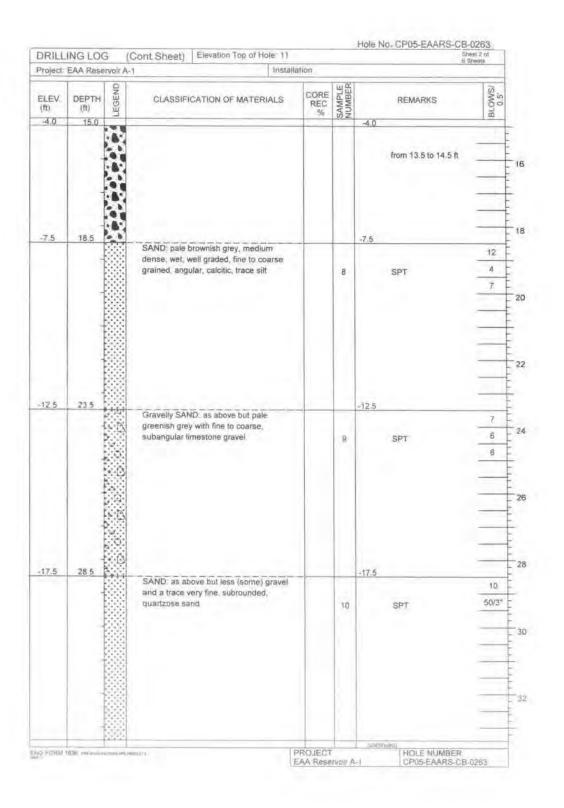


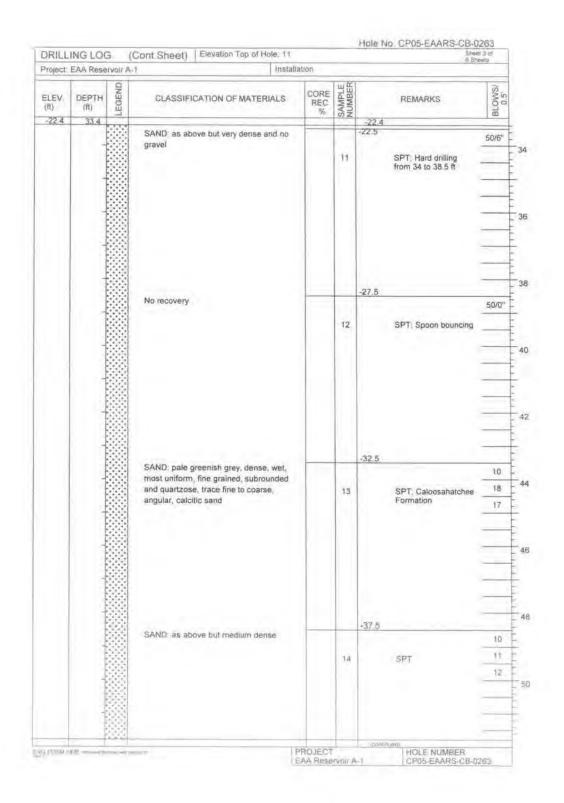


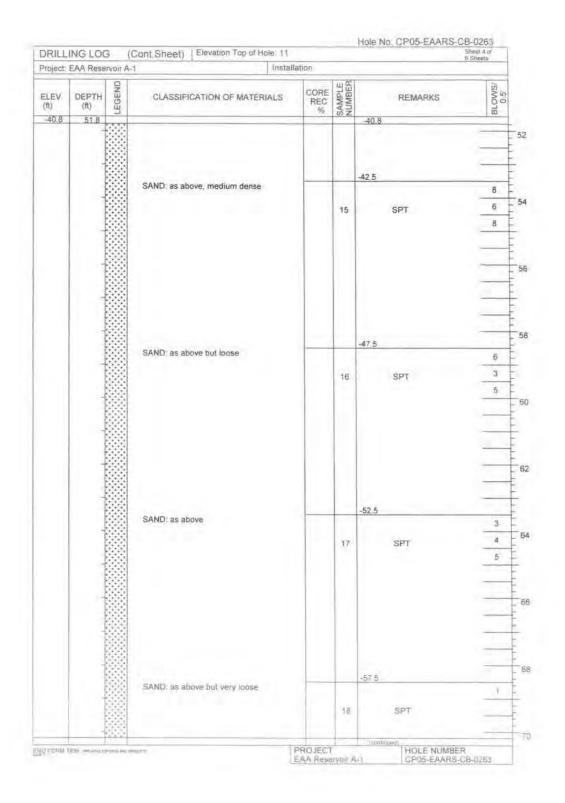


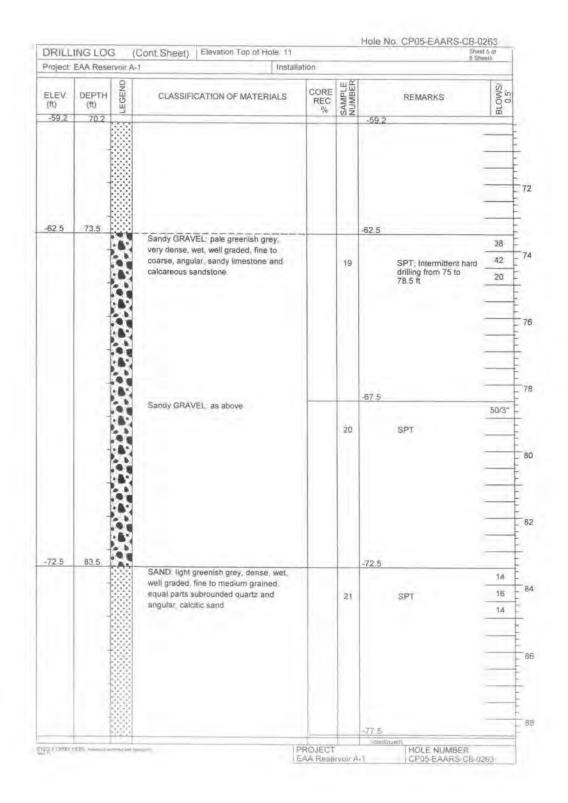


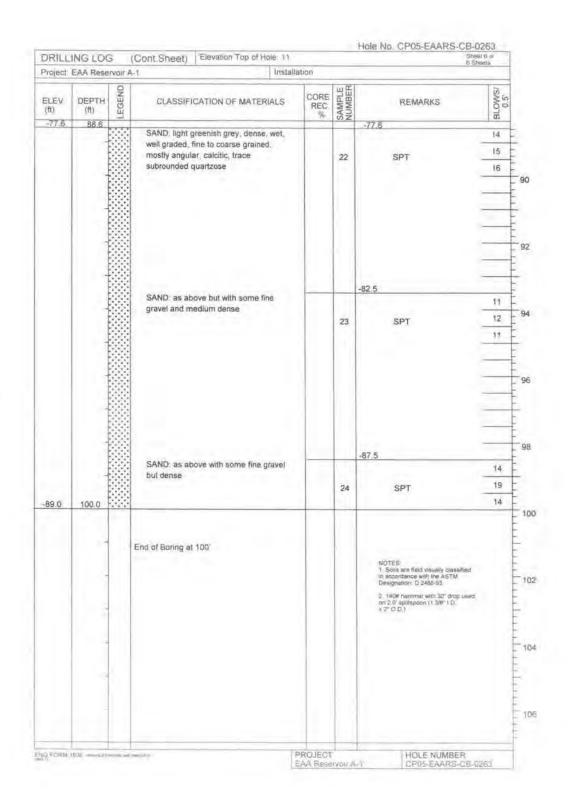
DRILLING LOG Division:					Hole No. CP05-EAARS-CB-0263 Shell 1 of Sheets 4 (Figure 1)						
Projec	EAA RE	servoir	A-1	10. Size and type of bit, 3" bit, Rotary Method							
2. Location: N759777, E784912.5 - NAD 1983							r Shown: NAVD 1988				
Drilling Agency: Nodarse & Associates, Inc.							signation for Drill Diedrich D-50				
4 Hole No. CP05-EAARS-CB-0263							verburden Samples Taken: N/A				
	tion of Hol	_	uemke			_	ore Boxes. 1.				
	ertical		ned				Vater. Not measured d Completed				
2.00	ness of Bi			16. Date Hole Started Completed 7/29/2005 8/2/2005							
	ness of ca		· ·	_			ole: 11 (ft)				
	of hole	-		18. Total Core Recovery for hole: N/A 19. Inspector: N. Holst							
	10000	-		19. Inspe	ector: N.	Hoist					
ELEV. (ft)	DEPTH (ft)	LEGEND	CLASSIFICATION OF MATERIA	ALS	CORE REC %	SAMPLE	REMARKS	BLOWS/ 0.5			
11.0	0.0	36.3	DEAT LINE do not be a long of	N. C		0,2	11.2	1			
		0 00	PEAT: black, dry, medium dense, f grained, organic, with some fine to	ille		111	11.0	5			
		340	coarse, angular limestone gravel. I	Moist.		4	SPT; Fill	5			
		6 16	at 3.6 ft				9,5	6			
		14. 11					3,4	10			
	1 5	4 110				1.0		-			
		24 3				2	SPT	7			
		12 11/2					8.0	4			
		11.12						4			
		2 34				3	SPT	2			
	4.5	10 34				-		3			
6.5	4.5	11	Silty SAND: light brownish grey, we	et.			6.5				
	1		medium dense, well graded, fine to					16			
			coarse, angular, calcitic, slightly pla			4	SPT	10			
		with some angular, fine to coars					5.0	14			
4.6	6.4							50/5"			
		LIMESTONE caproc				5	SPT; Limestone				
							caprock from 6.4 to				
		1					3.5 11 ft	en int			
								50/2"			
		1				6	2.5 SPT; Only limestone				
		1				111	chips at 7.5 ft; Hard drilling from 7.5 to 8.5				
					20	1	ft				
					(RQD	,	HQ coring				
		1			0%)						
		1									
0.0	11.0										
-			Sandy GRAVEL light brownish gre very dense, wet well graded fine to								
			coarse angular, limestone	u.							
	-	1									
	19										
		100					-2.5				
		9.8						30			
		1				195	POT CONTROL	50/3"			
		. 0 #				7	SPT: Quit at 13.5 ft. on 7/29/05; Resumed				
		. B.					6/2/05: Hard drilling	_			
FORM 1	ER - WIL	- COC 100		IPR	CUECT		HOLE NUMBER				



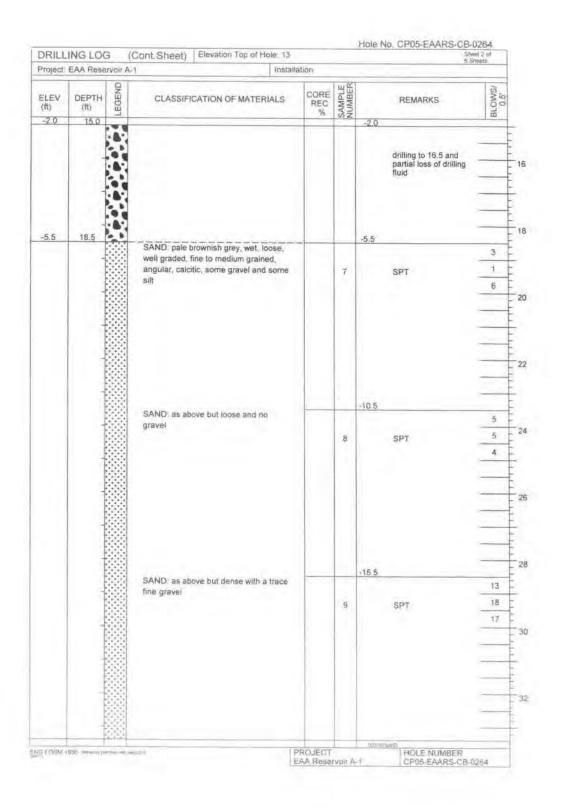


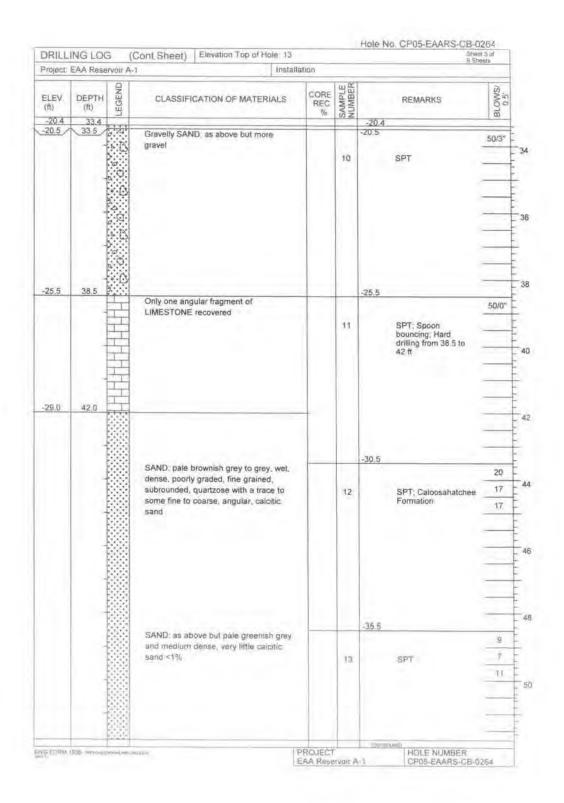


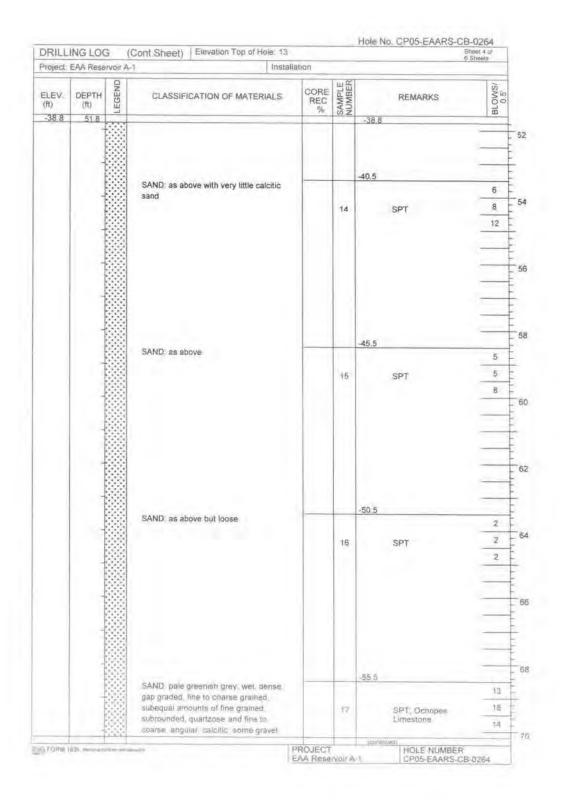


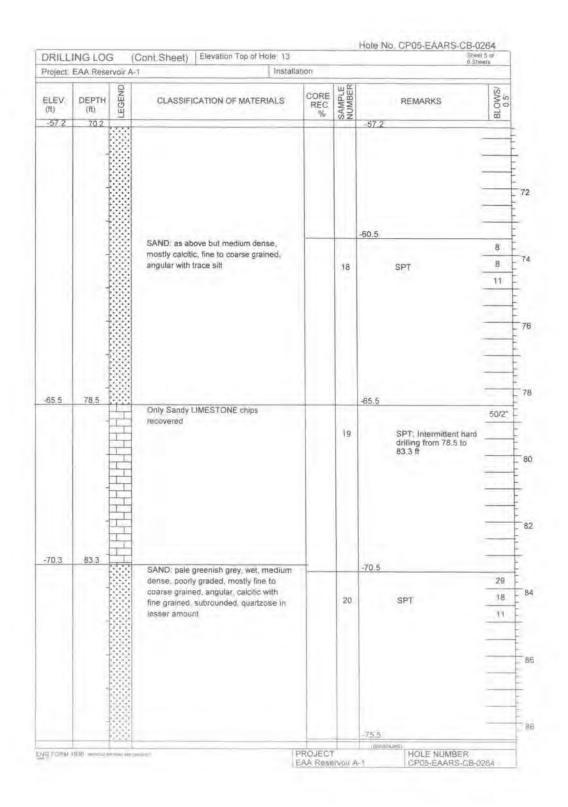


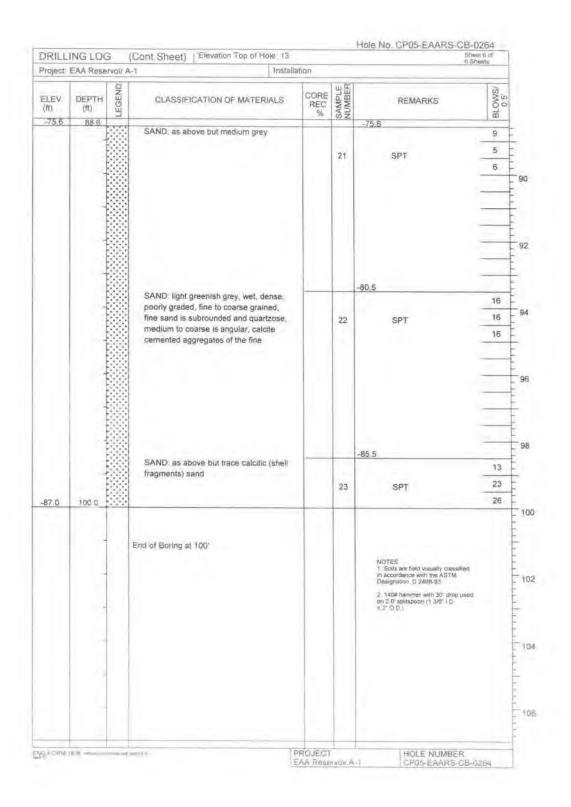
DRILLING LOG Division				Hole No CP05-EAARS-CB-0284						
Project	EAA Re	SERVOR A	4-1	10. Size and type of bit: 3" bit, Rotary Method						
2 Location: N756489.3, E786355.6 - NAD 1983				11. Datum for Elevation Shown: NAVD 1988						
3 Drilling Agency: Nodarse & Associates, Inc.						_	ignation for Drill: Diedri			
4 Hole No. CP05-EAARS-CB-0264				-			erburden Samples Tak	en. N/A		
	of Driller		emke	-		_	re Boxes, 1			
⊠ Ve	ion of Hol ertical	Inclin		15. Elev	: Hole	Starte	Vater Not measured d Completed 5 7/19/2005			
	ness of Bi			17 Elev			ole: 13 (ft)			
	ness of ca		N/A	-		_	y for hole. N/A.			
9. Depth	of hole: 1			19 Insp	ector N					
ELEV (ft)	DEPTH (ft)	EGEND	CLASSIFICATION OF MATER	RIALS	CORE REC %	SAMPLE	REMARKS	BLOWS/		
13.0	0.0	* ****				0)2		12		
		d. 51	Gravelly SAND yellowish grey to brown, dry, very dense, well grade				13.0	39		
			to coarse grained, angular, calcib			,	SPT	40		
		10	roadfill			1		33		
		101					11.5			
		* D						18		
		a.				2	SPT	18		
		J.CI				-		23		
95	35	4:03					2.5			
9.5	23	25 0	PEAT at 3.5 ft to 5.7 ft				9.5	1		
		2 14								
		20.2				3	SPT	2		
		0.35					8.0	2		
		2.0						2		
7.3	57				-	10	444	7		
-50			Gravelly CLAY: dark gray, plastic	, 5011		A	SPT	50/5"		
6.5	6.5	. 1	LIMESTONE pale yellow grey, gr	mir mad	-		5,5	20/2		
- 1		Jane San	light brown, fine grained, thinly be		4	1.5	HQ coring; UCS=1530psi	_		
		7	fossiliferous, vuggy, moderately strong		64	1				
		T	and hard to soft and weak		(RQD 40%)			Si .		
		扭			10.07					
								-		
			T							
3.0	10.0									
	1									
	1									
			GRAVEL reals because how are		-	-	15			
		9.0	GRAVEL pale brownish grey wet, loose, angular, calcitic, trace sand					2		
		tivos, angular, calcino, trace sand			.9	SPT	2			
	. 0	. 0 6						5		
							1.4			
		. 0 0	GRAVEL as above fur very ten	SN.		-	-0.5			
		(SPONVEL) as above for very fair						50/4"		
		. 9 8				5	SPT Haro o	niong		
		-					from 13 9 to	20.0		
		B.					then interme			



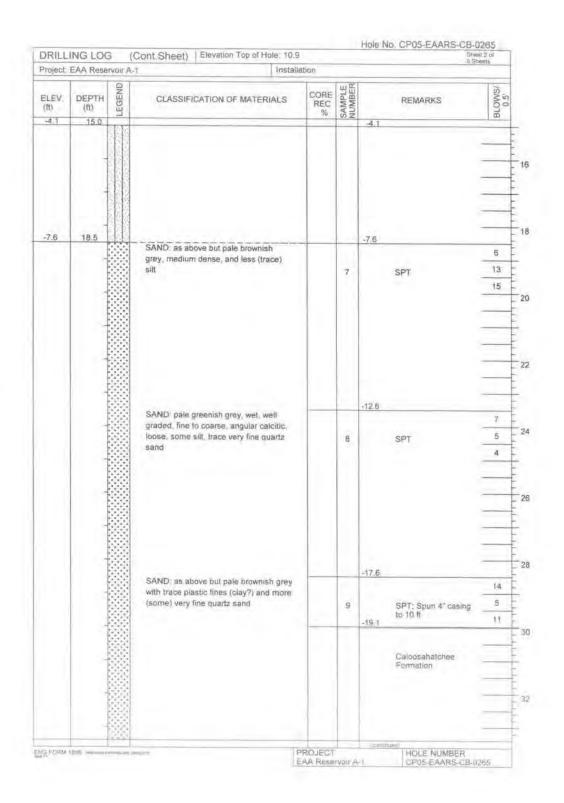


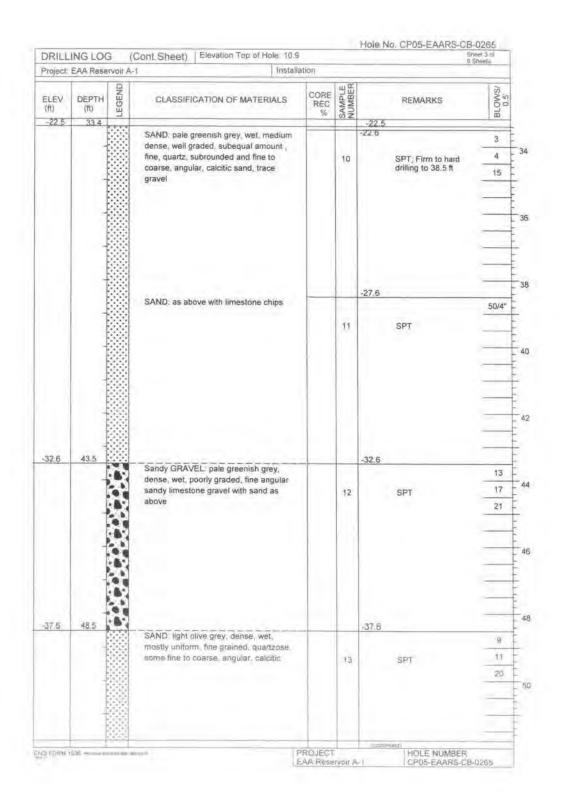


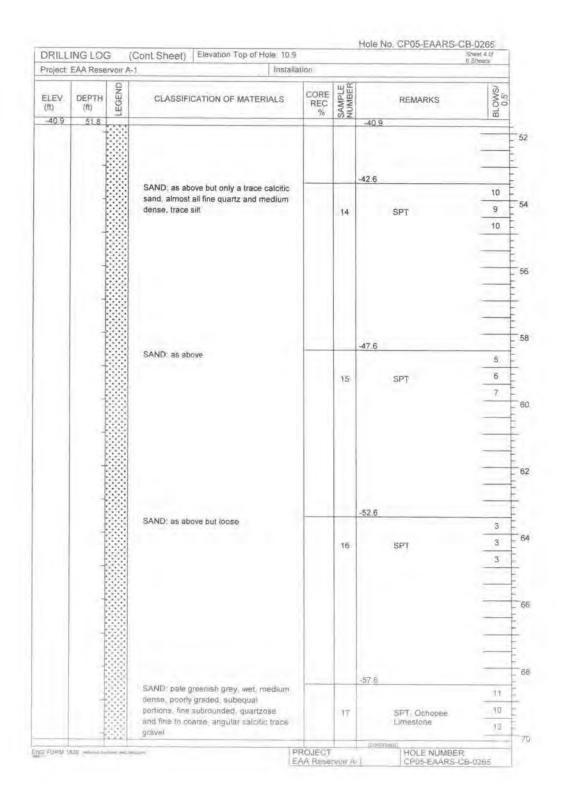


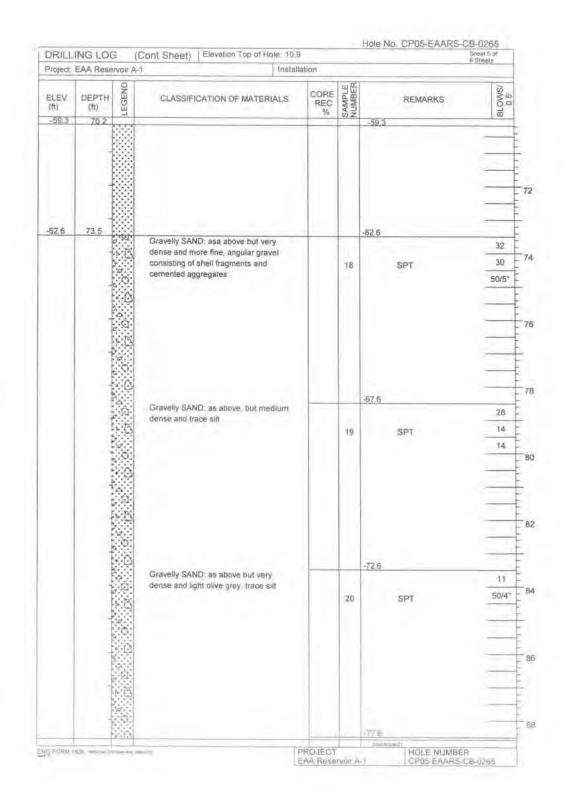


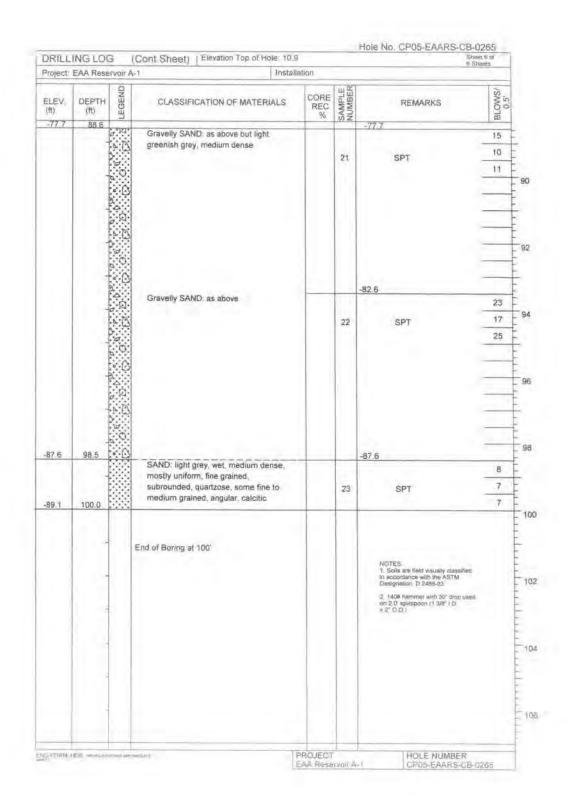
DRILL	ING LO	3 D	noision	Installati	Dn		Sher	0265 u 1 til		
Projec	t EAA Re	servoir A	iv)	10 Size	and type	of bit	3" bit, Rotary Method			
			789525.6 - NAD 1983	11 Datu	m for Ele	evation	Shown: NAVD 1988			
3. Drillin	g Agency.	Nodarse	& Associates, Inc.	12. Manufacturer's Designation for Drilt Diedrich D-50						
# Hole	No CP05-	EAARS-	CB-0265	13. Total Number of Overburden Samples Taken, N/A						
_	of Driller		emke	14 Total Number of Core Boxes: 1						
	tion of Hole ertical		no.	15. Elevation Ground Water. Not measured						
				16 Date		Starte 8/200	d Completed 5 7/28/2005			
	ness of Bu			17 Elevi			ole: 10.9 (ft)			
	ness of ca		WA .	18 Total Core Recovery for hole: N/A						
Depth	of hole: 1	29.65		19. Inspe	ector N					
ELEV. (性)	DEPTH (ft)	EGEND	CLASSIFICATION OF MATERI	IALS	CORE REC %	SAMPLE	REMARKS	BLOWS/ 0.5'		
10.9	0.0	171			744	ωz		1 th		
		to the	PEAT: black, fine grained, moist, le	opsa			10.9	4		
		05-0	to moderately dense, organic			1	SPT	4		
	1	4 449				1		9 -		
		24.2					9.4			
		404						5 -		
		06.0				2	SPT	5		
		6 04					7.9	3		
		11. 0	Wet at 3.0 ft, limestone chips					0 -		
		400				0		-		
		11/4 1				3	SPT	0 -		
6.5	4.4	to all	LIBADOTONIO C	the feet by			6.5	18		
			LIMESTONE (an, fine to coarse, to bedded, fossiliferous, vuggy, strong			1		50/0"		
		T	hard to soft, weak, porous, friable		-	X	57 SPT			
		1				X	HQ coring, GW, Well			
	1		Limestone above is probably a bo	uldare	. 1		graded gravel with sand, Hard drilling at	-		
			in Fill	unuer			4.4 ft; Spoon bouncing	_		
3.9	7.0						Limestone is probably			
		500 71	PEAT; as above and limestone	ips			a boulder in FIII	-		
		0.00								
		107 14						-		
		9 19								
		14 14						-		
		104 0					1.4			
		4 24						50/5"		
		24.0								
		4 24				5	SPT	-		
	1	1000								
		2 44						t		
		ES 2				1		- 1		
		5 63								
		25 0								
21	13.0	4.34	03-0310		1					
		16	Sity SAND pale greenish grey wi graded fine to course grained an				28			
		111	diagram in the second diagram of the second					3		
			WEL .		1	4	Bathelli Announce and an	5		
		11/1/				12	SPT. Hard drilling from 12 s to 13 ft			
								3		



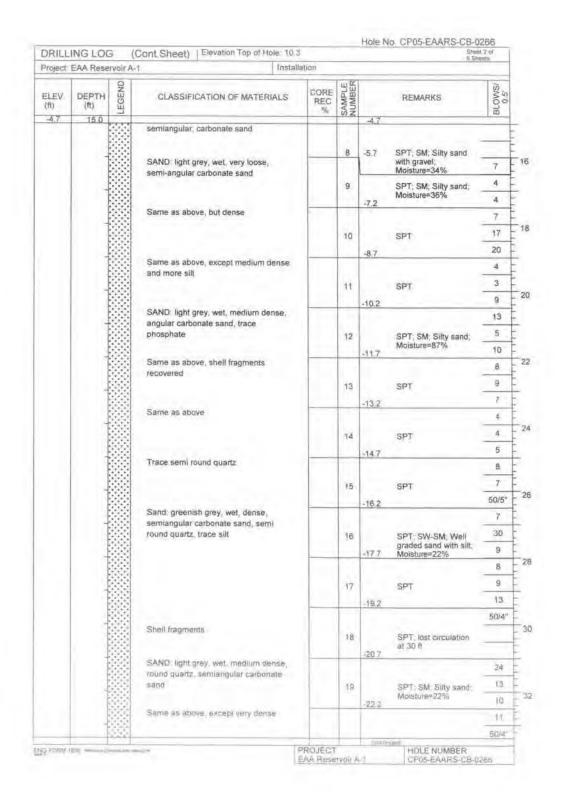




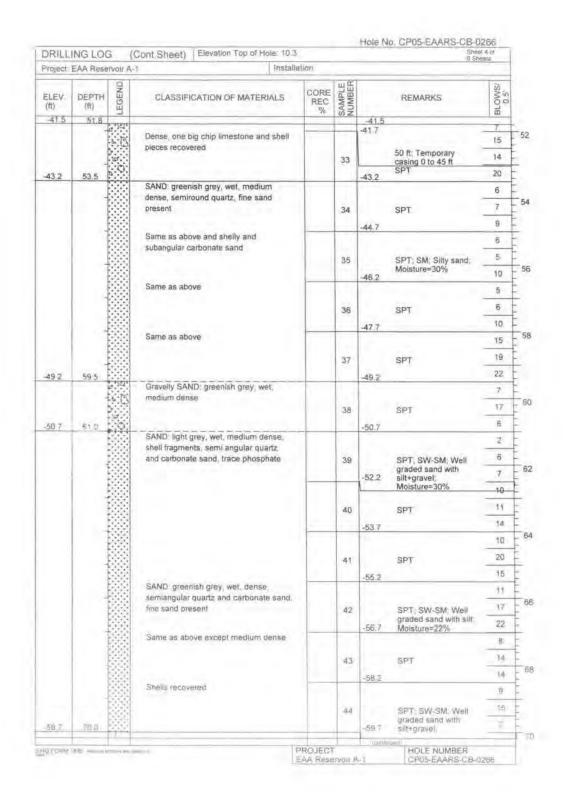




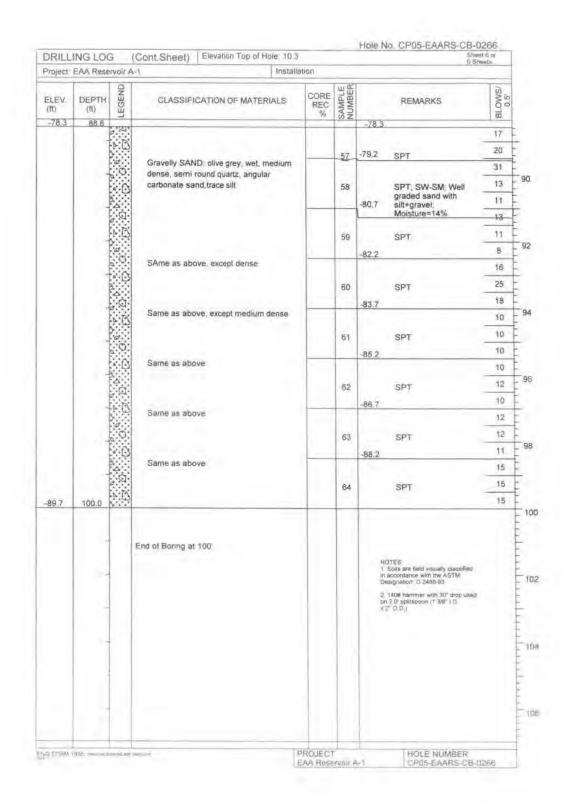
ency P05 hiller of Bi	Nodars EAARS Travis	#1226.8 - NAD 1983 1 e & Associates, Inc -CB-0286 1 Williams 1 ed 1 A	17 Datum 12 Manul 13 Total 14 Total 15 Eleval 16 Date 1 17 Eleval 18 Total 19 Insper	n for Ele facturer Number Number Hole 7/6 Top Top Core Ri	evation 's Des r of Co ound V Starte s/2005 o of Ho ecover	Shown sgration refourde are Boxe Vater. No. d. Co. 7/6/20 ole: 10.3 ry for ho	ot measured impleted IQ5 (ft)	BLOWS/ 0.5:																
ency PDS hriller of Bi of Bi of ca of Bi PTH	Nodars EAARS Travis I e Inclin irden N ip rock I	e & Associates, Inc. CB-0266 Nilliams ed IA CLASSIFICATION OF MATERIA SAND: light brownish grey, wet, top- ft medium dense, semi angular calci- sand, shell fragments, trace gravel a silt CORE only a few chips of limestone	12 Manual 13 Total 14 Total 15 Eleval 16 Date I 17 Eleval 18 Total 19 Insper	Number Number Number Number Number Hole 7/6 tion Top Core Ric ctor: A.	's Des r of Co ound V Starte 5/2005 o of Ho ecover	agriation verourde ire Boxe Vater. N. d. Co. 7/6/20 ole: 10.3 ry for ho ronha	for Drill Diedrich D-50 in Samples Taken N/A is: 1 of measured impleted i05 (ft)	18																
of Bi of cole.	EAARS Travis I e Inclin urden N ip rock: I	CB-0286 Nilliams ed IA N/A CLASSIFICATION OF MATERIA SAND: light brownish grey, wet, top fit medium dense, serni angular calcus and, shell fragments, trace gravel a sitt CORE, only a few chips of limestone	13 Total 14 Total 15 Eleval 16 Date 17 Eleval 18 Total 19 Insper	Number Number Hole 7/6 tion Top Care Rictor: A.	of Cound V Starte 5/2005 of He ecover	verourde are Boxe Vater. Ni d. Co. 7/6/20 ole: 10.3 ry for ho ronha	on Samples Taken: N/A es. 1 of measured impleted O5. (ft) let: N/A	18																
of Bi of ca ole: 1	Travis I e Inclin irden N ip rock: I	CLASSIFICATION OF MATERIAL SAND: light brownish grey, wet, top fit medium dense, serni angular calcu- sand, shell fragments, trace gravel a sitt CORE only a few chips of limestone	14 Total 15 Eleval 16 Date 17 Eleval 18 Total 19 Inspec	Number fion Gro Hale 7/6 tion Top Care Ri ctor. A.	starte s/2005 of He ecover	Vater. No d Co 7/6/20 Jole: 10.3 Ty for ho ronha	ot measured impleted io5 i (ft) let N/A	18																
of Bi of ca ole: 1	inclin liden N prock I	CLASSIFICATION OF MATERIAL SAND: light brownish grey, wet, top fit medium dense, serni angular calci sand, shell fragments, trace gravel a sitt CORE only a few chips of limestone	15 Eleval 16 Date I 17 Eleval 18 Total 19 Insper 1.LS	tion Gro Hale 7/6 tion Top Care Ri ctor. A.	Starte 5/2005 of Ho ecover	Vater. No d Co 7/6/20 ble: 10.3 ry for ho ronha	of measured impleted io5. (ft) let N/A	18																
of Bi of ca ole: 1 PTH ft)	Inclin irden N ip rock: I	CLASSIFICATION OF MATERIAL SAND: light brownish grey, wet, top fit medium dense, serni angular calcu- sand, shell fragments, trace gravel a sitt CORE only a few chips of limestone	16. Date I 17. Elevar 18. Total 19. Insper LS	Hale 7/6 tion Top Care Ri ctor: A.I CORE REC	Starte 5/2005 of Ho ecover	d Co 7/6/20 ole: 10.3 ry for ho ronha	mpleted 05. (ft)	18																
of ca ole: 1 PTH (t)	p rock: I	CLASSIFICATION OF MATERIAL SAND: light brownish grey, wet, top fit medium dense, serni angular calci sand, shell fragments, trace gravel a sitt CORE only a few chips of limestone	18. Total 19. Insper LS	Core Roctor A.	o of Ho ecover M. No:	ole: 10.3 ry for ho ronha	(ft) le: N/A	18																
PTH (t)	00 ft	CLASSIFICATION OF MATERIAL SAND: light brownish grey, wet, top fit medium dense, serni angular calci sand, shell fragments, trace gravel a silt CORE only a few chips of limestone	19. Insper	ctor A.	M. No	onha		18																
PTH (t)		CLASSIFICATION OF MATERIAL SAND: light brownish grey, wet, top ft medium dense, serni angular calci sand, shell fragments, trace gravel a silt CORE only a few chips of limestone	0.5	CORE			REMARKS	18																
ft) 0.0	LEGEND	SAND: light brownish grey, wet, top ft medium dense, serni angular calci sand, shell fragments, trace gravel a silt CORE, only a few chips of limestone	0.5	REC	SAMPLE	10.3	REMARKS	18																
		ft medium dense, semi angular calci sand, shell fragments, trace gravel a silt CORE only a few chips of limestone	itic		0,2	10.3		-																
0		ft medium dense, semi angular calci sand, shell fragments, trace gravel a silt CORE only a few chips of limestone	itic			10.3																		
0	88 11 11	sand, shell fragments, trace gravel a silt CORE only a few chips of imestons) 1			15																
		Sitt CORE, only a few chips of limestone	1		4	93	SPT	50/5"																
						9.3	SAC 1	-																
		recovered, pale white, shall franmen																						
	\pm		nta in	16	1	HQ coring, Core		- 1																
	the state of the s	chips		(RQD)			to 4.0 ft																	
	1			200																				
																								-
		I						-																
0.1	E 7 (5/8)					6.3																		
	0.34																							
	4.(2)																							
	10.																							
	0.0	Court CANC and and and				4.8																		
. 1		Gravelly SAND grey, wet, very dem angular, carbonate sand	Se					7																
	Contract of the contract of th	angular, carbonate same			2		SPT: CO3=76.6%, SW-SM, Well graded	32																
	Q.				-			32																
	1.0	Same as above, except medium der	nse			3.5	sand with sitt+grave). Moisture=13%	-																
	2.3	-22-200000000000000000000000000000000					SPT	-																
	o.O.				3			8																
						1.8		6																
		.0	Gravelly SAND dark greenish grey	wet					50/4"															
	4				1		Geo.																	
		σ				4		SPT	-															
		- 1				-			10.					0.3										
	15.	Gravelly SAND angular carbonates	sand					22																
	0.0				4		SETT spent beinging	50/1"																
	5.0				8	1 3 2	Quite hard drilling	-																
15	7	SAND fines registers trace and trace	W		-	-12	from 9 0 to 15 5 ft																	
300	1832		7					30																
	833				8		SPT SM. Smy sand.	50/4																
	1888				10	29.4	Moistur=24%																	
133	133	Same as above whelly fragments				125		39																
18		133	138	recovered						-														
	888				7		SPT	11																
	333					42		40																
	333	SAND amenian grey wet very mer	ne					53)51																
1.	15		SAND finer particles trace sitt varioense Same as above, shelly fragments recovered	SAND american arey liver, very mene	SAND times particular traces six view dense Same as above whelly fragments recovered SAND premium grey livet very mine	SAND finer carbonate and FROMETT	Gravelly SAND angular corbonate aand 5 SAND finer corbonate aand 5 SAND finer corbonate aand 5 -12 Same as above, anally fragments recovered 7 SAND american gray wet, vary divice	Gravelly SAND angular corbonate sand SPT: spoon bouncing. Queen hard drilling 12 from 9 0 to 15 5 ft SAND finer carboses trace siit. vary dense SAND sand shelly fragments 1800vared SAND american grey livet, vary diline																



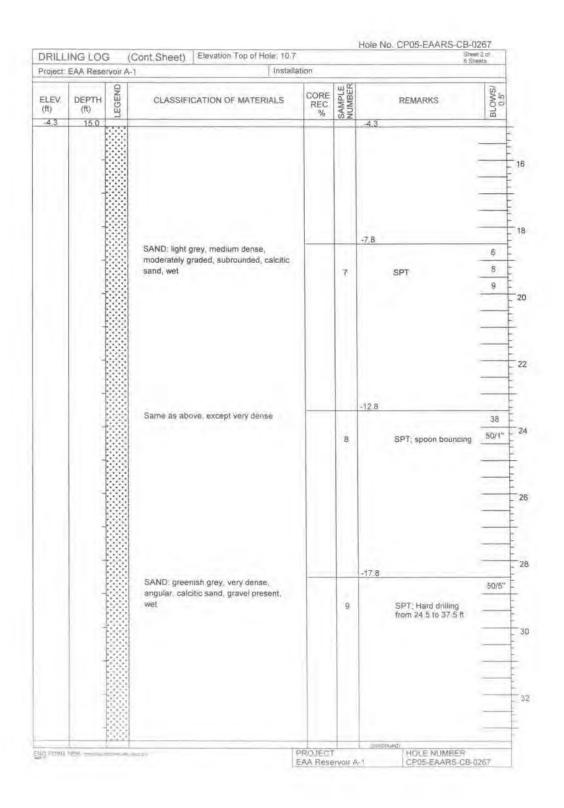
ING LOG	19	ont Sheet) Elevation Top of Hole 10.3				5 63	ents
EAA Remerv	orr A-1	Installa	itian				
	LEGEND	CLASSIFICATION OF MATERIALS	CORE REC %	SAMPLE		REMARKS	BLOWS/
33.4	930				-23.1		
		Same as above	-		-23.7	PROT	FRIDE
3				20		SP1_spoon bounding	50/3"
1				21		SPT; spoon bouncing	
1		was a second			-25.2		
- 8		Same as above					50/5"
				22	SPT		
13					-26.7		
18	33	SAND light grey, wet, dense, semi					40
+		trace sill and phosphate		23		SPT: SW-SM Well	30
				1	graded sand Moisture 27	graded sand with sitt.	17
		Same as above, except medium dense,				circulation at 37 ft	- 11
1		shells recovered		24		COT	6
400				24	70.7	Set	6
0.		Hard dark fragments and shell pieces.	1		-29.1		50/4"
. 8	Ø:	recovered				You	
12				25	100	SPT	-
4.	9	8	-	-	31.2		50/5"
1	Gravelly SAND: green/sh grey, well very dense, angular, carbonate sand						2019
1				28		SPT spoon bouncing	_
0				-32.7			
						31	
			27		SPT	31	
44.5	0		-	-	-34.2		20
18			1				6
				28		SPT, SP Poorly	7.1
					-35.7	graded sand, Moisture=25%,	12
		Top part of sample as above. Bottom			1	Caloosaha(chee Formation	9
47.0		part crips of innestories		29		SPT	
47.5		Chips of limestone			37.2		g
- 6		Same as above (top portion of above					B
73		sample)		30		591	9
450.	33			-	38.9		20
1		Gravelly SANO greenish grey, wet.			138.1		4
15	B	medium dense, semiround quartz and	1	dia.		STEEL STATE STATE OF THE STATE OF	Б
- 6		semi angular varponate sano		31	100	graded eanst with silt.	10
in the		As above but light grey polor, fine sand	-	+	340.2	Mototore=21%	6
1	32	present					-
154	23		A'	35		SFT Barehole starting to save in all	3
	DEPTH (M) 33.4	40.0 24.5 CO 44.5 CO 47.5	Same as above Same as above Same as above SAND light grey, wet, dense, semi- angular quartz and carbonate sand, trace silt and phosphate Same as above, except medium dense, shells recovered 40.0 Hard dark fragments and shell pieces recovered SAND greenish grey, wet, medium dense, mostly semiround quartz, trace silt and phosphate Top part of sample as above. Bottom part chips of limestone Top part of sample as above. Bottom part chips of limestone Chips of limestone Gravelly SAND greenish grey, wat, medium dense, mostly semiround quartz, trace silt and phosphate Top part of sample as above. Bottom part chips of limestone Gravelly SAND greenish grey wat, medium dense, as above, greenish grey wat, medium dense, greenish grey wat, greenish greenish grey wat, gr	DEPTH (m) SIL4 Same as above Same as above SAND light grey, wet, dense, semil angular quarts and carbonate sand, trace sitt and phosphate Same as above, except medium dense, shells recovered 40.0 Hard dark fragments and shell pieces recovered Gravelly SAND: greenish grey, wet, very dense, angular, carbonate sand Top part of sample as above. Bottom part chips of limestone Top part of sample as above. Bottom part chips of limestone Same as above (top portion of above sample) Gravelly SAND: greenish grey, wet, medium dense, mostly semiround quartz, trace silt and phosphate Top part of sample as above. Bottom part chips of limestone Gravelly SAND: greenish grey wet, medium dense, and semilar dense, seminound quartz and semilar dense, seminound quartz and semilar dense, seminound quartz and semilar guilar carbonate sand Ascapove buil light grey links), fine sand	DEPTH HOUSE CLASSIFICATION OF MATERIALS CORE OF HOUSE REC WAY N. Same as above 21 Same as above 22 SAND. light grey, wet, danse, semil angular quartz and carbonate sand, trace silt and phosphate 23 Same as above, except medium dense, shells recovered 24 40.0 Hard dark fragments and shell pieces recovered 25 Cravelly SAND: greenish grey, wet, very dense, angular, carbonate sand 27 SAND: greenish grey, wet, medium dense, mostly semiround quartz, trace silt and phosphate 28 Top part of sample as above. Bottom part chips of limestone 29 47.0 Chips of limestone 29 Gravelly SAND: greenish grey, wet, medium dense, mostly semiround quartz and semi angular carbonate sand 31 Asiabove, bull light grey soloi, fine sand present	DEPTH MED CLASSIFICATION OF MATERIALS CORE REC % 23.7 Same as above 21 Same as above 22 -25.2 Same as above 22 -26.7 SAND: light grey, wet, dense, semil angular quartz and carbornate sand, trace silt and phosphate 23 Same as above, except medium dense, shells recovered 24 40.0 Hard dark fragments and shell pieces recovered 25 ST. Gravelly SAND: greenish grey, wet, very dense, angular, carbonate sand 27 SAND: greenish grey, wet, medium dense, mostly semiround quartz, trace silt and phosphate 28 SAND: greenish grey, wet, medium dense, mostly semiround quartz, trace silt and phosphate 28 Top part of sample as above. Bottom part chips of limestone 29 37.2 47.0 Gravelly SAND: greenish grey, wet, medium dense, mostly semiround quartz, trace silt and phosphate 28 As above, but light grey isolor, fine sand preasent 31 As above, but light grey isolor, fine sand preasent	DEPTH (P) 2 CLASSIFICATION OF MATERIALS CORE REC SOLUTION (P) MATERIALS (CORE REC SOLUTION SET SET SECOND SOLUTION (P) SPT. Spoon bounding SPT. Spoon as above, except medium dense, shells recovered SPT. SpT. Spoon bounding SPT. Spoon bounding SPT. SpT. Spoon bounding SPT. SpT. SpT. SpT. SpT. SpT. SpT. SpT. Sp

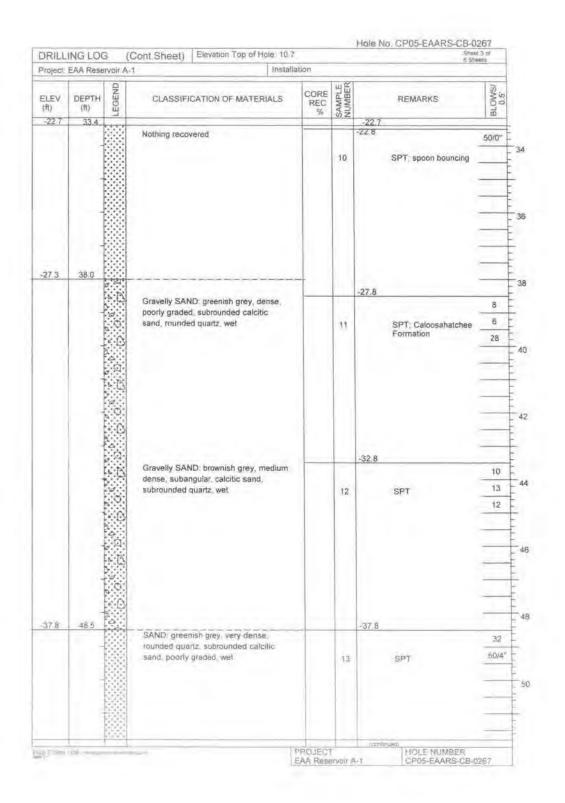


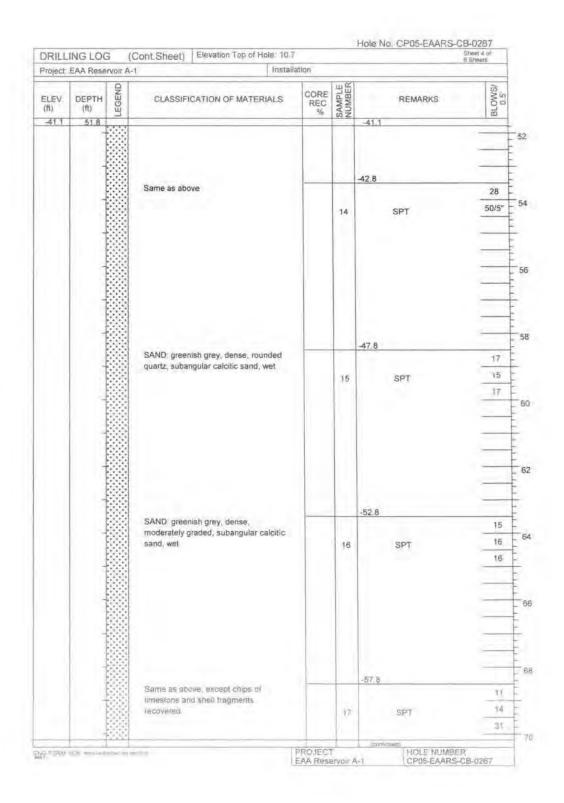
med	ING LO	-	Cont.Sheet) Elevation Top of Hole: 10	allation			8 89	HOUSE
ELEV (ft)	DEPTH (ft)	LEGEND	CLASSIFICATION OF MATERIALS	CORE REC %	SAMPLE		REMARKS	BLOWS/
-59.9	70.2		Chips of limestone, semi angular			-59.9		11
		Ŧ	carbonate sand, angular quartz, pieces of shell, trace phosphate	1	45		Moisture=22%	50/4"
		1	Same as above, except dense	-		-61.2	de i	20
	-	1						19
		1			46		SPT; spoon bouncing, Temporary casing	15
-62.7	73.0	0.22	SAND: light grey, wet, medium dense.	+-	-	-62.7	from 45 to 70 ft	12
	8.0	5.0	Chips of limestone recovered from					13
		2	bottom of sample		47		SPT; SW-SM; Well graded sand with	15
		σ.Ο.		-	-	-64.2	silt+gravel Moisture=21%	15
		0					SPT, Hard drilling from 74 to 76 ft;	14
		o.			48			
		. D	Gravelly SAND; greenish grey, wet, ver		-	-65.7	spoon bouncing	50/6"
	1		dense					13
	3	0.			49		SPT, SW-SM, Well graded sand with	50/4"
	1 3	Ô	Gravelly SAND oilve grey, wet, dense		-	-67.2	silt+gravel, Moisture=18%: apoon	-
		4	semi round quartz				bouncing	20
		9.0			50		SPT	75
		1.17	Same as above, except medium dense		-	-68.7		21
		. O.	Saine as above, except median dense					17
		0			51	1	SPT	9
			Same as above	-	-	-70.2		9
		0	Same as above					14
		1.0			52	1	SPT	13
			All the same hand the	-	-	-71.7		15
		0.	Same as above, except dense					14
					53		SPT	
		۵. اوا				-73.2		20
		200	Same as above					17
					54	SPT		17
		101				-74.7		19
		00	Gravelly SAND, plive grey, wat den			1		18
		semiround quartz, trace silf	secondaria quinta, lingua ani		55		SPT, GW-GM, Well	2.1
		0.01				-75.2	graded gravel with sill and sand;	15
		1	Same as above			1	Moisture=14%	12
		198			58		SPT	175
					1	77.7	24.1	18
		163	Predominantly sumi mond quarts from sams present					21

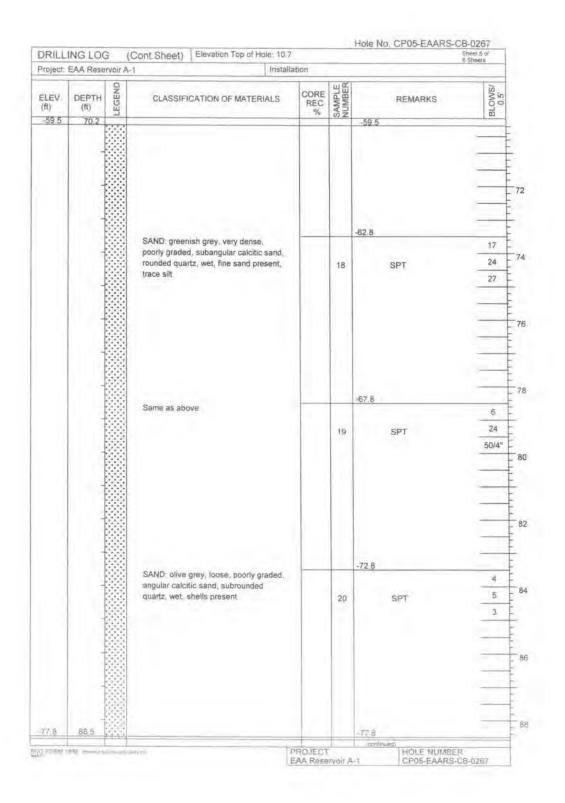


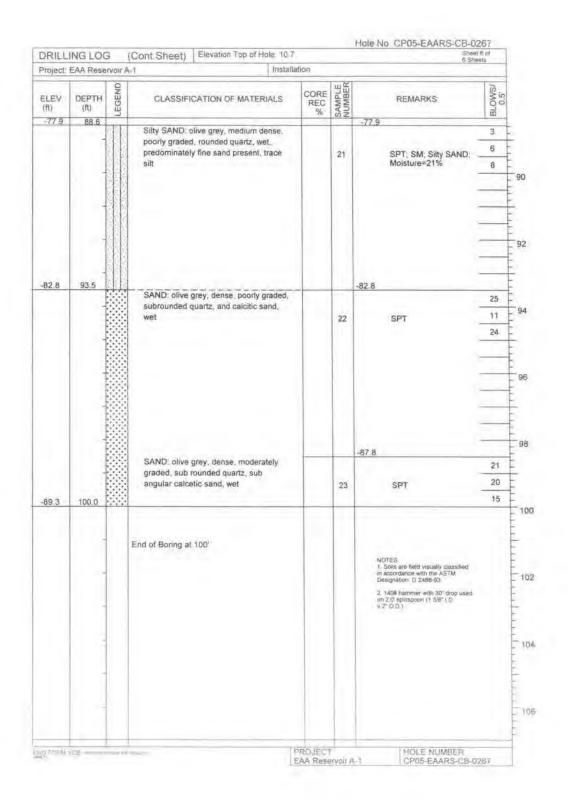
DRILL	ING LO	3 0	hyision	Installati	on			O. CP05-EAARS-CB-0	15 68	
l Projec	EAA Re	servoir.	A-1	10 Size	and type	of bit	3" bit.	Rotary Method		
2. Locat	on N750	246, E7	87702.1 - NAD 1983	_				NAVE) 1988		
			e & Associates, Inc.	12 Manufacturer's Designation for Drill Diedrich D-50						
	No CP05	-		13 Total Number of Overburgen Samples Taken N/A						
	s of Driller		Williams	14 Total Number of Core Boxes. 1						
	tion of Hol enical		and	15 Elevation Ground Water: Not measured						
				16 Date Hole Started Completed 7/14/2005 7/14/2005						
	ness of Bu			17. Elevation Top of Hole: 10.7 (ft)						
	ness of ca		N/A	18. Total Core Recovery for hole: N/A						
9 Depti	of hole. 1	19 Inspector A.M. Noronha					7			
ELEV.	DEPTH (ft)	EGEND	CLASSIFICATION OF MATER	RIALS	CORE REC %	SAMPLE		REMARKS	BLOWS?	
10.7	0.0	-			20	0.2			1 10	
			SAND greenish grey, very dense				10.7		19	
			moderate to well graded, angular sand, dry	calcitic		4	6	DDT of level	38	
		3.5	sonu, ury			7	0.	SPT, chips of limestone recovered	100	
		8.5	Come as about a second and to be				9.2	100000000000000000000000000000000000000	50/5"	
		333	Same as above, except wet to dr	y			SPT, hard drilling	27		
		3.3				2		SPT, hard dolling	38	
7.7	70	0.00					94	from 1 to 3 ft.	50/5"	
F.1.	3.0	0.74	Gravelly SAND pale to greenish	grey.			7.7		5	
		1.13	medium dense, poorly graded, as						-	
	1 4		wet			3		SPT	9	
6.2	4.5	35					6.2		8	
	1	333	SAND blive gray, very dense						6	
		381	moderately graded, angular calci	tic:					- 11	
		623	sand, wet, fine sand, trace sift			-4	10.0	SPT	-	
4.7	6,0	100					4.7		50/3"	
			CORE LIMESTONE white, very					HQ coring, Core 6 to		
		1	quite porous, shalls in upper can portion		54	1				
	1 3		a de la constante de la consta		IRQD		9.5 #		-	
		-			29%)		Design .		_	
	1	- 1								
		T								
35	1000						1			
1.2	9.5	2.5.7.4	SAND: greenish grey, dense,		-	-	12			
			moderately graded, subangular i	calcilic					1	
		333	sand, wet			5		SPT limestone chips	3	
		3.5%						recovered at the pottom of the sample	32	
	1							London of the sample		
	1 3	353							-	
		188								
		(888)								
		133								
		153	PAND washing the Control			-	-28			
		338	SAND greensh grey very tools subsequiar colottic sand, well						1	
		233	Attendance matchine and was			E		BRT	1	
		333				-			3	
	-	1000							- 4	

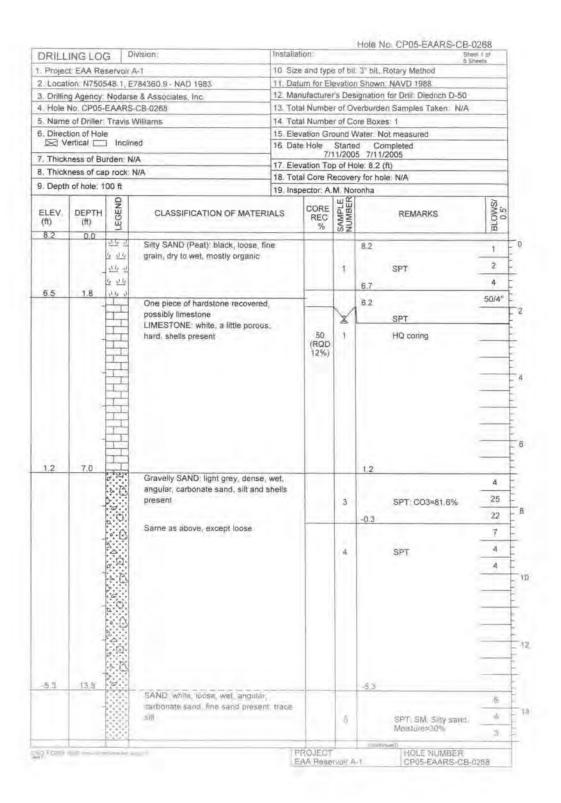


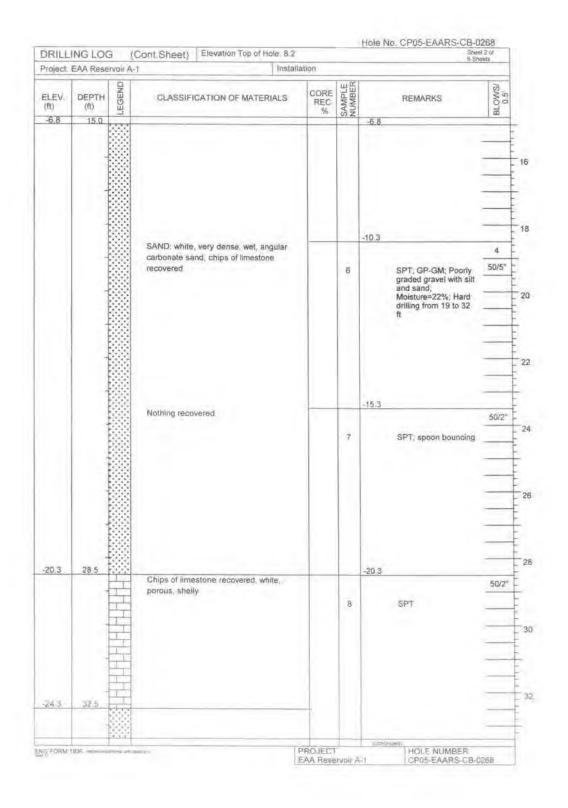


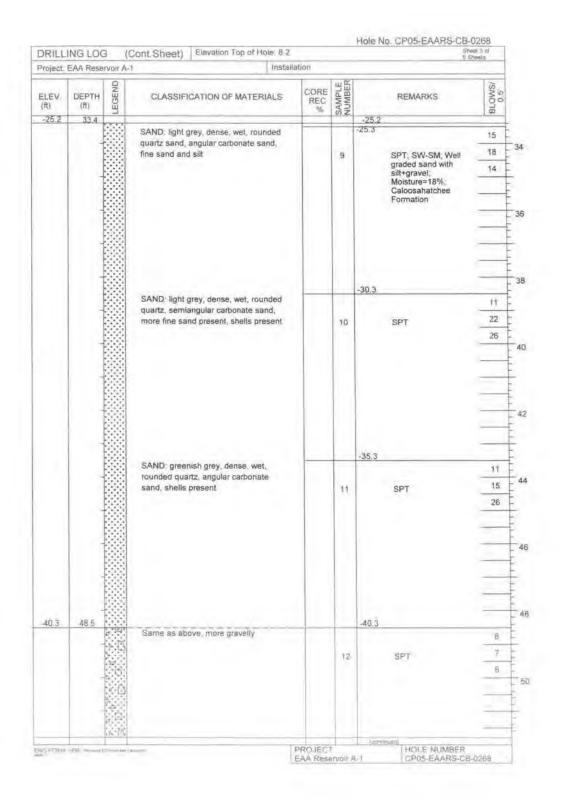


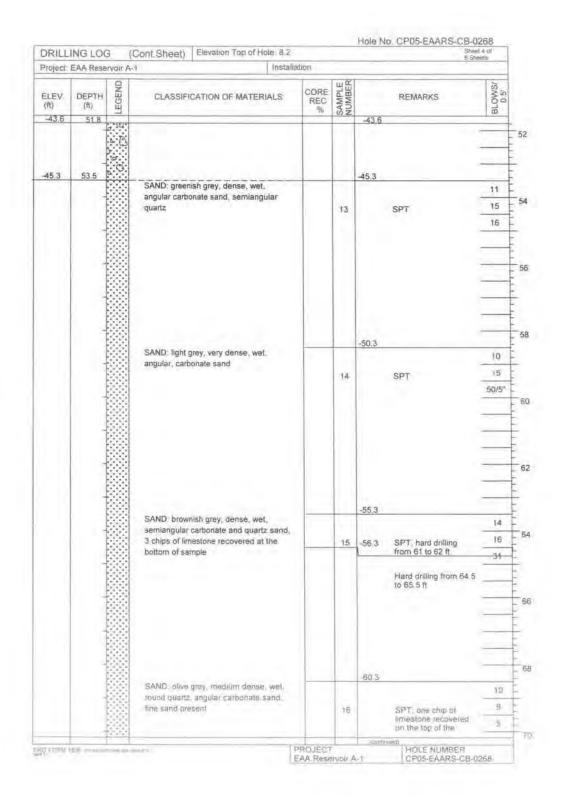


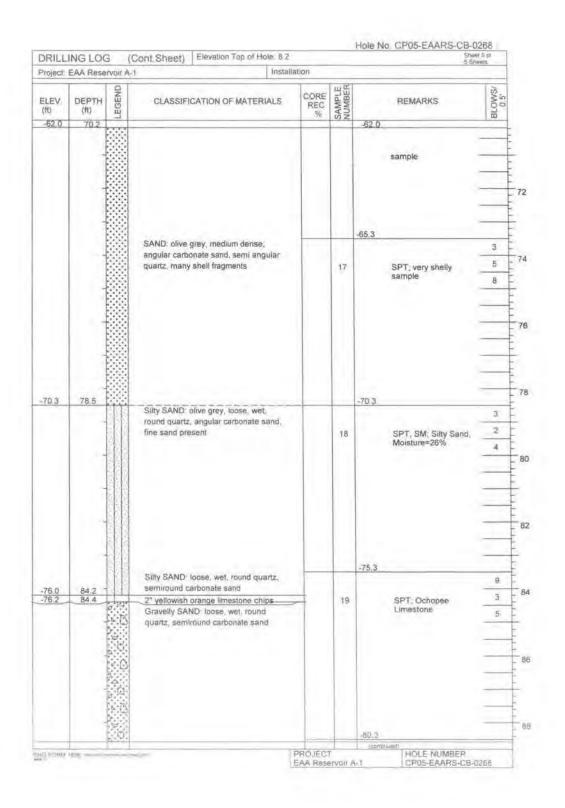


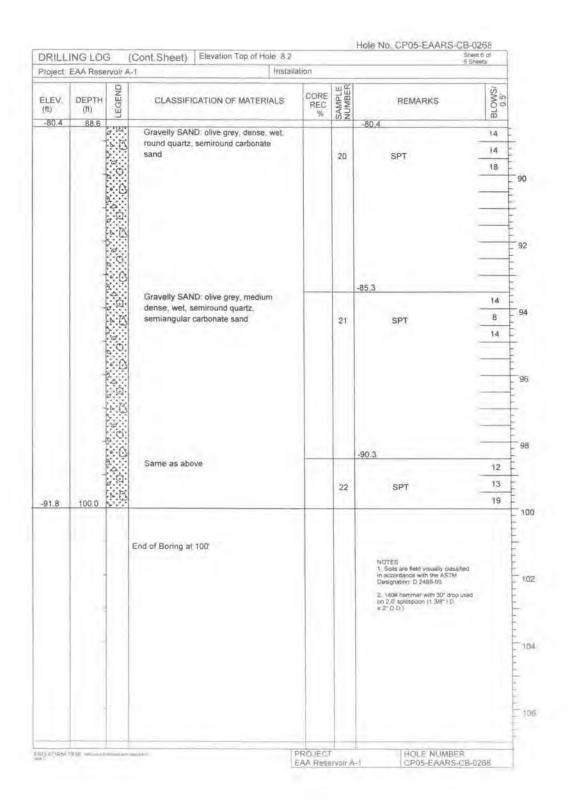


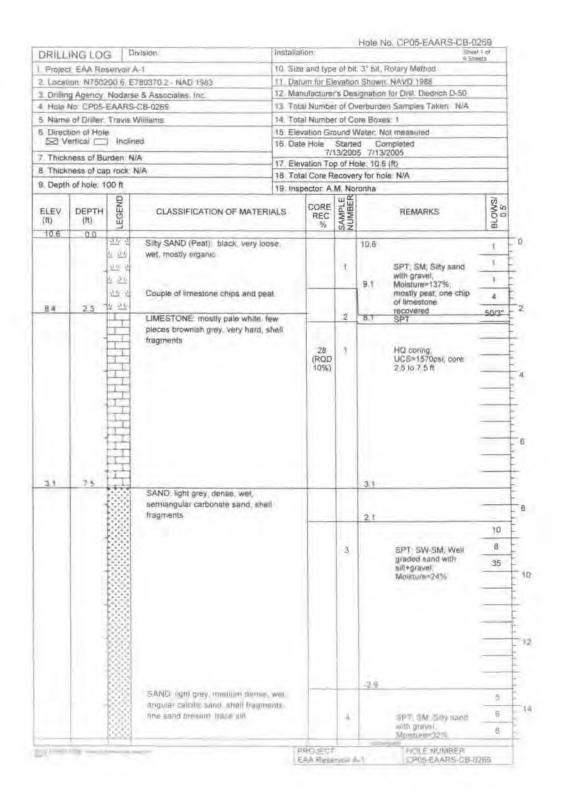


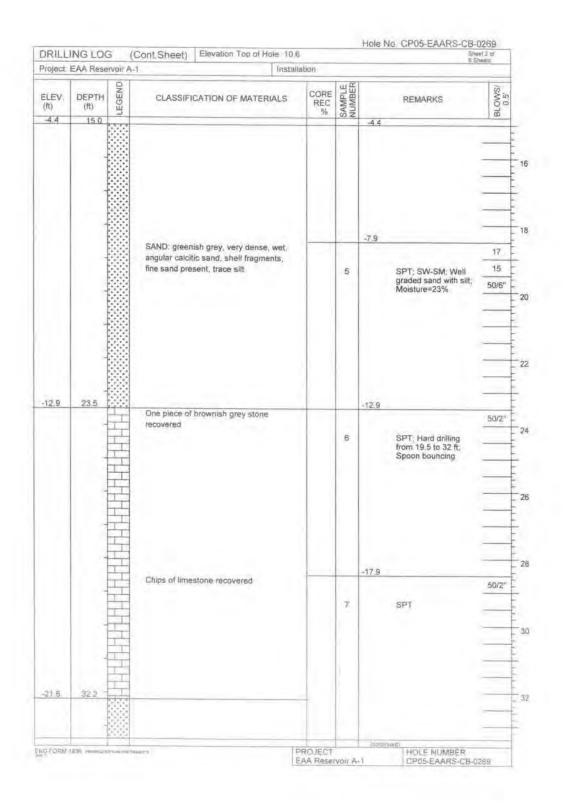


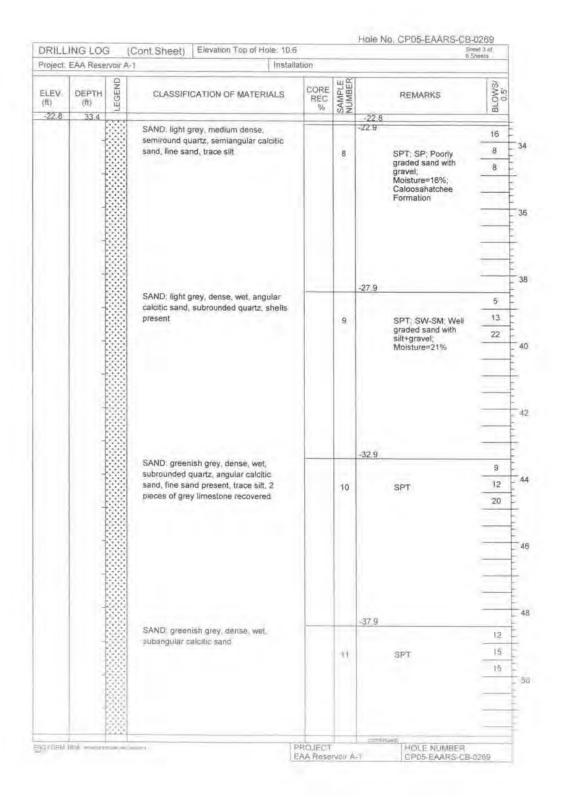


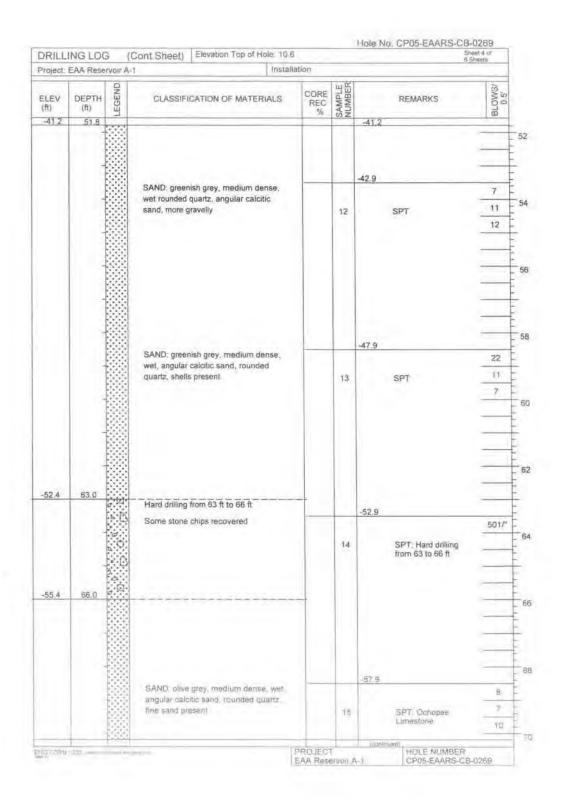


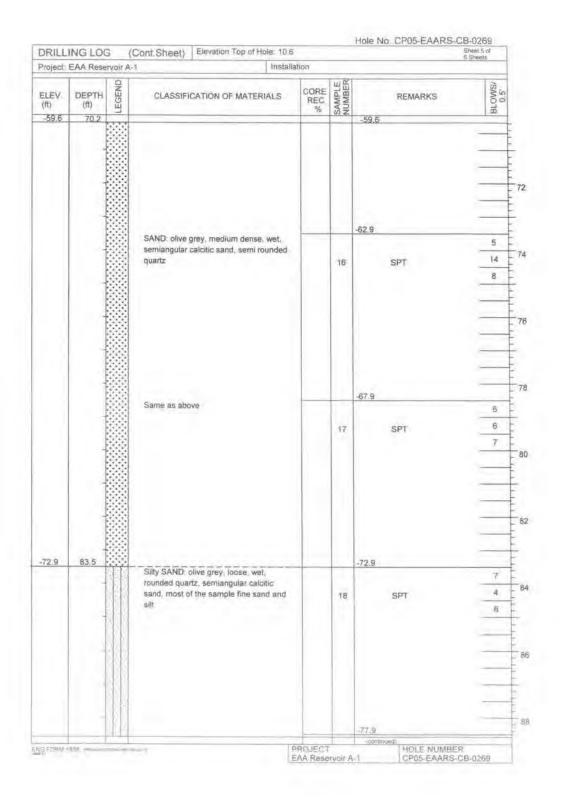


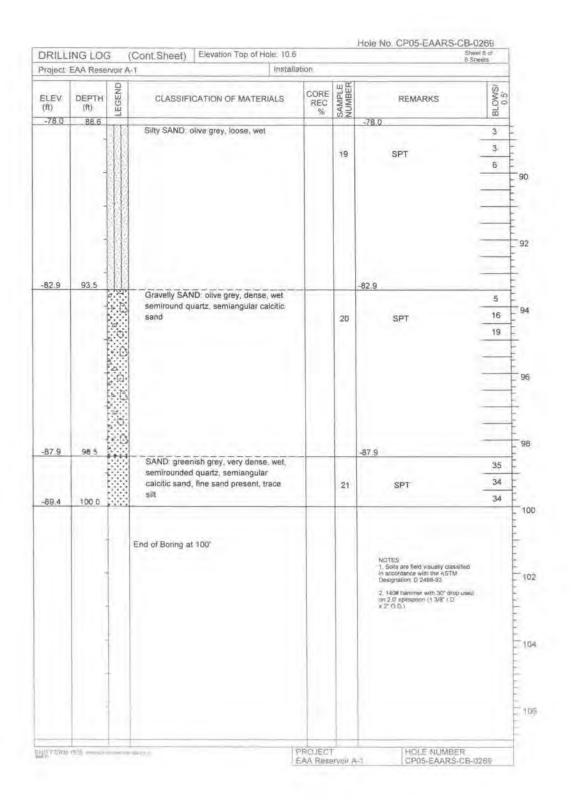




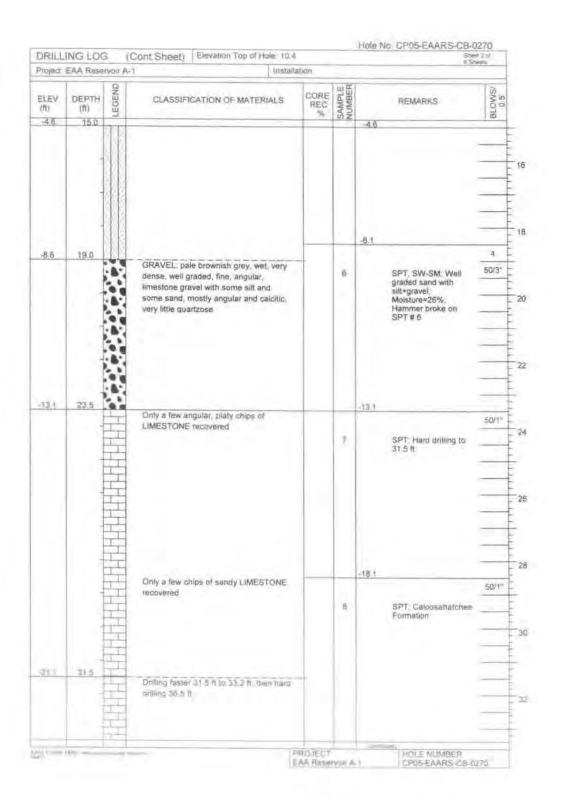


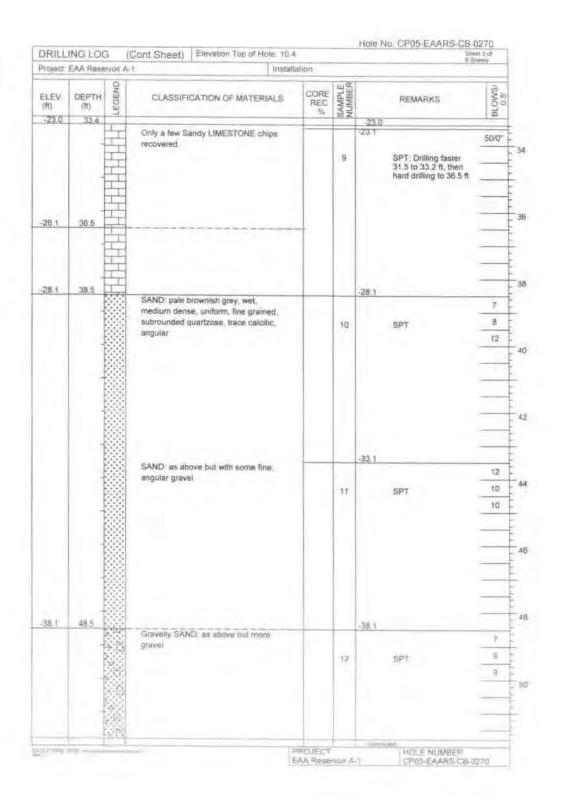


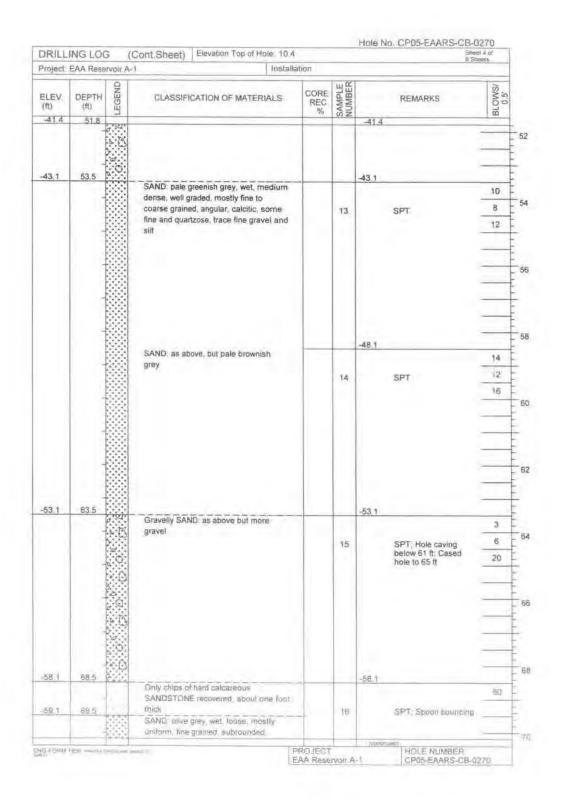


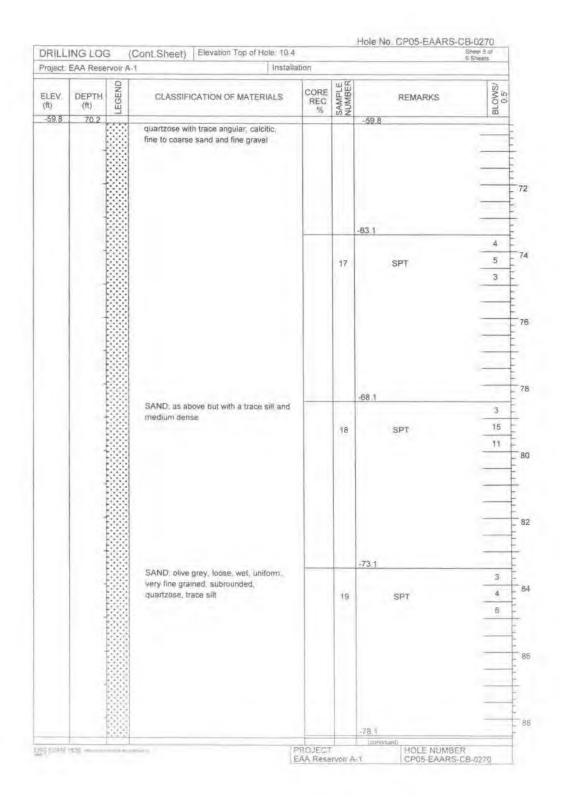


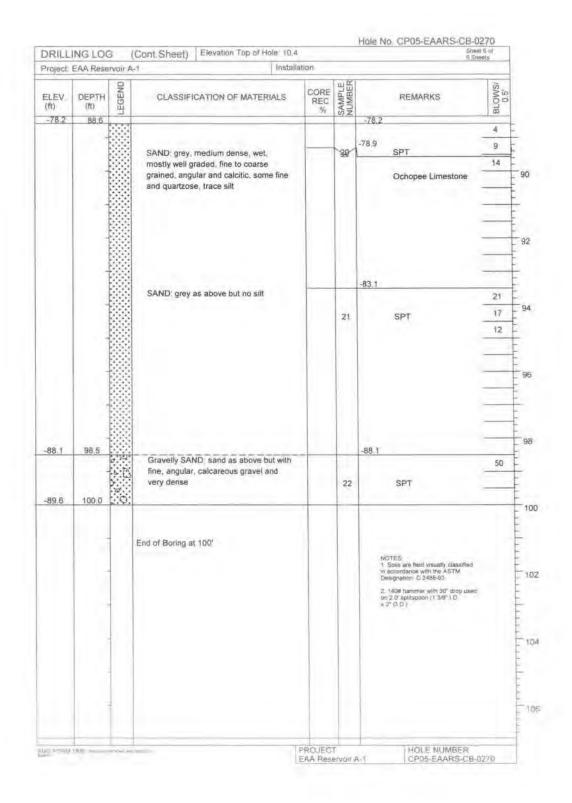
DRILL	ING LO	3 0	vision: Installation					Street 1 of 5 Simets	1	
Projec	L EAA Re	servoir /	4.1	10 Size	and type	of bit	3" bit_Rotary Method			
2 Locat	ion N750	91,9 E	776521.6 - NAD 1983	11. Datu	m for Ele	vation	Shown NAVD 1988			
	Annual Control		e & Associates, Inc.	12. Manufacturer's Designation for Drill, Diedrich D-50						
4. Hole I	No. CP05-	EAARS	CB-0270	13. Total Number of Overburden Samples Taken: N/A						
	of Driller		emke	14. Total Number of Core Boxes: 1						
	ion of Hole		ed	15. Elevation Ground Water: Not measured 16. Date Hole Started Completed						
7. Thick	ness of Bu	rden: N	/A.	17 Clair		_	7/11/2005 ble: 10.4 (ft)		+	
B. Thick	ness of ca	p rock: I	N/A	_			y for hole: N/A		7	
Depth	of hole: 1	00 ft		19. Inspi		_	I for field, tack			
ELEV (ft)	DEPTH (ft)	EGEND	CLASSIFICATION OF MATERIA		CORE REC %	SAMPLE	REMARKS	BLOWS/	0.5	
10.4	0.0	-			10	OZ.		141		
	1	7.1	Gravelly SAND: pale brownish grey				10.4	6		
		1-13	moist, medium dense, well graded, to coarse grained, calcitic, road fill	nne		,	SPT	10	T	
	-	10	is socios granies, tantans, road in			4	in a second	8	+	
		c.O.					8.9		-	
		6.0						6		
7.9	2.5	2				2	SPT, SW-SM; W	fell 2		
	2010	20 1	Peat				graded sand with			
	L. P	4 34					silt+gravel; Moisture=158%			
6.9	3.5	40.9	Sandy CDAVEL - washingd				6,9		-	
			Sandy GRAVEL weathered LIMESTONE					25		
		- 1	The state of the s			3.	SPT	17		
5.4	5.0	10					5.4	50/5		
5.4	5.0	1	LIMESTONE, tan to light brown, fir	ne			24			
		T	grained, fossiliferous, vuggy, hard				HQ coring: UCS=1860psi, Cased	-		
		1	strong to soft, weak and porous		26	1		anni -		
	T				(RQD) 22%)		10 5 ft	maeu		
				1	100					
		1								
								-	-	
42	64 .	T						_		
2.1	8.3	2332	SAND: pale brownish grey, wet, de	ense			1.9			
			well graded, fine to coarse grained					4		
			angular calcitic, trace silt and fine				SPT GW Sitty	15		
		350				4	with sand;	gravei 34	-	
	-						Moisture<27%	.34		
		333								
		2.5								
		933								
								-		
									_	
~ .		20.23					197.1			
-3.1	13.5	111	Sity SAND as above but more all	-	-	-	-3.1	-		
			and search we appeal that make the					9	-	
						8	SPT SW Well	5		
							graded sand wit gravet Mothure			
		Part I			1	+	disser alderine	200.70	-	

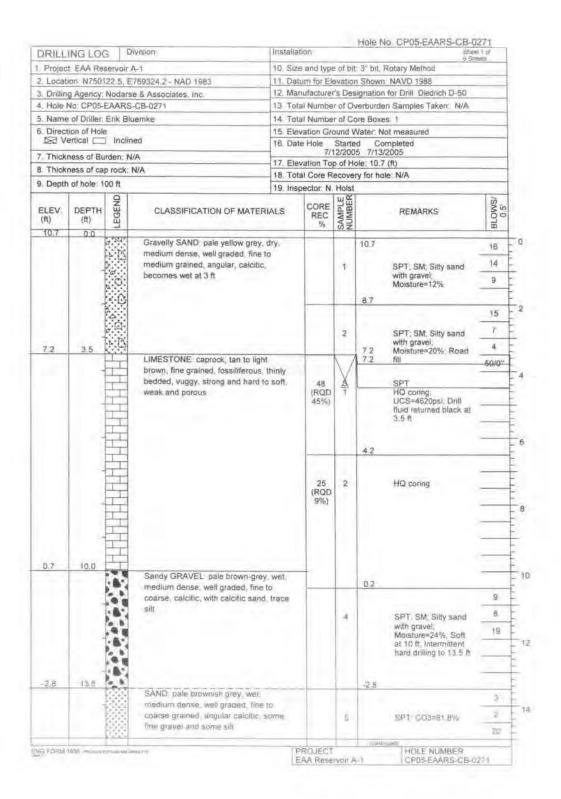


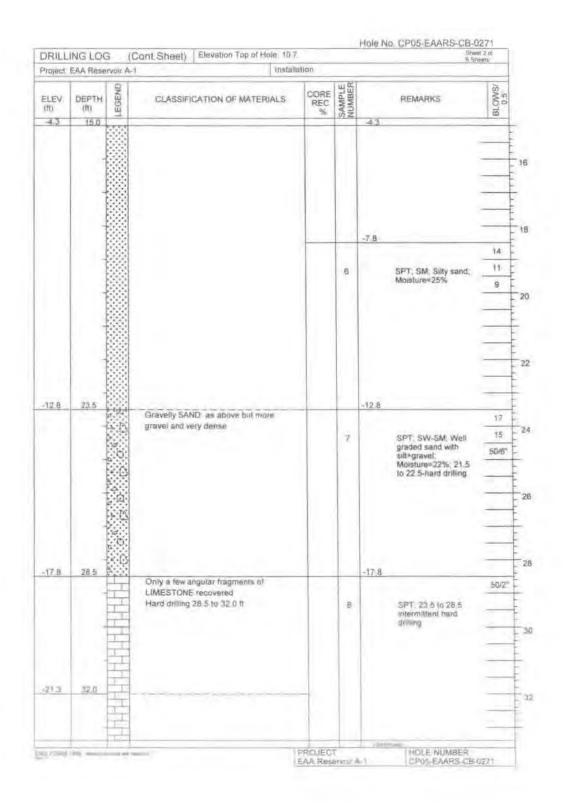


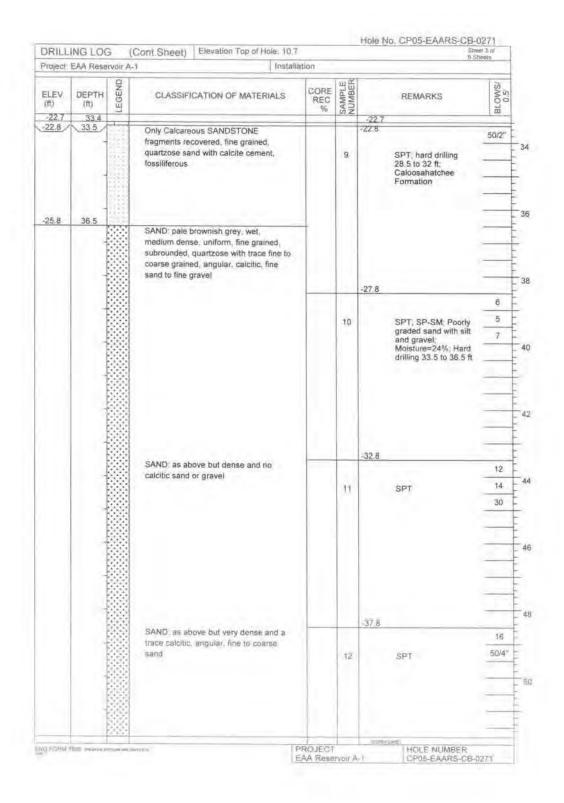


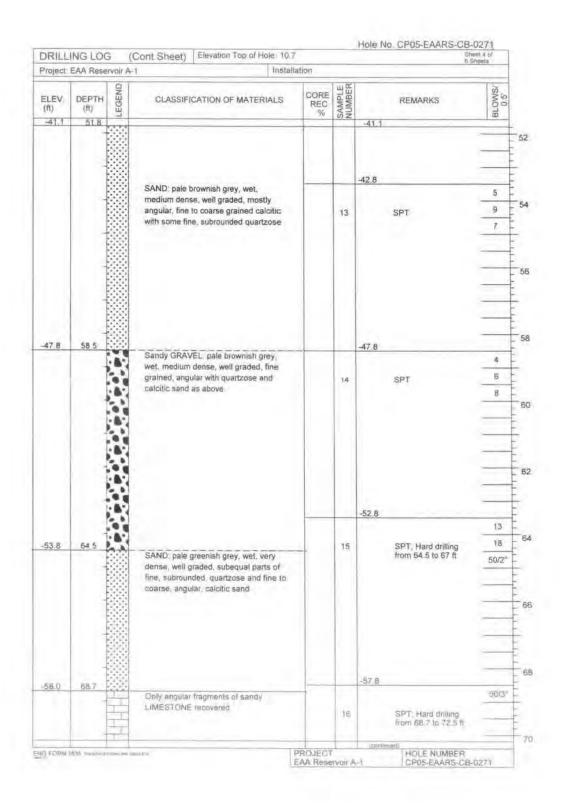


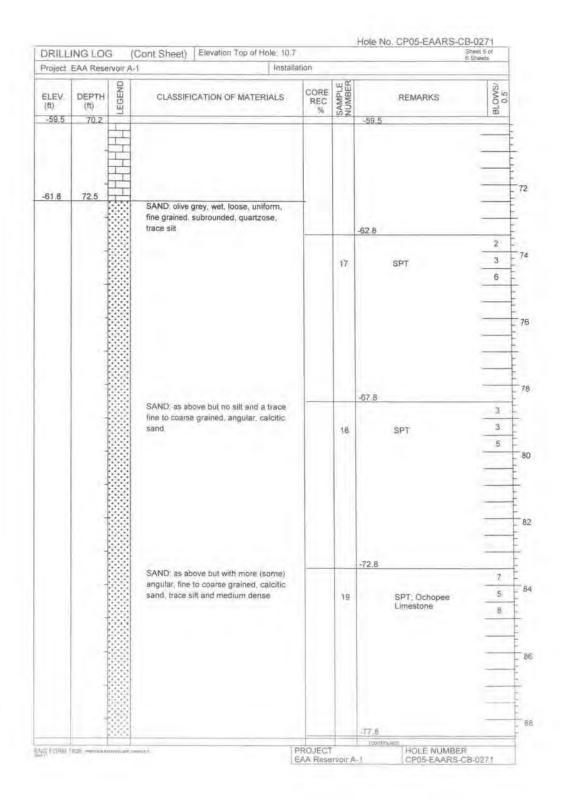


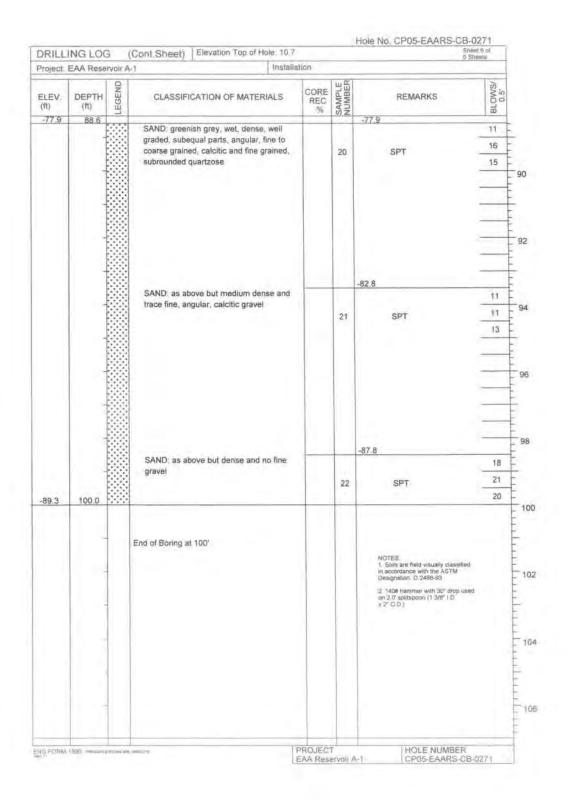












DRILLING LOG Division In					Hole No CPUS-EAARS-CB-0272				
					10 Size and type of bit. 3" bit, Rolary Method				
					11. Datum for Elevation Shown NAVD 1988				
or terming rigoring. Telegrap of recovering from					12. Manufacturer's Designation for Drill. Diedrich B-50				
The state of the s					13. Total Number of Overburden Samples Taken: N/A				
5. Name of Driller: Erik Bluemke					14. Total Number of Core Boxes 1				
5. Direction of Hote Solventical inclined					15. Elevation Ground Water. Not measured 16. Date Hole Started Completed				
7 Thickness of Burden N/A					7/6/2005 7/7/2005				
		-		17. Ele			ole: 8.7 (ft)		
	ness of car	-	NA	1	18. Total Core Recovery for hole, N/A				
9. Depth of hole: 100 ft					19 Inspector N. Holst				
ELEV (ft)	DEPTH (ft)	LEGEND	CLASSIFICATION OF MATE	TCATION OF MATERIALS		SAMPLE	REMARKS /S/AG		
8.7	0.0	36 3	Plants O January			37.00	**		
8.5	02	1	Peat 2 inches LIMESTONE caprock tan to lig	THI.			8.7	50/2"	
		1	brown, mostly fine grained, fossiliferous, vuggy, strong and hard to soft, weak			1	7.7 SPT		
	1 3				46 (RQD				
		1	and porous	3/4-2003					
	1 12	1				1	HQ coning. UCS=2650psi	-	
]	1		14%)					
	1	1							
	1	1							
								_	
3,7	5.0	d 257.	Controlly PANPs and beautiful.	th Carlo	-	-	3.7		
			Gravetly SAND: pale brownish gray, very dense, well angular, well gnaded, fine to coarse grained, calculic, trace still					6	
		88				2	SPT, SM, Sitty sand	11	
	1.0						with gravel. Moisture=18%	50/2"	
		333					MIONAINES 1076		
	1.5							_	
1.2	7.6	211	Sandy Sitty GRAVEL as above	toit week	+		1.2	-	
		= De	dense and mostly line calcitic					50	
		96				3	SPT		
		0 10				1			
		30							
		a 50						-	
43	10.0	AA	SAND as above but less dens		-		113	-	
			(medium) with some silt and fir				BPT SM; Silty sand	8	
	-	1.4 0				4		8	
	1					1	with gravet. Molesture+29%	6	
						1	William 2.2.5		
	1 6							-	
		230						_	
						1			
-4.0	13.0						4.0		
4.0	1	Tan	Drify a few angular fragments :	Te.				30/1"	
	1 3	1	LIMESTONE recovered				and the second	-490	
		1				5	SPIT Hard (Infiling to		
						10.50			
					1	A	T. Committee and		

